ARIZONA DEPARTMENT OF TRANSPORTATION

REPORT NUMBER: FHWA-AZ88-202-IV

SMALL SIGN SUPPORT ANALYSIS

Phase III Benefit/Cost Analysis

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August 1988

Prepared for:

Arizona Department of Transportation 206 South 17th Avenue Phoenix, Arizona 85007 in cooperation with U.S. Department of Transportation Federal Highway Administration The contents of the report reflect the views of the authors who are responsible for the facts and the accuracy of the data presented herein. The contents do not necessarily reflect the official views or policies of the Arizona Department of Transportation or the Federal Highway Administration. This report does not constitute a standard, specification, or regulation. Trade or manufacturer's names which may appear herein are cited only because they are considered essential to the objectives of the report. The U.S. Government and the State of Arizona do not endorse products or manufacturers.

		TECHNICAL REPORT STANDARD TITLE PAG
1. Report No.	2. Government Accession No.	3. Recipient's Catalog No.
FHWA-AZ88-202-3]	
4. Title and Subtitle		
		5. Report Date
SMALL SIGH SUPPORT ANA	212411	August 1983
PHASE III - BENEFIT/CO	ST ANALYSIS	6. Performing Organization Code
7. Author(s)		
Hayes E. Ross, Jr., Ro	ger P. Bligh, and	8. Performing Organization Report No.
Dean L. Sicking	30. 11 011gm, and	
9. Performing Organization Name and	Address	Research Report 7024-3
Texas Transportation	Institute	TO. WORK Unit No.
Texas A&H University		11. Contract or Grant No.
College Station, Texas	s 77843	HPR-PL-1(31) Item 202
		13. Type of Report and Period Covered
12. Sponsoring Agency Name and Addr		The private due Laures Covered
Arizona Department of	Transportation	Final Report
Highway Division		Phase III
206 South Seventeenth	_Avenue	14. Sponsoring Agency Code
Phoenix, Arizona 8500 15. Supplementary Notes	/	
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	Kolaja	
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Guidelines were sign installation, no replaced by a system cost effective sign sideveloped at TTI was existing systems installations for both carbon steel U-posts. installations and for to three 3 lb/ft high standard steel pipe wisections of laminated This report is or other two are: Small Sign Support Phase I - Crash Thase II - Development of the post of the standard steel pipe wisections of laminated. Small Sign Support Safety, Small, Impact, Upgrade	that does meet the safety stupport for new installations used to develop the guideling that were analyzed included a square steel tubes (ADOT I Systems that were consider upgrading existing systems carbon steel U-posts with a carbon steel U-posts with a carbon steel U-posts with a veneer lumber. The of three reports prepared that the program opposes the steel of the same of the series of the same opposes of the same of th	safety standards, should be candards, and (2) selecting a s. A benefit/cost (B/C) program, nes. I single and multiple post 22 systems) and 3 lb/ft high red as candidates for new include installations with up in improved short ground splice, pase, and hollow rectangular I in the subject project. The support (two volumes)
Guidelines were sign installation, no replaced by a system cost effective sign sideveloped at TTI was installations for both carbon steel U-posts. installations and for to three 3 lb/ft high standard steel pipe wisections of laminated This report is or other two are: Small Sign Support Phase I - Crash Thase II - Development of the properties of the properties of the standard steel pipe wisections of the sta	that does meet the safety stupport for new installations used to develop the guideling that were analyzed included a square steel tubes (ADOT I Systems that were consider upgrading existing systems carbon steel U-posts with a carbon steel U-posts with a carbon steel U-posts with a uni-directional slip to veneer lumber. The of three reports prepared that the program opposed of a New Small Sign Steet Progr	t safety standards, should be candards, and (2) selecting a s. A benefit/cost (B/C) program, nes. I single and multiple post 22 systems) and 3 lb/ft high red as candidates for new include installations with up in improved short ground splice, pase, and hollow rectangular I in the subject project. The support (two volumes)

ACKNOWLEDGMENTS

The authors express their sincere appreciation for the cooperation and guidance of Mumtaz Sarsam, Donald Cornelison, Larry Scofield, Rudy Kolaja, and Frank McCullagh of ADOT. Other ADOT personnel on the Project Advisory Committee who provided valuable input included Robert Conklin, John Hauskins, James Pyne, Roger Hatton, Robert Cromley and Richard Powers. The suggestions of Nate Banks of FHWA were also appreciated.

Thanks also go to Patsy Astle for her help in preparing the report and to Perry Brown and Asif Qureshy for their assistance in drafting figures and tables.

PREFACE

Arizona Department of Transportation (ADOT) Project HPR-PL-1(31), Item 202, "Small Sign Support Analysis," was initiated by the Texas Transportation Institute (TTI) October 1, 1984. Originally, the project consisted of 18 full-scale vehicular crash tests to evaluate ADOT small sign supports. Upon completion of one-half of the tests it became evident that additional tests would be needed. The project was modified May 31, 1985 to increase the number of tests to 23. Also, the modification included a benefit/cost (B/C) study to develop guidelines for upgrading existing ADOT small sign supports and for selection of new small sign supports. The project was again modified in August, 1986 to develop an improved small sign support system. The B/C study was also modified to include results of the improved support system.

A description of the 23 crash tests and results therefrom are presented in a report entitled "Small Sign Support Analysis: Phase I \sim Crash Test Program."

A description of the study in which an improved sign support system was developed is presented in a report entitled "Small Sign Support Analysis: Phase II - Development of New Small Sign Support," (two volumes).

A description of the B/C study and results therefrom are presented herein.

TABLE OF CONTENTS PHASE I

				Page
I.	INTROD	ICTION		1
п.	SUMMAR	AND EVALUATION OF	TEST RESULTS	2
	II-A.	Impact Performance	Criteria	2
	II-B.	Test Results		6
			Sign Support (Tests 1 and 2)	8
			eel Tube, Single Post 3 and 4)	8
			eel Tube, Multiple Posts	0
		(Tests	5, 6, 19, and 20)	9
			ost, Single Support 7, 8, and 13)	9
		II-B-5. Steel U-P	ost, Multiple Supports	_
		(Tests 17, 18	9, 10, 11, 12, 14, 15, 16, , 21, 22, and 23)	9
III.	CONCLU	SIONS		12
APPEND	OIX A.	EST DETAILS		A-1
	A-1	. Test Vehicles		A-2
	A-2	. Design and Insta	llation Details of Test Articles	A-2
			Sign Support (Tests 1 and 2)	A-2
			teel Tube, Single Post 3 and 4)	A-9
		A-2-3. Square S	teel Tube, Multiple Posts	
			5, 6, 19, and 20 Post, Single Support	A-9
			7, 8, and 13)	A-16
		A-2-5. Steel U-	Post, Multiple Supports	
			9, 10, 11, 12, 14, 15, 16, 8, 21, 22, and 23)	A-16
	A-3	. Test Results	o, 10, 11, 111	A-16
		A-3-1. Test 1 A-3-2. Test 2		A-33
		A-3-3. Test 3		A-40 A-48
		A-3-4. Test 4		A-40 A-54
		A-3-5. Test 5		A-61
		A-3-6. Test 6		A-68
		A-3-7. Test 7		A-76
		A-3-8. Test 8		A83

	<u>Page</u>
A-3-9. Test 9	A-90
A-3-10. Test 10	A-97
A-3-11. Test 11	A-103
A-3-12. Test 12	A-109
A-3-13. Test 13	A-115
A-3-14. Test 14	A-123
A-3-15. Test 15	A-129
A-3-16. Test 16	A-135
A-3-17. Test 17	A-141
A-3-18. Test 18	A-147
A-3-19. Test 19	A-153
A-3-20. Test 20	A-159
A-3-21. Test 21	A166
A-3-22. Test 22	A-172
A-3-23. Test 23	A-178
APPENDIX B. PROPERTIES OF SIGN POSTS	8-1
APPENDIX C. SOIL PROPERTIES AT TEST SITE	C-1
APPENDIX D. DATA ACQUISITION SYSTEMS	D-1
D-1. Deceleration Measurements	D-2
D-2. High-Speed Cine	D-2
APPENDIX E. REFERENCES	E-1
THE THE TALLET THE THE THE THE THE THE THE THE THE T	E~1

TABLE OF CONTENTS PHASE II

Sec	ction Volume I (Report)	<u>Page</u>
1.	Introduction and Background Information	1
2.	Splice Configurations	3
	2.1. Selection of Moment Arm Lengths 2.2. Assumptions and Calculations 2.3. Calibrated Bolts 2.4. Back to Back 2.5. Nested 2.6. Face to Face 2.7. Box.	3 4 6 7 8 9
3.	Test Procedures	11
	3.1. Bending	13
4.	Results	17
	4.1. Bending Tests	22
5.	Discussion	28
	5.1. Critical and Non-critical Splice Configurations	28 28
	5.2.1. Bolted Splices	
	5.3. Splice Performance in Bending	
	5.3.1. Back to Back and Nested Splices	39 40
	5.4. Splice Performance in Torsion	46
	5.4.1. Back to Back Splices	47
	5.5. Splice Performance in Combined Bending and Torsion	51 58 59
6.	Pendulum Tests	62
	6.1. Purpose	62 62 62

<u>Section</u>	(11 11 11 11 11 11 11 11 11 11 11 11 11	<u>Page</u>
6.5. 6.6.	Electronic Instrumentation	. 64 . 64
	ale Crash Tests	
7.2. 7.3.	Introduction Instrumentation and Data Analysis U-Post Test Installations Test Results	. 67 . 67
	7.4.1. Test 7024-24. 7.4.2. Test 7024-25. 7.4.3. Test 7024-26. 7.4.4. Test 7024-27.	. 73 . 78
7.5. 7.6.	Slip-Base Test Installations	. 88 . 88
	7.6.1. Test 7024-29	. 88 . 96
8. Conclus	ion	. 100
References		. 101
	Volume II (Appendices)	
Appendix A	: Preliminary 72" Bending Tests	. A-1
Mario Back	Bolt Summary Tables on 3 lb/ft 80 ksi Nominal Yield Stress to Back, Nested and Face to Face Splices cal and Non-critical Configurations	
Appendix B	3: 17" Bending Tests	. B-1
Frank Mario Back	Bolt Summary Tables Ilin 3 & 4 lb/ft Posts - 60 ksi In 3 & 4 lb/ft Posts - 80 ksi In 3 & 4 lb/ft Posts - 80 ksi In Back, Nested and Face to Face Splices Ito Back Non-critical Configurations	
Appendix C	: Final 71" Bending Tests	. C-1
Frank Mario Back	Bolt Summary Tables Clin 3 & 4 lb/ft Posts - 60 ksi En 3 & 4 lb/ft Posts - 80 ksi to Back and Nested Splices cal Configuration	

<u>Section</u> <u>Page</u>
Appendix D: Torsion Tests D-1
Field and Calibrated Bolt Summary Tables Franklin 3 & 4 lb/ft Posts – 60 ksi Marion 3 & 4 lb/ft Posts – 80 ksi Back to Back and Nested Splices Post W/O Splice
Appendix E: 75" Combined Bending and Torsion Tests E-1
Field Bolt Summary Tables Franklin 3 & 4 lb/ft Posts - 60 ksi Marion 3 & 4 lb/ft Posts - 80 ksi Back to Back and Nested Splices Critical Configuration
Appendix F: Preliminary 72" Bending Tests F-1
Calibrated Bolt Summary Tables Marion 3 lb/ft 80 ksi Nominal Yield Stress Back to Back, Nested and Face to Face Splices Critical and Non-critical Configurations
Appendix G: 17" Bending Tests G-1
Calibrated Bolt Summary Tables Franklin 3 & 4 lb/ft Posts - 60 ksi Marion 3 & 4 lb/ft Posts - 80 ksi Back to Back, Nested, Face to Face and Box Splices Critical and Non-critical Configurations
Appendix H: Final 71" Bending Tests H-
Calibrated Bolt Summary Tables Franklin 3 & 4 lb/ft Posts - 60 ksi Marion 3 & 4 lb/ft Posts - 80 ksi Back to Back and Nested Splices Critical Configuration
Appendix I: 75" Combined Bending and Torsion Tests I-
Calibrated Bolt Summary Tables Franklin 3 & 4 lb/ft Posts - 60 ksi Marion 3 & 4 lb/ft Posts - 80 ksi Back to Back and Nested Splices Critical Configuration
Appendix J: Preliminary 72" Bending Tests J-
Base Stress vs Tip Deflection (Field Bolts) Marion 3 lb/ft 80 ksi Nominal Yield Stress Back to Back, Nested and Face to Face Splices Critical and Non-critical Configurations

<u>Pac</u>	<u>ge</u>
Appendix K: 17" Bending Tests K-	- l
Base Stress vs Deflection at Point of Load (Field Bolts) Franklin 3 & 4 lb/ft Posts - 60 ksi Marion 3 & 4 lb/ft Posts - 80 ksi Back to Back, Nested, Face to Face and Box Splices Critical and Non-critical Configurations	
Appendix L: Final 71" Bending Tests L-	- 1
Base Stress vs Tip Deflection (Field Bolts) Franklin 3 & 4 lb/ft Posts - 60 ksi Marion 3 & 4 lb/ft Posts - 80 ksi Back to Back and Nested Splices Critical Configuration	
Appendix M: Torsion Tests M-	- 1
Applied Torque vs Post Rotation (Field Bolts) Franklin 3 & 4 lb/ft Posts - 60 ksi Marion 3 & 4 lb/ft Posts - 80 ksi Back to Back and Nested Splices Post W/O Splice	
Appendix N: 75" Combined Bending and Torsion Tests N-	- l
Base Stress vs Tip Rotation (Field Bolts) Franklin 3 & 4 lb/ft Posts - 60 ksi Marion 3 & 4 lb/ft Posts - 80 ksi Back to Back and Nested Splices Critical Configuration	
Appendix O: Preliminary 72" Bending Tests	- 1
Base Stress vs Relative Bolt Tension (Calibrated Bolts) Marion 3 lb/ft 80 ksi Nominal Yield Stress Back to Back, Nested and Face to Face Splices Critical and Non-critical Configurations	
Appendix P: 17" Bending Tests p.	- 1
Base Stress vs Relative Bolt Tension (Calibrated Bolts) Franklin 3 & 4 lb/ft Posts - 60 ksi Marion 3 & 4 lb/ft Posts - 80 ksi Back to Back, Nested, Face to Face and Box Splices Critical and Non-critical Configurations	
Appendix Q: Final 71" Bending Tests	- 1
Base Stress vs Relative Bolt Tension (Calibrated Bolts) Franklin 3 & 4 lb/ft Posts - 60 ksi Marion 3 & 4 lb/ft Posts - 80 ksi Back to Back and Nested Splices Critical Configuration	

<u>Section</u>	ge
Appendix R: Torsion Tests	(-1
Back to Back and Nested Splices Post W/O Splice	
Appendix S: 75" Combined Bending and Torsion Tests S	;-1
Base Stress vs Relative Bolt Tension (Calibrated Bolts) Franklin 3 & 4 lb/ft Posts – 60 ksi Marion 3 & 4 lb/ft Posts – 80 ksi Back to Back and Nested Splices Critical Configuration	
Appendix T: Final 71" Bending Tests With Predicted Bolt Tension Envelopes (Calibrated Bolts)	- 1
Base Stress vs Relative Bolt Tension (Absolute) Franklin 3 & 4 lb/ft Posts – 60 ksi Marion 3 & 4 lb/ft Posts – 80 ksi Back to Back and Nested Splices Critical Configuration	
Appendix U: Bolt Calibration Curves	J-1
Applied Tension vs Milli-Volts (From Wheatstone Bridge) 3/8 Inch Grade 9 Bolts 1-3/4 and 4-1/2 Inch Bolt Lengths TML Bolt Strain Gauges MM AE-10 and TML Epoxies	
Appendix V: Physical and Chemical Test Results V Franklin 4 lb/ft Posts - 60 ksi	<i>l</i> - 1
Appendix W: Physical and Chemical Test Results	V-1
Appendix X: Results of Pullout Tests	(-1

TABLE OF CONTENTS PHASE III

																												<u>Page</u>
	ACKN	IOWLE	DGMEN	STF			•	•			• .	•	•	•	•		•	•	•	•	•	•	•	•	•	•	•	ii
	PREF	ACE			•					•	•	•	•	•														iii
I.	INTE	RODUC	CTION	AND	OB-	JEC	TI	/ES			• ,		•	•		•			•									1
11.	STU	Y AF	PROAG	CH .							•		•				•							•				2
111.	GUIC	DELIN	NES FO	OR U	PGR	ADI	NG	•														•	•			•	•	5
	Α.	Exis	sting	Sys	tem	s N	ot	in	С	om	pl:	i a	nc	e '	wi	th	S	af	et	y	St	an	ıda	ırc	İs			5
	B. C.	Upgı B/C	rading Anal	g Al ysis	ter	nat 	ive •	es •			•	•	•			•	•		•	•	•	•	•	•	•		•	5 8
		1.	U-Po: P2 Po	sts			•	•			•	•	•															10 13
	D.	Exar	nples													•										•		17
		1.	Exam Exam																									17 21
	E.	Disc	cussi	on o	f R	esu	ılt	s																				21
IV.	GUI	DELII	NES F	OR N	EW	INS	TA	LLA	ΙT	ON	S								•		•			•				28
٧.	CON	CLUS	IONS						•					•				•			•							33
	REF	EREN	CES .						•		•	•			•			•										34
	APP	ENDI	CES .																									
	A. B. C. D.	RET RET	IMATI ROFIT ROFIT DELIN	GUI GUI	DEL DEL	INI.	S S	FOF	R I	J-P 22	OS PO	TS ST	S	•	•	•										•	•	
	υ.	au I	ひににより	LJ F	VΛ	ILL	7 L	113	ıΛl	ニレハ	11	UII	J	•	•		•	•	•	•	•	•	•	•	•	•	•	74

LIST OF FIGURES PHASE III

		Page
1.	SIGN PARAMETER DEFINITIONS	9
2.	RETROFIT GUIDELINES	11
3.	P2 POST DETAILS	14
4.	SLIP-BASE RETROFIT FOR P2 POST	16
5.	U-POST RETROFIT GUIDELINES, EXAMPLE 1-1	18
6.	U-POST RETROFIT GUIDELINES, EXAMPLE 1-2	19
7.	U-POST RETROFIT GUIDELINES, EXAMPLE 1-3	20
8.	P2 POST RETROFIT GUIDELINES, EXAMPLE 2-1	22
9.	P2 POST RETROFIT GUIDELINES, EXAMPLE 2-2	23
10.	P2 POST RETROFIT GUIDELINES, EXAMPLE 2-3	24
	NEW INSTALLATION GUIDELINES	
	SEVERITY INDEX VERSUS OCCUPANT IMPACT VELOCITY	
B1.	U-POST RETROFIT GUIDELINES, PART A	51
	U-POST RETROFIT GUIDELINES, PART B	
	U-POST RETROFIT GUIDELINES, PART C	
	U-POST RETROFIT GUIDELINES, PART D	
	U-POST RETROFIT GUIDELINES, PART E	
	U-POST RETROFIT GUIDELINES, PART F	56
	U-POST RETROFIT GUIDELINES, PART G	
	U-POST RETROFIT GUIDELINES, PART H	
	U-POST RETROFIT GUIDELINES, PART I	
	U-POST RETROFIT GUIDELINES, PART J	
	U-POST RETROFIT GUIDELINES, PART K	
	U-POST RETROFIT GUIDELINES, PART L	
BIJ.	U-POST RETROFIT GUIDELINES, PART M	63

LIST OF FIGURES PHASE III (continued)

																						<u>Page</u>
B14.	U-F	POST 1	RETROFIT	GUIDELIN	ES,	PART	N.	•	•	•	•		•				•		•			64
B15.	U-f	POST I	RETROFIT	GUIDELIN	ES,	PART	0.			•	•		•	•	•		•	•			•	65
C1.	P2	POST	RETROFIT	GUIDELII	NES,	PART	A	•	•	•	•		•		•	•	•				•	67
C2.	P2	POST	RETROFIT	GUIDELII	NES,	PART	В	•	•	•	•	•	•		•	•	•	•		•	•	68
C3.	P2	POST	RETROFI	GUIDELII	NES,	PART	C	•	•	•	•		•	•					•	•	•	69
C4.	P2	POST	RETROFIT	GUIDELI	NES,	PART	D	•		•		•	•	•						•	•	70
C5.	P2	POST	RETROFI	GUIDELII	NES,	PART	E	•	•	•	•	•	•	•		•		•	•	•	•	71
C6.	P2	POST	RETROFI	GUIDELII	NES,	PART	F	•					•			•			•		•	72
C7.	P2	POST	RETROFIT	GUIDELI	NES,	PART	G		•	•	•	•	•		•		•	•		•	•	73
C8.	P2	POST	RETROFIT	GUIDELII	NES,	PART	H	•	•		•		•	•				•	•	•	•	74
С9.	P2	POST	RETROFI	GUIDELII	NES,	PART	1	•	•		•		•	•			•		•		•	75
C10.	P2	POST	RETROFI	GUIDELII	NES,	PART	J				•		•			•	•	•	•		•	76
C11.	P2	POST	RETROFIT	GUIDELII	NES,	PART	K	•	•		•		•			•		•			•	77
C12.	P2	POST	RETROFI	GUIDELII	NES,	PART	L			•	•	•	•	•	•	•			•	•	•	78
C13.	P2	POST	RETROFI	GUIDELII	NES,	PART	M	•	•	•		•	•	•	•	•	•	•	•	•	•	79
C14.	P2	POST	RETROFI	r GUIDELII	NES,	PART	N	•	•	•		•	•	•	•	•	•		•	•		80
C15.	P2	POST	RETROFI	r GUIDELII	NES,	PART	0	•	•	•	•	•	•	•		•	•	•	•		•	81
C16.	P2	POST	RETROFI	r GUIDELII	NES,	PART	P		•	•		•	•	•	•	•				•	•	82
C17.	P2	POST	RETROFI	r GUIDELII	NES,	PART	Q		•	•	•	•	•	•		•	•			•	•	83
C18.	P2	POST	RETROFI	r GUIDELI	NES,	PART	R	•	•		•	•	•		•		•	•		•		84
C19.	P2	POST	RETROFI	r GUIDELI	NES,	PART	S	•			•		•	•	•	•	•	•		•		85
C20.	P2	POST	RETROFI	r GUIDELI	NES,	PART	T	•	•	•	•		•	•	•	•	•	•	•	•	•	86
C21.	P2	POST	RETROFI	F GUIDELI	NES,	PART	U	•	•		•	•	•	•	•	•		•	•	•		87
C22.	P2	POST	RETROFI	GUIDELI	NES,	PART	V			•			•		•		•	•		•	•	88
C23.	P2	POST	RETROFI	r GUIDELI	NES,	PART	W															89

LIST OF FIGURES PHASE III (continued)

																				<u>Page</u>
C24.	P2	POST	RETROFIT	GUIDELINES,	PART	X	•	•			•	•	•				•	•		90
C25.	P2	POST	RETROFIT	GUIDELINES,	PART	Y	•	•				•		•		•		•		91
C26.	P2	POST	RETROFIT	GUIDELINES,	PART	Z			•	•		•	•	•	•	•	•			92
C27.	P2	POST	RETROFIT	GUIDELINES,	PART	AA		•				•	•			•				93
D1.	NEV	INS	TALLATION	GUIDELINES,	PART	Α.						•		•					•	95
D2.	NEV	INS	TALLATION	GUIDELINES,	PART	В		•	•			•	•							96
D3.	NEV	INS	TALLATION	GUIDELINES,	PART	С														97

LIST OF TABLES PHASE III

		<u>Page</u>
1.	SEVERITY INDEX SCALE	3
2.	PROPERTIES OF POSTS	6
3.	TENTATIVE RETROFIT GUIDELINES	25
4.	ESTIMATED REPLACEMENT COSTS	26
5.	NEW INSTALLATION GUIDELINES	32
6.	ESTIMATE INCREMENTAL COST	32
A1.	VEHICULAR VELOCITY CHANGE FOR IMPACTS WITH EXISTING ADOT SYSTEMS	38
A2.	VEHICULAR VELOCITY CHANGE FOR IMPACTS WITH ALTERNATE SYSTEMS DEVELOPED BY ADOT	. 39
A3.	SEVERITY INDICES FOR EXISTING ADOT SYSTEMS BASED ON CRASH TESTS	42
A4.	SEVERITY INDICES OF ALTERNATE SYSTEMS DEVELOPED BY ADOT	42
A5.	SEVERITY INDICES FOR EXISTING ADOT SINGLE POST INSTALLATIONS	44
A6.	SEVERITY INDICES FOR ALTERNATE SINGLE POST INSTALLATIONS	45
A7.	SEVERITY INDICES FOR EXISTING ADOT MULTIPLE POST INSTALLATION	46
A8.	SEVERITY INDICES FOR ALTERNATE MULTIPLE POST INSTALLATIONS	. 47

 1 in = 2.64 (exactly). For other exact conversions and more detailed tables, see NBS Misc. Publ. 286, Units of Weights and Measures, Price \$2.26, SD Catalog No. C13.10:286.

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I. INTRODUCTION AND OBJECTIVES

This report is the culmination of a three part study on small sign supports. In phase I (1), widely used small sign installations by the Arizona Department of Transportation (ADOT), were crash tested and evaluated according to nationally recognized safety standards (2,3). Phase II (4) of the study focused on developing and testing improved sign support designs.

The third and final phase of this study, described herein, addressed the economics of small sign supports. The objectives were as follows:

- Develop guidelines that identify conditions for which an existing sign installation should be replaced with an improved (i.e. safer) system.
- (2) Evaluate candidate systems for new installations in terms of their cost effectiveness.

II. STUDY APPROACH

A benefit/cost (B/C) program, developed at TTI, was used to accomplish the study objectives. Details of the program are documented in the literature $(\underline{5},\underline{6})$ and will not be given here. Basically, the program estimates the benefits and costs of one safety alternative with respect to another. The B/C ratio is computed as follows:

Benefits are measured in terms of reductions in accident costs. A key element of the program is an encroachment-probability model used to estimate the frequency a given roadside feature will be struck by errant vehicles. Factors such as traffic volume (ADT), vehicle mix, operating speeds, the size and location of the roadside feature, and roadway alignment can be included in the analysis.

For each accident predicted to occur with a given roadside feature (e.g., a roadside sign) the program computes an accident's cost. Accident costs are related to impact severity through a "severity index" (SI). Table 1 shows the relationship between SI, the implied severity probability, and net accident costs that would result. The scale is given in the AASHTO Barrier Guide (1). Accident costs have been updated to reflect current estimates (8). Interpretation of the values in Table 1 are illustrated as follows. For an accident having an SI of 5.0, there is a 30 percent probability it will be a "property damage only" accident, a 65 percent probability it will be an accident with injuries, and a 5 percent probability it will be an accident with fatalities.

In general, the SI depends on the impact speed, impact angle, vehicle size, and the type of feature struck. The methodologies and assumptions used to estimate the SI for impacts with existing and candidate ADOT sign support systems are given in Appendix A.

The following are basic assumptions made in the application of the B/C model. Limitations relate primarily to input data.

- 1. Average Daily Traffic (ADT) for a highway design increases at an annual rate of 3% for a design life of 20 years.
- 2. The speed limit is 55 miles per hour.

TABLE 1. SEVERITY INDEX SCALE

Severity <u>Index</u>	% Property Damage Only Accidents	%Injury Accidents	Percent Fatal Accidents	Societal Cost <u>Per Accident</u>
0	100	0	0	\$ 1,960
1	85	15	0	\$ 4,230
2	70	30	0	\$ 6,750
3	55	45	0	\$ 9,200
4	40	59	1	\$ 19,400
5	30	65	5	\$ 52,000
6	20	68	12	\$107,800
7	10	60	30	\$203,000
8	0	40	60	\$482,000
9	0	21	79	\$629,000
10	0	5	95	\$753,000

- 3. Service life for all alternatives is 20 years with an annual interest rate of 4%.
- 4. The relationship between severity indices and accident costs are as given in Table 1. Severity indices associated with the various sign support systems evaluated are discussed in Appendix A.
- 5. No adjustment is made for roadway alignment (roadway assumed to be straight).
- 6. Traffic delay cost associated with sign accidents is negligible.
- Maintenance costs of all sign support systems considered are similar and therefore can be neglected.
- 8. Impact repair costs of all sign support systems considered are similar and therefore can be neglected.
- 9. Salvage value of all sign support systems considered is negligible.

It should be noted from item 2 that the assumed speed limit for the benefit/cost analysis was 55 mph. Many states, including Arizona, have recently adopted a higher limit of 65 mph for rural Interstate highways. The researchers do not anticipate a change in the guidelines developed herein due to this increased speed limit. Although there my conceivably be a slight increase in encroachment frequency due to higher speeds, there is not enough data currently available to quantify this change or to determine whether or not it is even significant. Furthermore, as discussed in Appendix A, crash tests have indicated that the severity associated with impacting small sign supports is independent of impact speed.

III. GUIDELINES FOR UPGRADING

A. Existing Systems Not in Compliance With Safety Standards

A review of Table A5, Appendix A, and data in reference 1 shows that all single post systems now used by ADOT are in compliance with current safety performance standards (2,3). From Table A7, Appendix A, and data in reference 1, it can be seen that the following systems are not in compliance:

- (A) Installations with two or more square steel tubes (ADOT P2 Systems) within a 7 ft spacing, and
- (B) All installations with three or more 3 lb/ft high carbon steel U-posts within a 7 ft spacing.

From Table A7, Appendix A, it can be seen that the two post, 3 lb/ft U-post system with the "short lap splice" is "marginally acceptable" since the estimated occupant impact velocity (15.5 ft/sec) is slightly greater than the recommended limit (15 ft/sec). However, preliminary B/C analysis of this system clearly showed that it would not be cost beneficial to replace existing installations.

B. Upgrading Alternatives

In the second phase of this contract, an alternate steel U-post support system was developed ($\underline{4}$). It employs a short ground splice with the high carbon steel U-posts. Based on impact performance standards, up to three 3 lb/ft posts and two 4 lb/ft posts can be used within a 7 ft spacing for a given installation. Also developed in Phase II was a candidate for upgrading existing ADOT P2 systems ($\underline{4}$). It consists of standard square steel tubes mounted on multi-directional slip bases. Up to four of these posts may be used within a 7 ft spacing for a given P2 system. Performance data for these systems are given in Tables A6 and A8, Appendix A.

Other systems were also considered as candidates to upgrade existing systems. These included standard steel pipe with a uni-directional slip base, aluminum tube, solid wood posts, and hollow rectangular sections of laminated veneer lumber.

Cross-sectional properties of the candidate systems are given in Table 2. Parameters given in the table are second moment of area, I, section modulus, S, tensile yield stress, Fy, and yield moment, My. The yield moment is a direct measure of the load carrying capacity of the post.

TABLE 2. PROPERTIES OF POSTS

Post Type	Size	I (in+)	S (in 3)	Fy (ksi)	S (in 3) Fy (ksi) My (k-in)
Steel U-Post	3 Lb/Ft 4 Lb/Ft	0.415	0.419	80	33.5
Steel Pipe	3" Diam. 4" Diam. 5" Diam.	3.02 7.23 15.2	1.72 3.21 5.45	36	61.9 115.6 196.2
Solid Wood	4" x 4" 4" x 6"	21.3	10.7	1.4	14.9
Aluminum Tube	4" x 0.188"	4.08	2.04	35	71.4
Hollow Rectangular Laminated Veneer Lumber	7.875" x 7.875" x 1.25" 7.875" x 14.875" x 1.25"*	251.0 1110.3 b	63.7 149.3 ^b	2. 2. 5. 5.	159.3 373.3 b

a Post weakened at base by saw cuts (13)

b Properties through weakened section

After discussions with ADOT engineers, the list of alternate systems was narrowed down to the following systems:

- o 3 lb/ft High Carbon Steel U-Post with Ground Splice
- o Square Steel Tube (ADOT P2 Post) with Slip Base
- o Steel Pipe Post with Uni-Directional Slip Base
- o Laminated Veneer Wood Post

The U-post and square tube were included because of their relatively low costs for upgrading and the improved safety performance of the ground splice and slip-base designs over existing installations. Also, ADOT has used the U-Post and square tube as sign supports for many years. Their sign crews are obviously familiar with these designs and little, if any, additional installation equipment would be required. The other two candidate systems were selected because of their excellent impact performance as demonstrated through crash tests and their load carrying capacity. Appendix A contains information and references on tests of these systems.

The selection of a steel pipe post with uni-directional slip base was meant to be representative of a wide variety of slip base designs. It was assumed that the performance of any slip base installation with features similar to the ones tested would be essentially the same and, therefore, would have a similar impact severity. In instances where a slip base design is judged to be cost-effective, it is intended for ADOT to use their slip-base design or some other slip-base installation which complies with current AASHTO specifications.

It is noted that at the present time only two post sizes are known to be available for the laminated wood posts as shown in Table 2. Currently, approval of these designs is limited to one support within a 7 ft spacing. The California Department of Transportation uses this system to support relatively large signs in lieu of steel shapes with slip bases. As will be shown later in this report, even the smaller of the two posts does not appear to be cost effective for most small sign installations. However, it is clear that if smaller sizes can be produced, the system has the potential to be a very cost effective small sign support alternative.

The 4 lb/ft U-posts were not considered because of ADOT's concern relative to the handling and installation of the heavier post by sign crews and problems that could arise if both the 3 lb/ft and the 4 lb/ft posts were used. Further, only two 4 lb/ft posts are permitted within a 7 ft spacing

while three 3 lb/ft posts are permitted. The three post, 3 lb/ft system will carry approximately 12 percent more load than the two, 4 lb/ft system.

Aluminum pipe was ruled out because of strength and cost factors. Most existing ADOT installations that are candidates for upgrading (i.e., those that do not meet current safety standards) involve those with three or more U-posts within an 8 ft spacing. As such, one $4" \times 0.188"$ aluminum pipe is not equivalent in load carrying capacity to three 3 lb/ft U-posts. Hence, a multiple-post aluminum system would be required. Based on crash test results (9), it is apparent that two $4" \times 3/16"$ aluminum pipe supports within a 7 ft spacing would not meet safety standards. Two smaller aluminum pipes (say two $3 \times 1/2" \times 3/16"$) would satisfy the strength requirements, but the impact performance of such a system is not presently known.

The solid wood post was not considered because of its relatively low load carrying capacity and its impact performance. Based on previous tests ($\underline{10}$), it was concluded that at most, only two 4" x 4" posts could be used within a 7 ft spacing and only one 4" x 6" could be used for compliance with current AASHTO safety standards ($\underline{2}$).

C. B/C Analysis

As discussed in Section III-A, existing installations with two or more posts in the P2 system and existing installations with three or more 3 lb/ft U-posts are candidates for upgrading. The three systems considered as upgrading alternatives were described in Section III-B.

The need to upgrade a given installation depends in part on its offset or lateral distance from the travelway and the number and spacing of the support posts. The frequency of vehicular impacts will increase as the offset decreases. The severity of impacts will increase as the post spacing decreases. The offset distance and post spacing parameters are given in Figure 1. ADOT engineers indicated that most of their small signs have offsets between 12 ft and 20 ft. The guidelines were therefore developed for three offsets, namely 12 ft, 16 ft, and 20 ft. Post spacing of most ADOT signs is greater than 2 ft. A post spacing of 2 ft was used in the B/C analysis. As such, the analysis will generally overstate the severity of multiple-post installations.

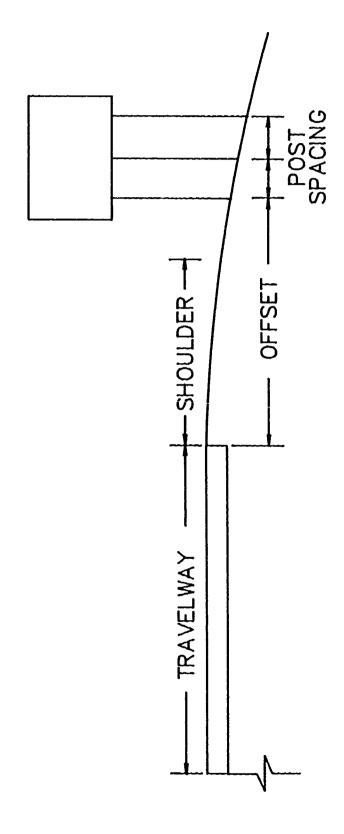


FIGURE 1. SIGN PARAMETER DEFINITIONS

1. U-Posts

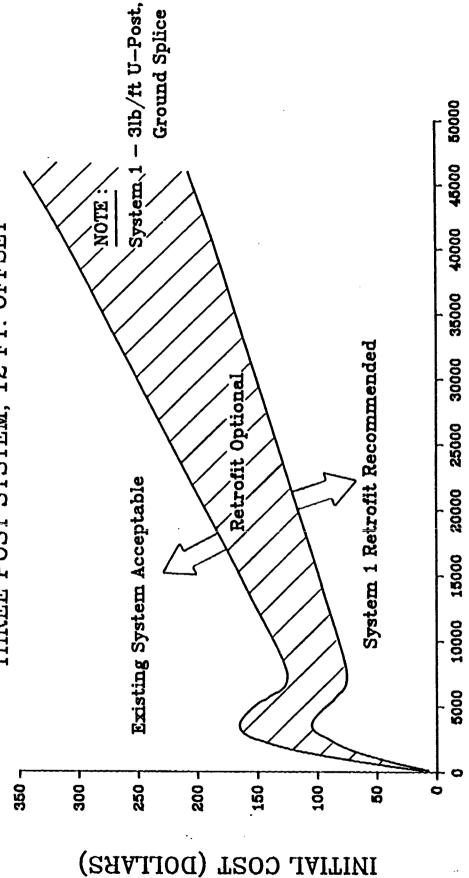
Shown in Figure 2 is a typical set of quidelines developed using the B/C program previously discussed. The shape of the B/C curve shown in Figure 2 is directly related to the encroachment data used in the B/C model. The encroachment data used for the study was collected by Cooper (11) and is the best and most extensive data base currently available. The transition at the lower end of the graph is due to a change in driver attention when going from a very low volume traffic condition to a condition of more steady or moderate traffic flow. On very low volume roadways driver attention is low, thus resulting in a relative increase in the number of run-off-road incidents. As the traffic flow increases to a more steady level, driver attention and awareness is increased due to the surrounding traffic, and the encroachment per vehicle mile of travel declines significantly. As the ADT continues to increase, howver, the effects of driver attention are negated by the sheer number of vehicles on the roadway, and the number of encroachments again begin to rise. Similar behavior is evident in other encroachment studies as well (12).

Specifically, the guidelines shown in Figure 2 are for upgrading an existing three-post, 3 lb/ft U-post "long lap splice" system with a three-post, 3 lb/ft U-post "ground splice" system (system 1). The guidelines are for a 12 ft offset. The B/C program was used to construct these curves as follows:

- (1) The severity indices for impacts with the existing and retrofit systems were obtained as described in Appendix A. It will be noted that ADOT uses one of three types of splices in the U-post system. From data developed in Appendix A, it can be seen that there are no major differences in the severity indices of the three systems. Values for the "long lap splice" system are nominal for the three systems and were therefore used to represent all existing ADOT U-post systems.
- (2) An initial cost for the retrofit system and an ADT were input to the B/C program. For these conditions together with the other impact parameters previously described, a B/C ratio was determined.
- (3) Step 2 was repeated as the initial cost was incremented until a value of initial cost was obtained that produced a B/C of one.
- (4) Steps 2 and 3 were repeated as the ADT was incremented until enough values were obtained to plot a curve whose coordinates represent initial

RETROFIT GUIDELINES

3 LB/FT U-POST - LONG LAP SPLICE THREE POST SYSTEM, 12 FT. OFFSET



AVERAGE DAILY TRAFFIC

RETROFIT GUIDELINES

FIGURE 2.

cost and ADT values for which the B/C is one.

(5) Due to the probabilistic nature of the B/C program and the assumptions inherent therein, a certain degree of judgment should be used in the use of the guidelines. As shown in Figure 2, there is an optional range (indicated by the hashed area) in which judgment should be used. The upper and lower limits of this area were obtained, arbitrarily, by a $\frac{1}{2}$ 5 percent variation in the B/C \approx 1 curve.

Use of the guidelines is made by first determining the initial cost to retrofit. This initial cost is defined as the actual cost to remove the existing installation plus the cost to replace it with the alternate system. In addition to materials and labor, replacement costs should include the purchase of any new equipment required to install the alternate system. Using the initial cost of the equipment and the anticipated number of posts to be installed within its life expectancy, a cost per post can be determined. The cost/post is then multiplied by the number of posts in the system and added to the initial cost. The B/C guidelines were developed in this way so that changing cost data may be reflected in the analysis without outdating the curves.

With the initial cost determined, a point is defined whose coordinates are the ADT of the roadway and the initial cost. If the point lies below the hashed area, the retrofit system is cost beneficial and should be installed if funds permit. If the point is above the hashed area, it is not cost beneficial to replace the installation. If the point lies in the hashed area, replacement is optional. Factors that should be considered in this case include the accident history of this or similar installations and the likelihood of the sign being hit. If there is a record of injury-producing accidents with the installation or if the sign is located in an area where higher than average run-off-the-road accidents occur (such as gore areas or on the outside of sharp curves), retrofitting should be strongly considered. In fact, retrofitting should be strongly considered for any installation for which injury-producing accidents are occurring, regardless of the guidelines.

The complete set of upgrading guidelines for existing three and four support, U-post installations is given in Figures B-1 through B-15, Appendix B. The following should be noted:

(1) System 1 in Figures B1-B3 is three, 3 lb/ft high strength U-posts with the ground splice (system developed in Phase 2 of study (4)). It is

- noted that no more than three posts are permitted within a 7 ft spacing for system 1 to comply with AASHTO safety criteria (2).
- (2) System 2 in Figures B4-B6 and Figures B10-B12 is a single, standard steel pipe with a uni-directional slip base. Note in Figures B4-B6 that a 4 inch pipe is used to replace the three-post system while a 5-inch diameter is needed to replace the four-post system in Figures B10-B12. In the absence of impact data for the 4 inch pipe, its severity index was assumed to equal that of the 5 inch pipe.
- (3) System 3 in Figures B7-B9 and Figures B13-B15 is a single, 7.875-inch by 1.25-inch "Micro-Lam" laminated wood post. Severity indices for this system were not directly available. As given in Table A8, Appendix A, test data for this system are available only for a much larger post (a 7.875 inch x 14.875 inch x 1.25 inch section) weakened by saw cuts at its base to facilitate fracture at impact. It was assumed that the severity index for impacting one unweakened 7.875 inch x 7.875 inch x 1.25 inch post equaled that for impacting one of two weakened 7.875 inch by 14.875 inch by 1.25 inch posts.
- (4) Systems 2 and 3 are single post systems. Since many of the signs that these systems would support would have relatively large horizontal dimensions, special provisions such as wind beams and connection details would be needed. The degree to which these structural details could be cost effectively accommodated would obviously have to be determined. At some point, the added costs and design complexities of a single-post support would dictate that a two-post system be used. In that case, the guidelines would not be applicable unless the post spacing was 7 ft or greater.
- (5) The system composed of square steel tubes with slip bases was not a cost-effective alternative for upgrading the U-post installations. Although the U-post with ground splice and the square steel tube with slip base have similar B/C ratios, the lower replacement cost of the ground splice system made it the more cost-effective alternative.

2. P2 Posts

Upgrading guidelines for the ADOT P2 sign support systems were developed in the same manner as were those for the U-post system. Details of the P2 system are given in Figure 3. Note that a 1.75 inch by 1.75 inch by 0.105

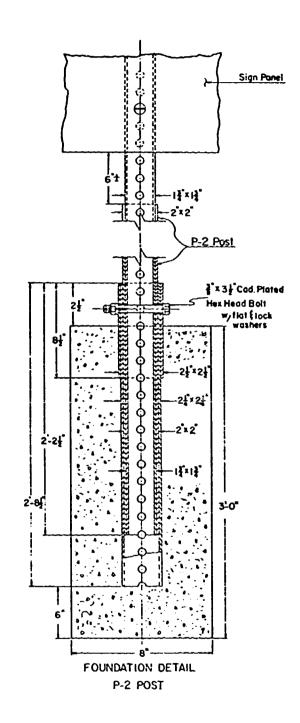


FIGURE 3. P2 POST DETAILS

inch post is nested in a 2 inch by 2 inch by 0.105 inch post. The "effective" section properties of the nested configuration are as follows:

$$I = 0.64 \text{ in}_3^4$$

S = 0.68 in³

The minimum tensile yield stress for these posts is 40 ksi. Hence, the effective moment capacity, My, is 27.2 k-in.

Figures C1 through C24 contain the retrofit guidelines for two, three, and four post P2 systems. Note the following:

- (1) System 1, the 3 lb/ft U-post with ground splice, was not a cost-effective alternative for upgrading the ADOT P2 sign support systems. Use of the U-post installation requires removal and replacement of the existing system, whereas use of the square steel tube (ADOT P2 post) with slip base is simply a retrofit operation. Therefore, since these systems have similar B/C ratios, the lower initial cost of the retrofit system made it the more cost-effective alternative.
- (2) System 2 in Figures C4-C6, C13-C15, and C22-C24 is a single, standard steel pipe with a uni-directional slip base.
- (3) System 3 in Figures C7-C9, C16-C18, and C25-C27 is a single, 7.875 inch by 7.875 inch by 1.25 inch laminated wood post.
- (4) System 4 in Figures C1-C3, C10-C12, and C19-C21 is composed of square steel tubes (ADOT P2 posts) mounted on multi-directional slip bases. This system was developed in Phase 2 of this study (4), and details of the retrofit are shown in Figure 4. Retrofitting an existing ADOT P2 sign support system simply involves cutting the existing post and bolting both ends to the pre-fabricated slip base mechanism. Based on test results (4) and analysis in Appendix A, it is noted that up to four posts are permitted with a 7 ft spacing for this system to comply with AASHTO safety criteria (2). It is further noted that the stub height of this system is below the recommended value set forth in the AASHTO specification (2).

Guidelines for two-post systems are found in Figures C1-C3, three-post systems are shown in Figures C10-C12, and four-post systems are presented in Figures C19-C21.

(5) See note 4 in previous section regarding systems 2 and 3.

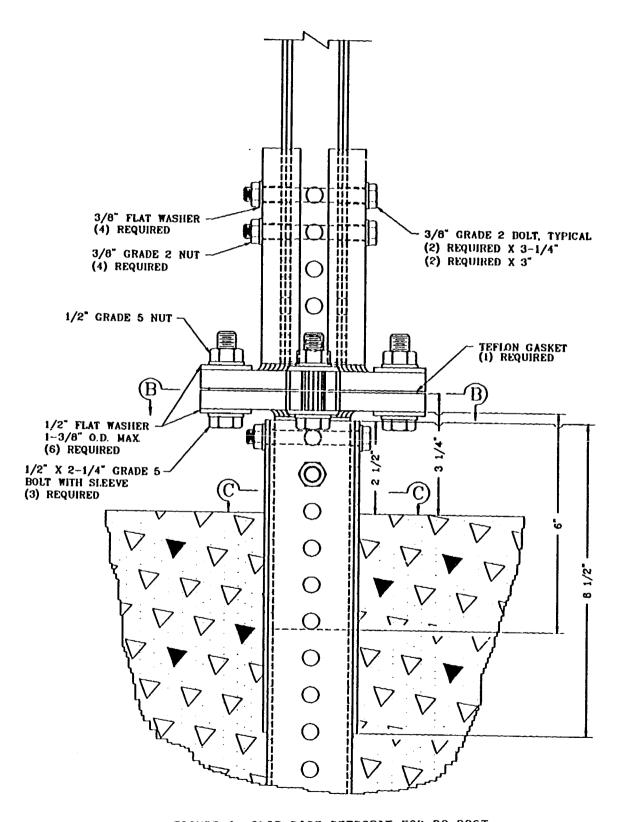


FIGURE 4. SLIP-BASE RETROFIT FOR P2 POST

D. Examples

The following examples are included to illustrate use of the upgrading guidelines. Although costs used in the examples are estimates, they are believed to be "in the ballpark." As such, tentative conclusions can be drawn from results of these examples.

Example 1

Given: - An existing three-post, 3 1b/ft U-post installation

- Offset = 12 ft

- ADT = 10,000

Replacement costs (initial costs) for following systems:
 (1) three-post, 3 lb/ft U-post with ground splice - \$200

(2) one 4" diameter pipe with slip base - \$400

(3) one 7.875" x 7.875" x 1.25" laminated wood post - \$300

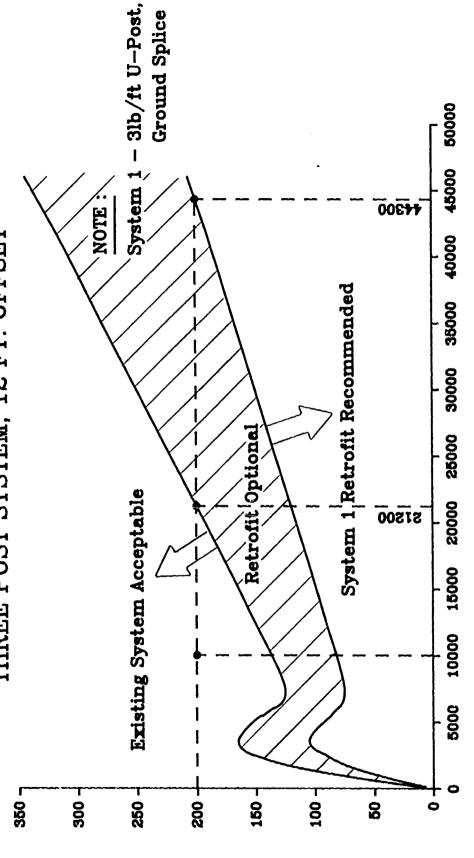
Required: Determine if the existing installation should be replaced by any of the above systems.

- Solution: (1) From Figure 5 it can be seen that replacement by system 1 is not warranted since the point representing the combination of ADT and cost lies above the hashed area. Further, replacement by system 1 is not warranted for ADT's up to 21,200. For ADT's between 21,200 and 44,300, the replacement is optional and for ADT's above 44,300, replacement is recommended.
 - (2) From Figure 6 it can be seen that replacement by system 2 is not warranted since the point representing the combination of ADT and cost lies above the hashed area. Further, replacement by system 2 is not warranted for ADT's up to 21,300. For ADT's between 21,300 and 44,500, replacement is optional and for ADT's above 44,500, replacement is recommended.
 - (3) From Figure 7 it can be seen that replacement by system 3 is not warranted since the point representing the combination of ADT and cost lies above the hashed area. Further, replacement by system 3 is warranted for ADT's up to 20,000. For ADT's between 20,000 and 42,600, replacement is optional and for ADT's above 42,600, replacement is recommended.

A review of the guidelines for larger offsets shows that if replacement is not warranted for a 12 ft offset, replacement will not be warranted for larger offsets.

RETROFIT GUIDELINES

3 LB/FT U-POST - LONG LAP SPLICE THREE POST SYSTEM, 12 FT. OFFSET



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U-POST RETROFIT GUIDELINES, EXAMPLE 1-1

5.

FIGURE

COST (DOLLARS)

System 2- 4" Diam. Steel Pipe Slip Base 50000 45000 00977 10000 15000 20000 25000 30000 35000 40000 THREE POST SYSTEM, 12 FT. OFFSET NOTE 3 Lb/Ft U-POST-LONG LAP SPLICE System 2'Retrofit Recommended Retrofit Optional Existing System Acceptable s7300 5000 7007 0 500 600 -4004 1001 300 2002 (DOLLARS) COZL **JAITINI**

RETROFIT GUIDELINES

AVERAGE DAILY TRAFFIC FIGURE 6. U-POST RETROFIT GUIDELINES, EXAMPLE 1-2

System 3-8" x 8" Micro-Lam Laminated Timber 40000 45000 NOTE 42600 THREE POST SYSTEM, 12 FT. OFFSET 10000 15000 20000 25000 30000 35000 System 3 Retrofit Recommended Retrofit Optional Existing System Acceptable 5000 800 J 400 600 300 200 100 ò INITIAL COST

3 Lb/Ft U-POST-LONG LAP SPLICE

RETROFIT GUIDELINES

GURE 7. U-POST RETROFIT GUIDELINES, EXAMPLE 1-3

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Example 2

Given: - An existing three-post P2 installation

- Offset = 12 ft - ADT = 10.000

- Initial costs for following systems

- (1) System 4: three-post, square tube with slip base \$150
- (2) System 2: one 4" diameter pipe with slip base \$400 (3) System 3: one 7.875" x 7.875" x 1.25" laminated post -\$300

Required: Same as Example 1.

- Solution: (1) From Figure 8 it can be seen that replacement by system 4 is optional for the given conditions. Except for relatively low ADT's, system 4 is optional for ADT's up to 14,900. It is recommended that the existing system be replaced for ADT's above 14,900.
 - (2) From Figure 9 it can be seen that replacement by system 2 is probably not warranted for the given condition. However, replacement by system 2 appears to be optional for most ADT's up to 26,800. Beyond this point, system 2 is recommended.
 - (3) From Figure 10 it can be seen that replacement by system 3 is optional. Except for relatively low ADT's, system 3 is optional for ADT's up to 21,900. It is recommended that the existing system be replaced for ADT's above 21,900.

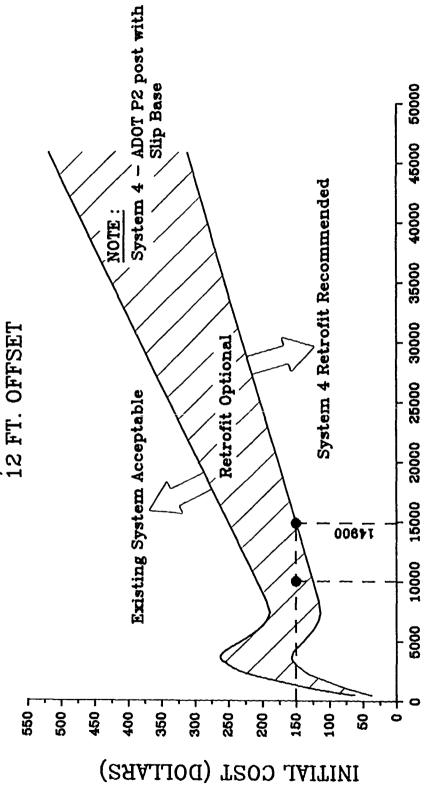
E. Discussion of Results

Shown in Table 3 is a set of tentative guidelines for retrofitting existing ADOT small sign installations. They are based on the criteria in Appendices B and C, the aforementioned assumptions, and estimated costs to replace or retrofit the existing systems. Estimated replacement costs for the three retrofit systems are listed in Table 4. These findings must be viewed as preliminary in nature, subject to change as more definitive data are developed by ADOT on costs and design alternatives.

From the criteria of Table 3, certain findings are noted as follows:

<u>U-Posts</u> - Retrofit of three-post installations is not necessary for ADT's approximately 20,000 or less. Retrofit of four-post installations with offsets of 12 ft or less is optional for ADT's up to 25,000. It is important to note that the guidelines are based on an assumed post spacing of 2 ft. If the spacing of a four-post installation exceeds approximately 2.5 ft, the three-post guidelines apply. It should also be noted that only systems 2 and

ADOT P2, THREE POST SYSTEM 12 FT. OFFSET



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P2 POST RETROFIT GUIDELINES, EXAMPLE 2-1

FIGURE 8.

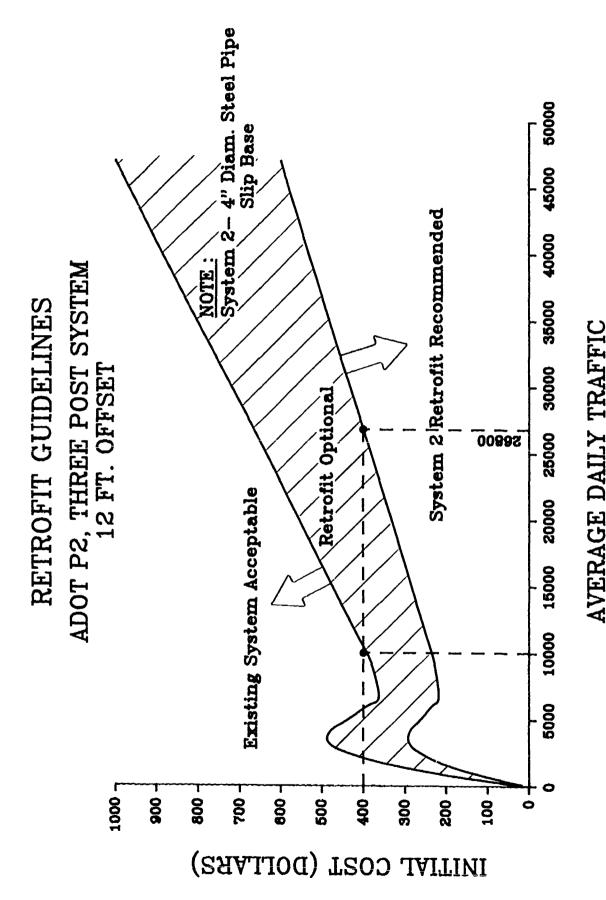
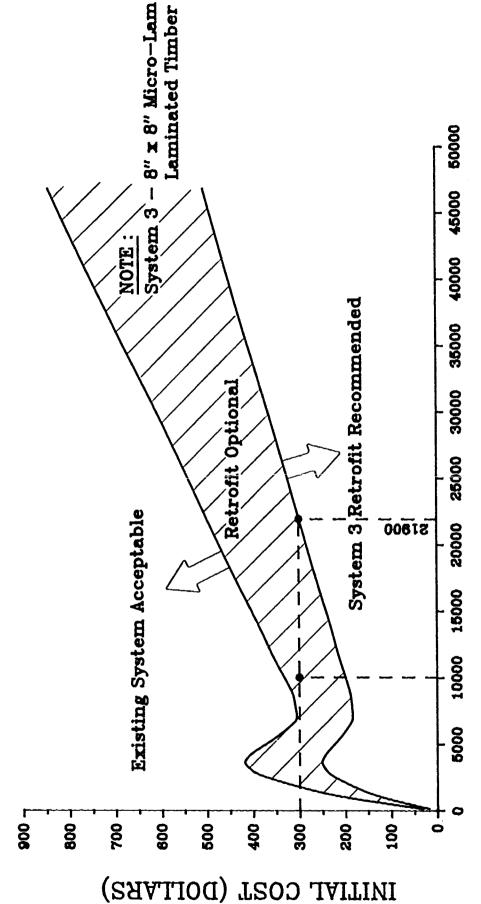


FIGURE 9. P2 POST RETROFIT GUIDELINES, EXAMPLE 2-2

ADOT P2, THREE POST SYSTEM 12 FT. OFFSET



AVERAGE DAILY TRAFFIC

P2 POST RETROFIT GUIDELINES, EXAMPLE 2-3

FIGURE 10.

TABLE 3. TENTATIVE RETROFIT GUIDELINES

SUGGESTED RETROFIT SYSTEM	Three 3 lb/ft U-Post with Ground Splice Three 3 lb/ft U-Post with Ground Splice Three 3 lb/ft U-Post with Ground Splice	Steel Pipe with Slip Base or Laminated Wood Post Steel Pipe with Slip Base or Laminated Wood Post Steel Pipe with Slip Base or Laminated Wood Post	Two Square Steel Tubes (ADOT P2) with Slip Base Two Square Steel Tubes (ADOT P2) with Slip Base Two Square Steel Tubes (ADOT P2) with Slip Base	Three Square Steel Tubes (ADOT P2) with Slip Base Three Square Steel Tubes (ADOT P2) with Slip Base Three Square Steel Tubes (ADOT P2) with Slip Base	Four Square Steel Tubes (ADOT P2) with Slip Base Four Square Steel Tubes (ADOT P2) with Slip Base Four Square Steel Tubes (ADOT P2) with Slip Base	Steel Pipe with Slip Base or Laminated Wood Post Steel Pipe with Slip Base or Laminated Wood Post Steel Pipe with Slip Base or Laminated Wood Post
ING: RETROFIT RECOMMENDED	> 45,000	> 25,000 > 30,000 > 40,000	> 21,000 > 30,000 > 41,000	> 14,500 > 22,300 > 31,000	> 1,000 > 1,300 > 1,500	> 1,000 > 1,500 > 2,000
RANGE FOR FOLLOWING RETROFIT OPTIONAL RI	10,000-45,000 >30,000 >43,000	0-25,000 12,000-30,000 20,000-40,000	1,700-21,000 2,300-30,000 19,000-41,000	1,300-14,500 1,800-22,300 13,000-31,000	0-1,000 0-1,300 0-1,500	0-1,000 0-1,500 0-2,000
ADT RETROFIT NOT NECESSARY	0-20,000 0-30,000 0-43,000	0-20,000	0- 1,700 0- 2,300 0-19,000	0- 1,300 0- 1,800 0-13,000		1 1 1
OFFSET (FT)	12 16 20	12 16 20	12 16 20	12 16 20	12 16 20	112 16 20
NO. OF POSTS	ო	4	2	m	4	4
EXISTING POST TYPE	U-Post		PS Post			

TABLE 4. ESTIMATED REPLACEMENT COSTS

RETROFIT SYSTEM		NO. OF POSTS	ESTIMATED REPLACEMENT COST (\$)
1. Steel U-Post with Ground Splice	3 1b/ft 3 1b/ft	2 3	150 200
2. Square Steel Tube with Slip Base	ADOT P2 ADOT P2 ADOT P2	2 3 4	115 150 190
3. Standard Steel Pipe with Slip Base	3 in. dia. 4 in. dia. 5 in. dia.	1 1	350 400 450
4. Hollow, Square Laminated Lumber Post	7.875 in x 7.875 in x 1.25 in.	1	300

3 are retrofit designs for four-post installations. Reference should be made to note 4 at the end of Section III-C-1 for potential limitations of these systems.

<u>P2 Posts</u> - Retrofit of two and three-post systems is not necessary for low volume roads (i.e. ADT's of approx. 2000 or less). Retrofit of two-post installations with offsets of 12 ft or less is optional for ADT's of 14,850 or less. Retrofit of three-post installation (for all posts within a 7 ft spacing) is recommended for all but low volume roads, i.e., ADT's of 2000 or less.

IV. GUIDELINES FOR NEW INSTALLATIONS

The purpose of this phase of the study was to evaluate candidate designs for new installations and to develop tentative selection guidelines. With minor exceptions, the same B/C procedure used in the development of retrofit guidelines (Chapter III) was used to develop guidelines for new installations. The main difference is that the retrofit guidelines were developed in terms of the cost to replace an existing system while the new installation guidelines were developed in terms of the incremental cost between the candidate designs.

The candidate systems for the new installations are the same systems considered as retrofit alternatives in the previous chapter (see Section III-B). For use with the new installation guidelines, the systems were classified as follows:

- (1) System 1 is three, 3 lb/ft high strength U-posts with ground splice
- (2) System 2 is three, square steel tubes with slip bases
- (3) System 3 is either a single, standard steel pipe with slip base, or a single, 7.875-inch by 7.875-inch by 1.25-inch laminated wood post.

Note that system 3 is either the steel pipe with slip base or the laminated wood post. These installations were grouped together because of their similarities to each other. Both systems are single-support installations with high load carrying capacities and similar B/C ratios. Either system would, therefore, be acceptable for larger signs. The more cost-effective system would be the system with the lower maintenance cost. However, as has been previously discussed, there are inherent design limitations associated with these <u>single</u> post systems (see note 4 at the end of Section III-C-1).

Figure 11 shows the criteria comparing the candidate systems for an offset of 12 ft. These guidelines involve a relative comparison of three acceptable sign support systems. The selection of one system over the others is, therefore, not as critical as for the upgrade guidelines discussed in the previous section. For this reason, the optional range was eliminated and the B/C = 1 curves were plotted directly. Use of the guidelines is accomplished as follows:

(1) Determine material, fabrication, and installation cost of the candidate systems.

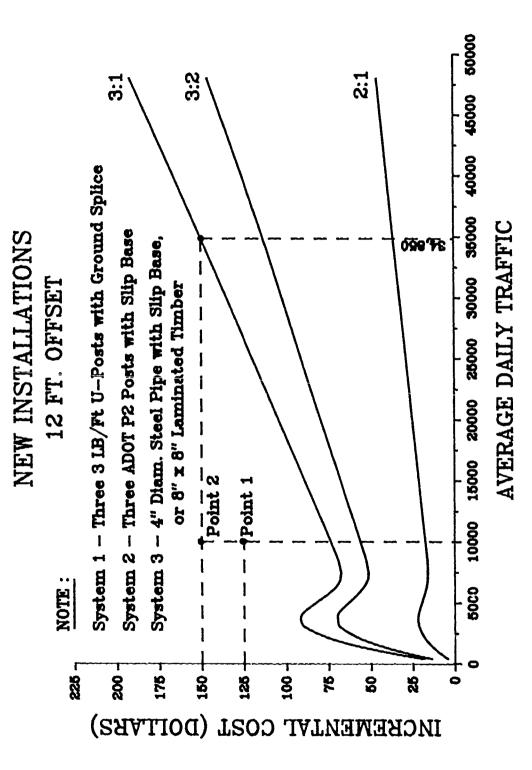


FIGURE 11. NEW INSTALLATION GUIDELINES

- (2) Determine the cost difference between the alternatives (i.e. incremental cost). Denote IC31 as the incremental cost of alternative 3 with respect to 1.
- (3) Determine ADT.
- (4) Plot point 1 whose coordinates are the ADT and IC21.
- (5) Plot point 2 whose coordinates are the ADT and IC31.
- (6) If point 2 lies above the 3:1 curve and point 1 lies above the 2:1 curve, system 1 is cost beneficial.
- (7) If point 2 lies on or below the 3:1 curve and point 1 lies above the 2:1 curve, system 3 is the preferred option.
- (8) If point 2 lies on or below the 3:1 curve and point 1 lies on or below the 2:1 curve, do the following:
 - (a) Determine IC32, the incremental cost of alternative 3 with respect to 2.
 - (b) Plot point 3 whose coordinates are ADT and IC32.
 - (c) If point 3 lies above the 3:2 curve, alternative 2 is the preferred option.
 - (d) If point 3 lies on or below the 3:2 curve, alternative 3 is the preferred option.

The following example is included to illustrate the use of new installation guidelines. Although costs used in the examples are estimates, they are believed to be "in the ballpark." As such, tentative conclusions can be drawn from the results of this example.

Example

Given: - Offset = 12 ft

- ADT = 10.000

- Installation costs for following systems:

- (1) three-post, 3 lb/ft U-post with ground splice \$125
- (2) three-post, square steel tube with slip base \$250
- (3) single 7.875" x 7.875" x 1.25" laminated wood post \$275

Required: Determine which of the above systems should be installed.

- <u>Solution</u>: (1) Determine incremental costs. I21= 250-125 = 125 I31= 275-125 = 150
 - (2) From Figure 11 it can be seen that system 1 is cost beneficial since points 1 and 2 lie above the 2:1 and 3:1 curves respectively. Further, system 1 is the preferred option for ADT's up to 34,850.
 - (3) From Figure 11 it can be seen that installation of system 2 for new construction is not warranted since point 1 lies above the 2:1 curve for all ADT's below 50,000.

(4) From figure 11 it can be seen that system 3 is cost beneficial for ADT's above 34,850.

The complete set of criteria is given in Figures D1-D3, Appendix D. Shown in Table 5 is a set of tentative guidelines for new installations based on criteria in Appendix D, the aforementioned assumptions made in the B/C analysis, and estimated incremental costs between candidate systems. Estimated incremental costs between candidate systems are given in Table 6. It should be noted that system 1 is three 3 lb/ft U-posts with the ground splice. If more than three U-posts are needed to support a sign and if all the posts are within a 7 ft spacing, systems 2 and 3 are the only options that meet AASHTO safety criteria.

From Table 5 it appears that system 1 is preferred for all but high volume roadways. For signs near the travelway and for high volume, the breakaway supports of system 3 become warranted. These findings must be viewed as preliminary, subject to change as more definitive data are developed by ADOT on costs and design alternatives.

TABLE 5. NEW INSTALLATION GUIDELINES

ADT RANGE FOR FOLLOWING:

OFFSET (FT) SYSTEM 1 SYSTEM 2 SYSTEM 3

12 0-34,850 --- >34,850

16 0-47,000 --- >47,000

20 0-62,000 --- >62,000

TABLE 6. ESTIMATED INCREMENTAL COST

SYSTEM	INSTALLATION COST (\$)	INCREMENTAL COST (\$)
1	125 —	100 (0 to 1)
2	250	100 (2 to 1)
3	275	150 (3 to 1)

V. CONCLUSIONS

Guidelines were developed to assist ADOT in (1) determining if an existing sign installation, not in compliance with current safety standards, should be replaced by a system that does meet the safety standards (see Table 3), and (2) selecting a cost effective sign support for new installations (see Table 5). A benefit/cost analysis procedure was used to develop the guidelines. Results of the study provide the following general conclusions.

A. Retrofit Guidelines for Existing ADOT Sign Supports

- Three 3 lb/ft U-post installations with ADOT splice Most installations do not need to be replaced. For high traffic volumes and for installations in close proximity to the travelway, replacement is optional.
- 2. Four 3 lb/ft U-post installations with ADOT splice Replacement is optional for most installations. Replacement is recommended for high traffic volumes.
- Two and three P2 post installations Replacement is optional for most installations. Replacement is recommended for high traffic volumes.
- 4. <u>Four P2 post installations</u> Replacement of most installations is recommended.

B. New Installation Guidelines

The 3 lb/ft high strength U-post with ground splice system appears to be the most cost effective small sign support currently available. Up to three of these posts can be used within a 7 ft spacing. For signs requiring more than three of these posts within a 7 ft spacing, and for larger signs near the travelway of a roadway with high traffic volumes, a slip base system or the laminated wood post system is recommended.

These findings must be viewed as <u>tentative</u>, subject to change as more definitive data are developed by ADOT on costs and design alternatives. Users of the guidelines should also be cognizant of the assumptions and limitations inherent in the analysis procedures used.

REFERENCES

- 1. Ross, H. E., Jr., et. al., "Small Sign Support Analysis: Phase I Crash Test Program," Research Report 7024-1, Arizona Department of Transportation Study HPR-PL-1(31) Item 202, Texas Transportation Institute, Texas A&M University, January, 1986.
- 2. "Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals," American Association of State Highway and Transportation Officials," 1985.
- 3. Michie, J. D., "Recommended Procedures for the Safety Performance Evaluation of Highway Appurtenances," National Cooperative Highway Research Program Report 230. March, 1981.
- 4. Morgan, J. R., et. al., "Small Sign Support Analysis: Phase II Development of a New Small Sign Support System," Two Volumes, Research Report 7024-2, Arizona Department of Transportation Study HPR-PL-I(31) Item 202, Texas Transportation Institute, Texas A&M University, August, 1987.
- 5. Sicking, D. L., and Ross, H. E., Jr., "Evaluation of Safety Alternatives by Benefit/Cost Analysis," <u>Proceedings</u>, American Society of Civil Engineers Conference on Effectiveness of Highway Safety Improvements, Nashville, Tenn., 1985.
- 6. Sicking, D. L., and Ross, H. E., Jr., "Benefit-Cost Analysis of Roadside Safety Alternatives," <u>Transportation Research Record 1065</u>, Transportation Research Board, 1986.
- 7. "Guide for Selecting, Locating, and Designing Traffic Barriers," American Association of State Highway and Transportation Officials, 1977.
- 8. Rollins, J. B., McFarland, W. F., "Costs of Motor Vehicle Accidents in Texas," Research Report FHWA/TX-85/67+396-1, Texas Transportation Institute, Texas A&M University, College Station, Texas, May, 1985.
- 9. Zimmer, R. A., and Campise, W., "Full-Scale Crash Tests of Small Sign Supports for Florida Department of Transportation," Research Report 0315-1F, Texas Transportation Institute, Texas A&M University, October, 1985.
- 10. Ross, H. E., Jr., et. al., "Crash Tests of Small Sign Supports," Report No. FHWA/RD-80/502. Federal Highway Administration, Washington, D.C., May, 1980.
- Cooper, P., "Analysis of Roadside Encroachments Single Vehicle Run-Off-Road Accident Data Analysis for Five Provinces," B. C. Research, Mar. 1980.

- 12. Hutchinson, John W. and Kennedy, Thomas W., "Medians of Divided Highways Frequency and Nature of Vehicle Encroachments," <u>Engineering Experiment Station Bulletin</u> 487, University of Illinois, June 1966.
- 13. Ross, H. E., Jr., et. al., "Crash Test Evaluation of Small Signs Impacted from the Side," Research Report 0941-1, Texas Transportation Institute, Texas A&M University, College Station, Texas, October, 1979.
- 14. Hirsch, T. J., et. al., "Crash Tests and Evaluation of Single Post Highway Signs," Research Report 146-11, Texas Transportation Institute, Texas A&M University, College Station, Texas, August, 1973.
- 15. Federal Highway Administration Memorandum, subject "Signs Suppor Systems Compliance with 1985 AASHTO Specifications" to Regional FHWA Administrators, dated June 15, 1987.

APPENDIX A. ESTIMATING THE SEVERITY INDEX OF SMALL SIGN SUPPPORTS

The primary measure of severity of vehicular impacts with sign supports is the velocity at which an occupant will impact the interior of the vehicle (3). According to current standards ($\underline{2}$, $\underline{3}$), the occupant impact velocity should not exceed 15 ft/sec.

Estimating Vehicular Velocity Change

The vehicle population is composed of a wide variety of sizes and types, and the B/C program approximates the distribution of vehicle mix. Since impact data on the ADOT small sign supports were determined by tests of small cars only, it was necessary to estimate the impact performance of the sign supports for larger vehicles.

Since the mass of most small sign installations is relatively small in comparison to the impacting vehicle, the velocity change that an impacting vehicle experiences is due in large part to an energy loss (distortions in sign posts, soil displacements and damping, and vehicular crush) as opposed to a momentum transfer. For purposes of estimating velocity change, it was assumed that the energy loss is independent of vehicle size. It is recognized that this assumption will generally result in an overestimation of energy loss in larger vehicles since vehicular crush, and hence the energy loss due to crush, will generally decrease as the size of the impacting vehicle increases. Based on this assumption, the velocity change in larger vehicles is estimated from that measured in the tests of the small vehicles as follows:

$$(\Delta KE)_{T} = 1/2 M_{T}(V_{IT}^{2} - V_{FT}^{2})....(A1)$$

where

 $(\Delta KE)_{T}$ change in kinetic energy of test vehicle

 M_{r} = mass of test vehicle

 V_{ij} = impact velocity of test vehicle

 V_{FT} = final velocity of test vehicle (after post fracture)

Then, for a different size vehicle with mass M_v impacting at V_{II} :

and

$$V_{FV} = \sqrt{V_{IT}^2 - 2(\Delta KE)_T / M_V}$$
(A3)

where

 V_{FV}^{2} final velocity of vehicle

1

The change in velocity is then computed as follows:

$$\Delta V_V = V_{IT} - V_F$$
....(A4)

In the B/C program the vehicle population is assumed to be composed of four different vehicle sizes, the first three being automobiles and small trucks, and the remaining one being heavy vehicles. Each functional class of roadway has associated with it a pre-established vehicle mix. The first three sizes were assumed to collectively represent approximately 76 percent of the population. These three sizes are an 1800 lb vehicle, a 3000 lb vehicle, and a 4500 lb vehicle. Using crash test results (1) and formulas Al through A4, data in Table A1 were developed for these three vehicle sizes for existing ADOT sign supports. Similar data are presented in Table A2 for the alternative systems evaluated in the Phase II study (4). Impacts by heavy vehicles with the small sign supports were assumed to be inconsequential in terms of velocity change with a negligible probability of injury.

For sign impacts, the occupant impact velocity will, in most cases, approximately equal the velocity change of the vehicle during impact. This is borne out by the results given in Tables Al and A2. The predicted change in vehicular velocity for the 3000 lb and the 4500 lb vehicles can therefore be assumed to equal the occupant impact velocity for those vehicles.

Relationship Between Occupant Impact Velocity and Severity Index

According to NCHRP Report 230 (3), it is a life threatening event if an unrestrained occupant impacts the vehicle's interior compartment, in the fore-aft direction, at a velocity in excess of 40 ft/sec. It is the collective judgement of TTI researchers that the onset of a "life"

TABLE AL. VEHICULAR VELOCITY CHANGE FOR IMPACTS WITH EXISTING ADOT SYSTEMS

			CRASH TEST DATA - 1800 LB TEST CAR (1)	4TA - 1800 LE	3 TEST CAR (COSTIDANT TABACT	PREDICTED CHANGE IN VEHICULAR VELOCITY	E IN VEHICULAR VELOCITY
छ	SUPPORT TYPE	Vi, (ft/sec)	Vf, (ft/sec)	ΔV (FT/SEC)	(FT-LB)	>	3000 1b vehicle	4500 lb vehicle
¥	One Square Steel Tube	29.3	25.8	3.5	5,839	3.5b	2.2	1.5
	Abol P1 Post Design (Tests 3 & 4)	8.3	74.8	8.5	40,691	8 <u>.5</u> b	5.4	3.6
8		27.72	13.0	14.7	18,116	14.1	8.2	5.2
	ADDI PZ DESIGN (Tests 19 & 20)	84 E. 48	66.4	17.9	81,681	17.4	11.1	7.2
ان ا	Three Suare Steel Tubes	28.9a	8.2	20.7	23,254	21.0	10.6	6.4
	(Tests 5ª & 6)	87.0	2.09	26.3	117,623	24.9	16.0	10.3
		28.3	19.9	9.4	14,004	10.1	5.7	3.6
	(Two Foot Lap Splice) (Tests 22 & 23)	92.1	80.4	11.7	61,113	11.8	7.4	4.9
ய்	1	29.3	1.4	6.72	25,936	22.4	11.9	7.2
	Siller Steel U-Posts (80 KSI) (Long Lap Splice)	6.08	72.0	18.9	93,226	12.8	11.8	7.7
u.	1	28.6	3.7	24.9	24,353	2.5	11.4	6.9
!	Shiret Steel U-rosis (ou ksi) (Short Lap Splice) (Tests 18 & 21)	90.2	67.8	22.4	107,167	19.0	13.8	8.9

a Two of Three Posts Hit in Test 5. b Based on Vehicle ΔV_\bullet

TABLE A2. VEHICULAR VELOCITY CHANGE FOR IMPACTS WITH ALTERNATE SYSTEMS DEVELOPED BY ADOT

SUPPORT TYPE A. Three 3 lb/ft High Carbon Billet Steel U-Posts (80 ksi) (Ground Splice) (Ground Splice) (Ground Splice) B. Three 4 lb/ft High Carbon Chound Splice) Ground Splice) Ground Splice) 90.3 81.2 9.1 47,256 91.8 78.6 13.2 68,108	EZ	ΔV (FT/SEC) 12.6 9.1	AKE (FT-LB) 19,458	OCCUPANT IMPACT VELOCITY (FT/SEC)	- 1	(FT/SEC) FOR
A. Three 3 lb/ft High Carbon 31.8 19.2 12.6 Billet Steel U-Posts (80 ksi) (Ground Splice) (Tests 26 & 27) B. Three 4 lb/ft High Carbon 30.2 2.2 28.0 Billet Steel U-Posts (80 ksi) 91.8 78.6 13.2		12.6	19,458			3000 lb vehicle 4500 lb vehicle
Billet Steel U-Posts (80 Ks1) (Ground Splice) (Tests 26 & 27) B. Three 4 lb/ft High Carbon 30.2 2.2 28.0 Billet Steel U-Posts (80 ks1) (Ground Splice) 91.8 78.6 13.2		9.1		12.5	7.4	4.7
8. Three 4 1b/ft High Carbon 30.2 2.2 28.0 Billet Steel U-Posts (80 ksi) 91.8 78.6 13.2 (Ground Splice)			47,256	-0-(9.1) ^a	5.8	3.8
8111et Steel U-Posts (80 KS1) (Ground Splice) 91.8 78.6 13.2		0.85	27,470	21.9	12.2	7.4
		13.2	68,108	13.2	8.3	5.5
C. Three Square Steel Tubes 27.4 17.4 10.4 14,060	27.4	10.4	14,060	8.9	6.2	4.0
(ADO! PZ Design) with Unistrut Slip Base (Tests 7024-29, 30)	2.0 6	6.1	32,152	-0-(6.1) ^a	ۍ د	2.6

a Based on Vehicle AV.

threatening" event implies a severity index (SI) of 5. The SI scale is given in Table 1 of the main text. Furthermore, it is assumed that the SI varies linearly with occupant impact velocity as shown in Figure Al. It is noted that this assumption does not mean that the severity of an impact is directly proportional to the impact velocity. As can be seen in Figure VII-C-6 of the AASHTO Barrier Guide (1), accident costs (a measure of accident severity) vary exponentially with the SI, and hence with occupant impact speed.

Data in Tables A1 and A2 and the relationship given in Figure A1 were used to develop the SI values given in Tables A3 and A4. With reference to Tables A1 and A2, it can be seen that for most of the support systems the velocity change of the test vehicle (ΔV) did not vary significantly between the low and high speed impacts. It was therefore assumed that ΔV (and hence the occupant impact velocity and hence the SI) was constant within the impact speed range of interest (approximately 20 mph to 60 mph). However, in the interest of insuring that the severity of impact was not understated, the values of SI in Tables A3 and A4 were based on the higher value of ΔV from the high and low speed tests.

Estimating SI per Number of Posts Hit

Small signs are supported by one or more posts and the degree of hazard they present to an errant motorist depends on the number of posts that can be struck. Furthermore, the severity of striking one post of a given size and type in a multiple-post installation is generally greater than striking the same post in a single-post installation. In using the B/C procedure, it is necessary that an estimate be made of the SI for (a) impacts with single-post installations, and (b) impacts with one, two, three, etc., posts in a multiple-post installation. This was done as follows:

Single-Post Installation - When available, test data were used to determine the SI. System A of Table A3 was the only single post system tested in the present study. For single-post systems not tested, it was assumed that the energy loss equaled that of the multiple-post impact divided by the number of posts hit, that is:

$$E_1 = E_N / N.$$
 (A5)

where

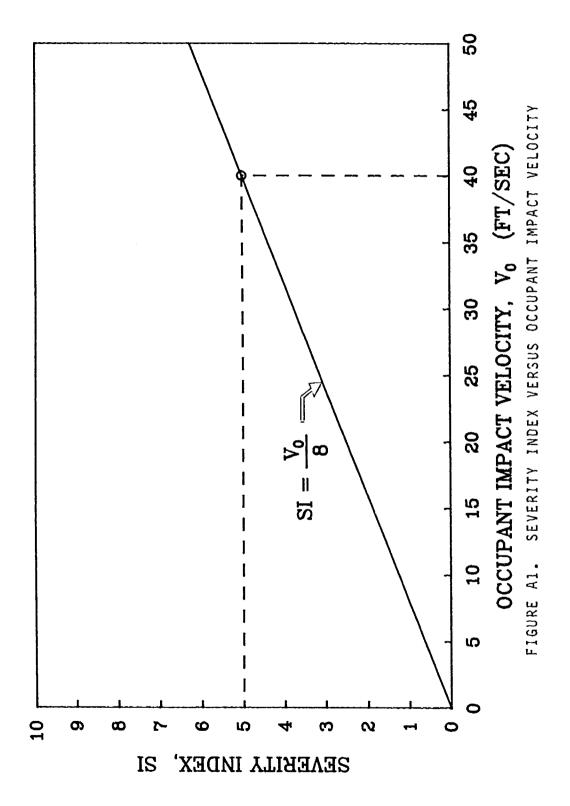


TABLE A3. SEVERITY INDICES FOR EXISTING ADOT SYSTEMS BASED ON CRASH TESTS

			R VEHICLE WE	IGHING
	SUPPORT TYPE	1800 LB	3000 LB	4500 LB
	0 0 0 171			
Α.	One Square Steel Tube ADOT P1 Design	1.1	0.7	0.5
В.	Two Square Steel Tubes ADOT P2 Design	2.2	1.4	0.9
c.	Three Square Steel Tubes ADOT P2 Design	3.1	2.0	1.3
D.	Two 3 lb/ft High Carbon Billet Steel U-Posts (Two Foot Lap Splice)	1.5	0.9	0.6
Ε.	Three 3 lb/ft High Carbon Billet Steel U-Posts (Long Lap Splice)	2.8	1.5	1.0
F.	Three 3 lb/ft High Carbon Billet Steel U-Posts (Short Lap Splice)	2.8	1.7	1.1

TABLE A4. SEVERITY INDICES OF ALTERNATE SYSTEMS DEVELOPED BY ADOT

	CONT. TOOGOUS		VEHICLE WEI	
	SUPPORT TYPE	1800 LB	3000 LB	4500 LB
Α.	Three 3 lb/ft High Carbon Billet Steel U-Posts (Ground Splice)	1.6	0.9	0.6
В.	Three 4 lb/ft High Carbon Billet Steel U-Posts (Ground Splice)	2.7	1.5	0.9
С.	Three Square Steel Tubes (ADOT P2 Design) with Unistrut Slip Base	1.1	0.8	0.5

 E_1 = energy loss in single post installation E_N = energy loss for impact with N posts

Tables A5 and A6 give energy loss values obtained in the described manner, the implied vehicular velocity change, and the associated SI value for existing ADOT systems and candidate (alternate) systems, respectively.

It should be noted that "alternate" systems C and D in Table A6 were not tested in the present study. Test data for these slip-base systems were obtained from the literature (13,14). They were included due to their demonstrated ability to easily meet impact performance standards. As a result, they are candidate systems for upgrading existing substandard systems or as new support systems. The SI values for systems C and D were also determined by use of Equations A1 through A4.

Multiple-Post Installations - When available, test data were used to determine the SI. For combinations not tested, the energy loss was estimated as follows:

$$E_n = (n / N) E_N \dots (A6)$$

where

 E_n = energy loss for impact with "n" posts E_N = energy loss for impact with "N" posts (from crash test data)

Tables A7 and A8 give energy loss, change in vehicular velocity, and the associated SI values for two, three, and four post systems for existing ADOT systems and candidate (alternate) systems, respectively. Equations A1 through A4 and A6 were used to compute values given in the tables.

It should be noted that "alternate" system C in Table A8 was not tested in the present study. Test data for this laminated wood system were obtained from the literature ($\underline{15}$). It was included due to its demonstrated ability to easily meet impact performance standards.

Adjustments to SI for B/C Program

The B/C program used in the analysis does not allow input of SI as a function of vehicle size (or weight). Thus, to account for size effects it was necessary to compute a weighted average of the SI.

TABLE AS. SEVERITY INDICES FOR EXISTING ADOT SINGLE POST INSTALLATIONS

a Low speed impact (approximately 20 mph)
b High speed impact (approximately 60 mph)
c Occupant impact velocity measured in crash test

SEVERITY INDICES FOR ALTERNATE SINGLE POST INSTALLATIONS TABLE AG.

A. 3 lb/ft High Carbon 6,4864 3.9 2.3 1.5 0.5 0.3 Steel U-Post (Ground Splice) B. 4 lb/ft High Carbon 52,703b 4.5 2.7 1.8 0.8 0.4 Steel U-Post (Ground Splice) C. Square Steel Tube with 4,6874 3.0 1.9 1.3 0.9 0.4 0.2 Unistrut Sip Base 10,717b 2.0 1.3 0.9 0.4 0.2 0.9 Unistrut Steel Pipe 13,647c 3.8 2.5 1.5 1.0 0.4 0.2 0.9 Unistrut Steel Pipe 2,0804 2.5 1.5 1.0 0.9 0.4 0.2 0.9 Slip Base 12,2 1.5 1.0 0.4 0.2 0.9 Slip Base 16,635f 4.7 2.8 1.8 1.2 0.7 0.4 0.5 0.9 Unistructional 21,1909 5.9 3.5 2.3 2.3 0.7 0.4 Ests S-18 8.5-17 (12) F. 5 in. Diam. Steel Pipe 21,180 5.9 3.5 2.3 2.3 0.7 0.4 Ests S-11 8.5-17 (12)				ENERGY LOSS AKE	그러	CHANGE IN VEHICULAR VELOCITY (FT/SEC) FOR VEHICLE WEIGHING	FLOCITY WEIGHING	S.I. FOR	VEHICLE WEI	GHING
A. 3 lb/ft High Carbon Steel U-Post (Ground Splice) B. 4 lb/ft High Carbon Steel U-Post (Ground Splice) C. Square Steel Tube with Unistrut Slip Base Test 4 [11] Som Multi-Directional Slip Base Test 8 S-18 (12) F. 5 in. Diam. Steel Pipe on Multi-Directional Slip Base Test 5 [12] F. 5 in. Diam. Steel Pipe on Multi-Directional Slip Base Test 5 [12] F. 5 in. Diam. Steel Pipe on Multi-Directional Slip Base Test 5 [12] F. 5 in. Diam. Steel Pipe on Multi-Directional Slip Base Test 5 [12] F. 5 in. Diam. Steel Pipe Slip Base Test 5 [12] F. 5 in. Diam. Steel Pipe Slip Base Test 5 [12] F. 5 in. Diam. Steel Pipe Slip Base Test 5 [12] F. 5 in. Diam. Steel Pipe Slip Base Test 5 [12] F. 5 in. Diam. Steel Pipe Slip Base Test 5 [12] F. 5 in. Diam. Steel Pipe Slip Base Test 5 [12]			SUPPORT TYPE	(FT-LB)	1800 LB	3000 18	4500 LB	1800 LB	3000 78	4500 LB
Steel U-Post (Ground Splice) 19,732 3.5 1.3		A.	3 1b/ft High Carbon	6,486a	3.9	2.3	1.5	0.5	0.3	0.2
B. 4 lb/ft High Carbon 9.157a 6.0 3.5 2.3 0.8 0.4 Steel U-Post (Ground Splice) 22,703b 4.5 2.7 1.8 0.4 0.2 C. Square Steel Tube with Unistrut Slip Base On Multi-Directional Slip Base Slip Base Tests S-8 & S-18 (12) 4,687a 10,717b 3.0 1.9 1.3 0.9 0.4 0.2 D. 3 in. Diam. Steel Pipe On Multi-Directional Slip Base Tests S-11 & S-17 (12) 1.6 3.8 2.2 1.5 1.8 0.4 0.2 F. 5 in. Diam. Steel Pipe On Multi-Directional Slip Base Tests S-11 & S-17 (12) 16,635f Tests S-11 & S-17 (12) 4.7 2.8 1.8 3.5 0.7 0.4			(Ground Splice)	~7c/°c1	2.5	7	7			
Steel U-Post (Ground Splice) 22,7030 4.5 2.7 1.8 (Ground Splice) Square Steel Tube with Unistrut Slip Base 4,687a 3.0 1.9 1.3 0.4 0.2 Unistrut Slip Base on Multi-Directional Slip Base rests S-8 & S-18 (12) 13,647c 3.8 2.2 1.5 0.4 0.2 Slip Base rests S-8 & S-18 (12) 7,618e 3.1 1.8 1.7 0.4 0.2 5 in. Diam. Steel Pipe on Multi-Directional Slip Base rests S-11 & S-17 (12) 16,635f 4.7 2.8 1.8 0.7 0.4 5 in. Diam. Steel Pipe on Multi-Directional Slip Base rests S-11 & S-17 (12) 16,635f 4.7 2.8 1.8 0.7 0.4		æ	4 lb/ft High Carbon	9,157a	6.0	3.5	2.3	0.8	0.4	0.3
C. Square Steel Tube with Unistrut Slip Base Uniti-Directional Uniti-Direction			Steel U-Post (Ground Splice)	22,703 ⁵	4.5	2.7	1.8			
D. 3 in. Diam. Steel Pipe on Multi-Directional Slip Base Test 4 (11) 13,647c 3.8 2.2 1.5 1.0 7,618e 3.1 1.8 1.2 2.2 1.5 1.0 7,618e 3.1 1.8 1.2 0.4 0.2 F. 5 in. Diam. Steel Pipe on Multi-Directional Slip Base Slip Bas		نا	Square Steel Tube with	4,687a	3.0	1.9	1.3	0.4	0.2	0.15
D. 3 in. Diam. Steel Pipe 13,647c 3.8 2.2 1.5 0.4 0.2 on Multi-Directional 7,618e 3.1 1.8 1.2 1.2			Unistrut Slip Base	10,7170	2.0	1.3	6.0			
on Multi-Directional 9,080 ^d 2.5 1.5 1.0 Slip Base Tests S-8 & S-18 (12) F. 5 in. Diam. Steel Pipe 16,635 ^f 4.7 2.8 1.8 On Multi-Directional 21,190 ^g 5.9 3.5 2.3 Tests S-11 & S-17 (12)	4	<u>ا</u>	3 in. Diam. Steel Pipe	13,647¢	3.8	2.2	1.5	0.4	0.2	0.1
Slip Base Test 4 (11) Test 4 (11) Test S-8 & S-18 (12) 5 in. Diam. Steel Pipe On Multi-Directional Slip Base Tests S-11 & S-17 (12)	15		on Multi-Directional	b,080d	2.5	1.5	1.0			
Tests S-8 & S-18 (12) 5 in. Diam. Steel Pipe 16,635			Slip Base Test 4 (11)	7,618 ^e	3.T	ω, C	7.5			
5 in. Diam. Steel Pipe 16,635 ^f 4.7 2.8 1.8 0.7 0.4 on Multi-Directional 21,1909 5.9 3.5 2.3 Slip Base Tests S-11 & S-17 (12)			Tests S-8 & S-18 (12)							
C*5 6.50 5051,12		l ii	5 in. Diam. Steel Pipe	16,635f	4.7	2.8	1.8	0.7	0.4	0.3
Tests S-11 & S-17 (12)			on Multi-Directional Slip Base	6061,12	ñ. C	ຕຸ	5.3			
			Tests S-11 & S-17 (12)							

a Low speed impact (approximately 20 mph)
b High speed impact (approximately 60 mph)
c Impact speed was 45.4 mph
d Impact speed was 31.3 mph
e Impact speed was 31.3 mph
f Impact speed was 44.5 mph
g Impact speed was 45.5 mph

TABLE A7. SEVERITY INDICES FOR EXISTING ADOT MULTIPLE POST INSTALLATIONS

	SUPPORT TYPE	NO. OF POSTS	ENERGY LOSS	CHANGE IN (FT/SEC) 1800 LB	CHANGE IN VEHICULAR VELOCITY (FT/SEC) FOR VEHICLE WEIGHIN 800 LB 3000 LB 4500	VELOCITY WEIGHING 4500 LB	S.I. FOR 1800 LB	S.I. FOR VEHICLE WEIGHING 1800 LB 3000 LB 4500	GHING 4500 LB
4	. Square Steel Tube	2	18,116 ^a 81,681 ^b	14.1 ^c 17.4 ^c	8.2 11.1	5.2 7.2	2.2	1.4	0.9
		m	34,881 ^d 117,623 ^b	29.3 24.9c	19.6 16.0	10.6 10.3	3.7	2.5	1.3
		4	46,508a 156,831 ^b	29.3 42.8	29.3 22.2	15.9 14.0	5.4	3.7	2.0
മ	3 1b/ft High Carbon Steel U-Post (80 ksi)	2	14,004a 61,113b	10.1c 11.8c	5.7	3.6 4.9	1.5	0.9	0.6
	(Two Foot Lap Splice)	m	21,006a 91,670b	19.0 20.0	9.1 11.4	5.7 7.4	2.5	1.4	0.9
46		4	28,008 ⁴ 122,226 ^b	29.3 28.0	13.3 15.6	7.9 10.0	3.7	2.0	1.3
نا	3 lb/ft High Carbon Steel U-Post (80 ksi)	2	17,291a 62,151b	13.8	7.2	4.6 5.0	1.7	1.0	0.6
	(Long Lap Splice)	m	25,936a 93,226b	22.4c 12.8c	11.9	7.2	2.8	1.5	1.0
		4	34,5814 124,301b	29.3 29.1	18.5 16.1	10.2 10.4	3.7	2.3	1.3
٥	3 1b/ft High Carbon Steel U-Post (80 ksi)	2	16,235a 71,445b	13.2	6.9 8.9	4.4 5.9	1.9	1.1	0.7
	(Short Lap Splice)	က	24,353 ⁴ 107,167 ^b	22.5 ^c 19.0 ^c	11.4 13.8	6.9 8.9	2.8	1.7	1.1
		4	32,471 ⁴ 142,889 ^b	29.3 35.2	17.6 19.0	9.8 12.2	4.4	2.4	1.5

a Low speed impact (approximately 20 mph)
b High speed impact (approximately 60 mph)
c Occupant impact velocity measured in crash test

SEVERITY INDICES FOR ALTERNATE MULTIPLE POST INSTALLATIONS TABLE AB.

	SUPPORT TYPE	NO. OF POSTS	ENERGY LOSS	CHANGE IN (FT/SEC) F 1800 LB	CHANGE IN VEHICULAR VELOCITY (FT/SEC) FOR VEHICLE WEIGHING 800 LB 3000 LB 4500 L	FELOCITY WEIGHING 4500 LB	S.I. FOR 1800 LB	S.I. FOR VEHICLE WEIGHING 1800 LB 3000 LB 4500	GHING 4500 LB
A	ļ	2	12,972a 31,504b	8.6 5.5	3.8	3.1	1.1	9*0	0.4
	(Ground Splice)	m	19,458a 47,256b	12.5c 9.1	7.4 5.8	4.7 3.8	1.6	6.0	9.0
		4	25,944a 63,008b	22.7 13.5	10.5 7.8	6.5 5.1	2.8	1.3	0.8
m	4 1b/ft High Carbon Steel U-Post (80 ksi)	2	18,314 ^a 45,406 ^b	14.2 9.3	7.4	4.7	1.8	6*0	9.0
47	(Ground Splice)	m	27,470a 68,108b	21.9c 13.2c	12.2 8.3	7.4 5.5	2.7	1.5	0.9
		4	36,628 ⁴ 90,812b	29.3 19.8	19.0 11.3	10.5 7.4	3.7	2.4	1.3
نا	Square Steel Tube with Unistrut Slip Base	2	9,373a 21,435b	6.3	4.0 2.6	2.6 1.7	0.8	0.5	0.3
		m	14,060a 32,152b	8.9c 6.1	6.2 4.0	4.0 2.6	1.1	0.8	0.5
		4	18,747a 42,869b	15.7 8.1	ອີເ ເລື່ອ	3.5	2.0	1.1	0.7
6	"Micro-Lam" Laminated Veneer (Saw Cut) 8"x15"x1 1/4" (13)	5q	11,705a 41,083b	7.8 ^c 8.8 ^c	4.5 5.3	2.9 3.5	1.1	0.7	0.44

a Low speed impact (approximately 20 mph)
b High speed impact (approximately 60 mph)
c Occupant impact velocity measured in crash test
d Installation had two posts but only one hit in test

Four vehicle sizes are assumed to represent the vehicle population in the B/C program. In the present analysis, the distribution is assumed as follows:

	Vehicle Weight	Percent of
<u>Vehicle</u>	W, (1b)	Population, P(%)
1	1800	12
2	3000	43
3	4500	21
4	>4500	24

The first step in the adjustment process is to compute a weighted average of the accident costs, AC, associated with the SI for each of the four vehicle sizes.

$$\overline{AC} = \frac{AC (P_1) + AC_2(P_2) + AC_3(P_3) + AC_4(P_4)}{1.0}$$
(A7)

where

$$AC_{i}$$
 = Accident cost for vehicle "i" for given SI_{i} , $i=1,4$
 P_{i} = Fractional portion of population of vehicle "i", $i=1,4$

The weighted \overline{SI} is then the SI corresponding to the value of \overline{AC} . It should be noted that in the present analysis, it was assumed that AC_4 was zero.

As an example of the use of Equation A7, consider a 3 lb/ft high carbon steel U-post system. It is desired to compute an SI for a two-post impact. From Table A8,

$$SI_1 = 1.1$$

 $SI_2 = 0.6$
 $SI_3 = 0.4$

Then, using Table 1 (in main text) the following values are obtained (using linear interpolation).

$$AC_1 = $4480$$

 $AC_2 = 3320
 $AC_3 = 2870

Next, from Equation A7,

$$\overline{AC}$$
 = (4480) (0.12) + (3320) (0.43) + (2870) (0.21)

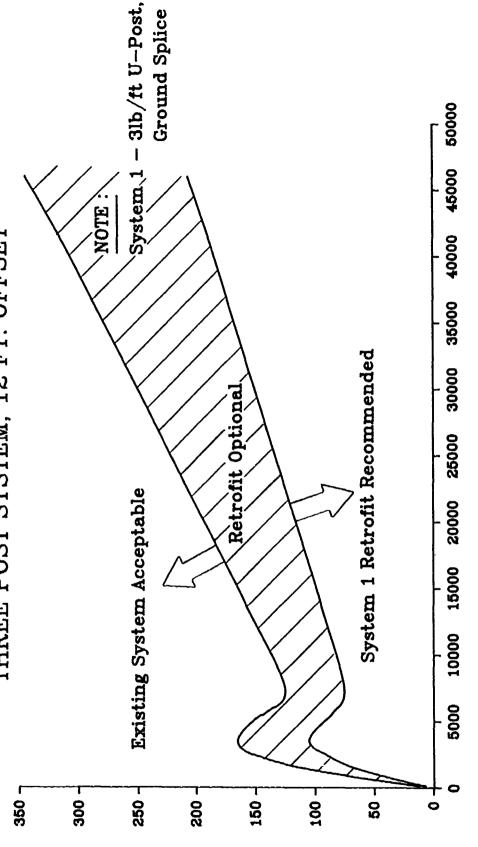
 $\overline{AC} = 2570

Finally, using Table 1 the value of $\overline{\text{SI}}$ is obtained by linear interpolation.

$$\overline{SI} = 0.3$$

A P P E N D I X B RETROFIT GUIDELINES FOR U-POSTS

3 LB/FT U-POST - LONG LAP SPLICE THREE POST SYSTEM, 12 FT. OFFSET



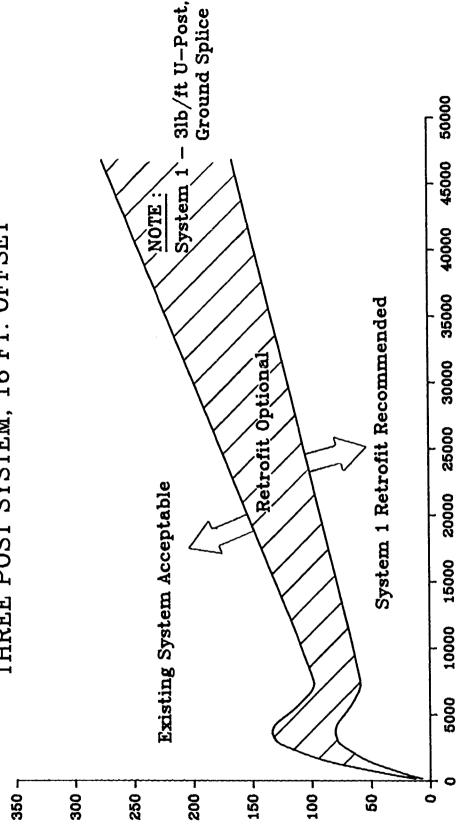
AVERAGE DAILY TRAFFIC

B1. U-POST RETROFIT GUIDELINES, PART A

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3 LB/FT U-POST - LONG LAP SPLICE THREE POST SYSTEM, 16 FT. OFFSET



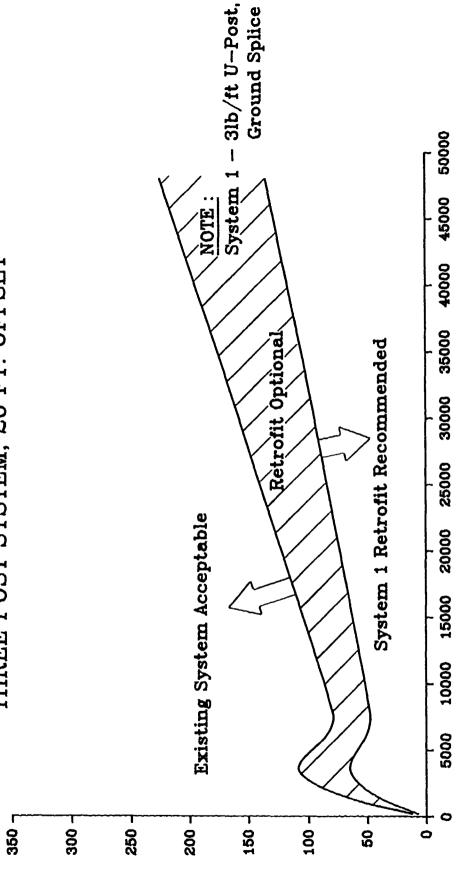
AVERAGE DAILY TRAFFIC

FIGURE B2. U-POST RETROFIT GUIDELINES, PART

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INITIAL COST

3 LB/FT U-POST - LONG LAP SPLICE THREE POST SYSTEM, 20 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE B3. U-POST RETROFIT GUIDELINES, PART C

INITIAL

COST (DOLLARS)

System 2- 4" Diam. Steel Pipe, Slip Base 10000 15000 20000 25000 30000 35000 40000 45000 50000 NOTE: THREE POST SYSTEM, 12 FT. OFFSET 3 Lb/Ft U-POST-LONG LAP SPLICE Retrofit Optional System 2 Retrofit Recommended Existing System Acceptable 5000 7007 **600** – 0 200 200 400 300 100 COZL **LAITINI**

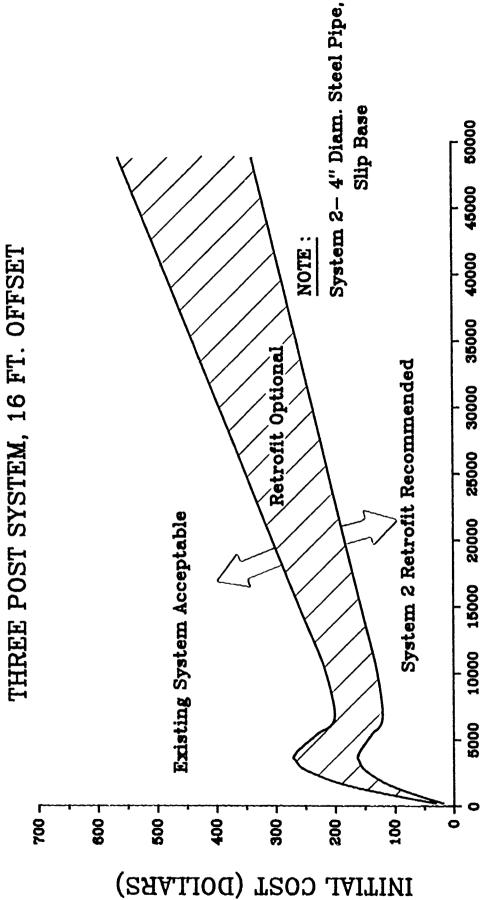
RETROFIT GUIDELINES

FIGURE 84. U-POST RETROFIT GUIDELINES, PART D

AVERAGE DAILY TRAFFIC

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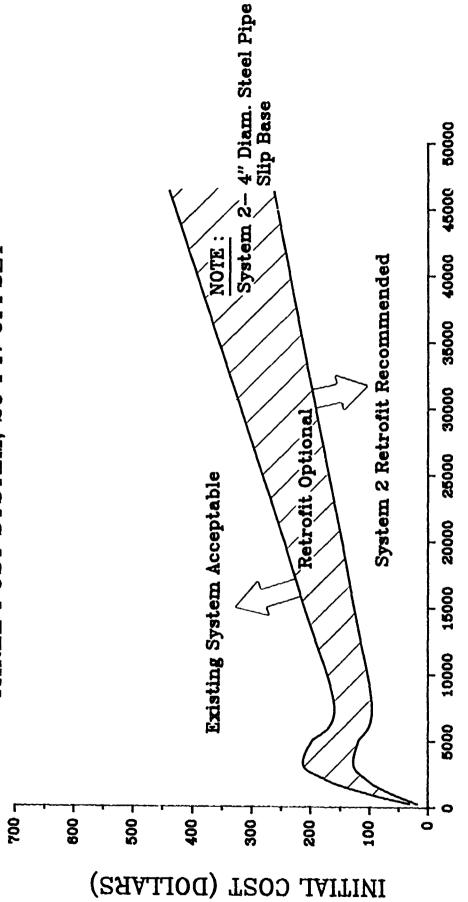
3 Lb/Ft U-POST-LONG LAP SPLICE THREE POST SYSTEM. 16 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE B5. U-POST RETROFIT GUIDELINES, PART E

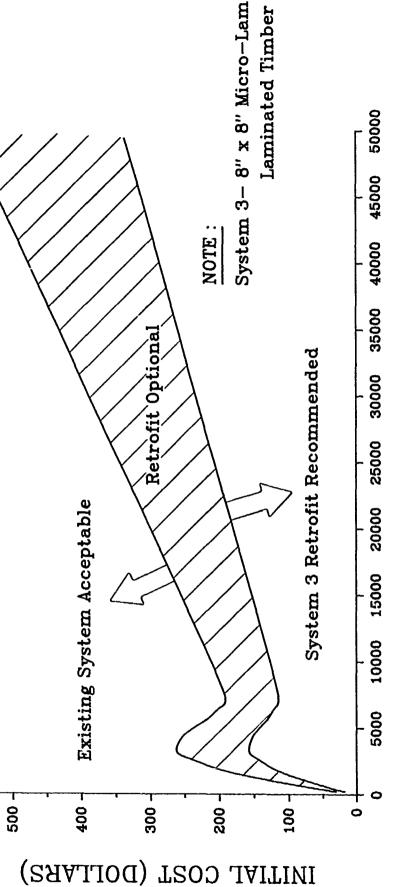
3 Lb/Ft U-POST-LONG LAP SPLICE THREE POST SYSTEM, 20 FT. OFFSET



AVERAGE DAILY TRAFFIC

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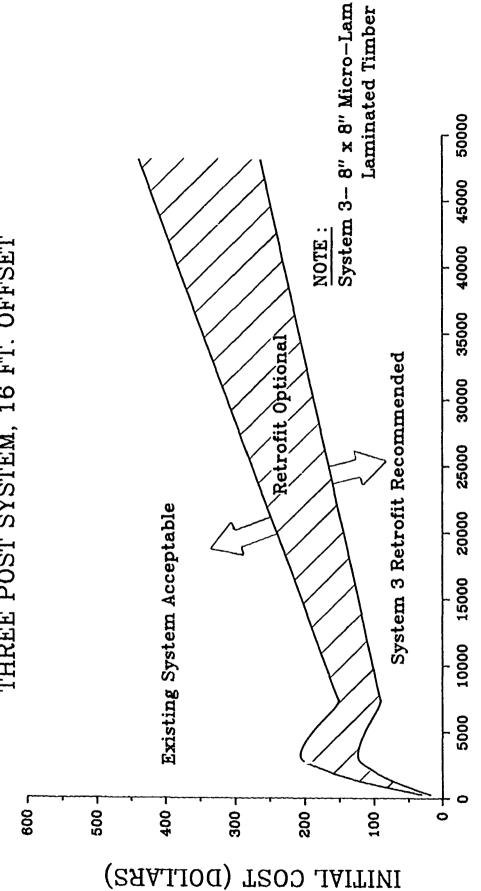
THREE POST SYSTEM, 12 FT. OFFSET 3 Lb/Ft U-POST-LONG LAP SPLICE RETROFIT GUIDELINES 600 m



AVERAGE DAILY TRAFFIC FIGURE B7. U-POST RETROFIT GUIDELINES, PART

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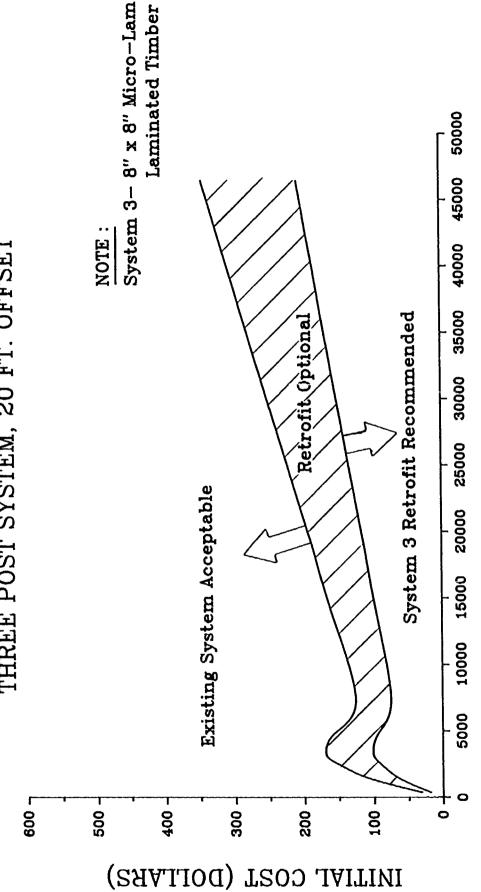
3 Lb/Ft U-POST-LONG LAP SPLICE THREE POST SYSTEM, 16 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE B8. U-POST RETROFIT GUIDELINES, PART H

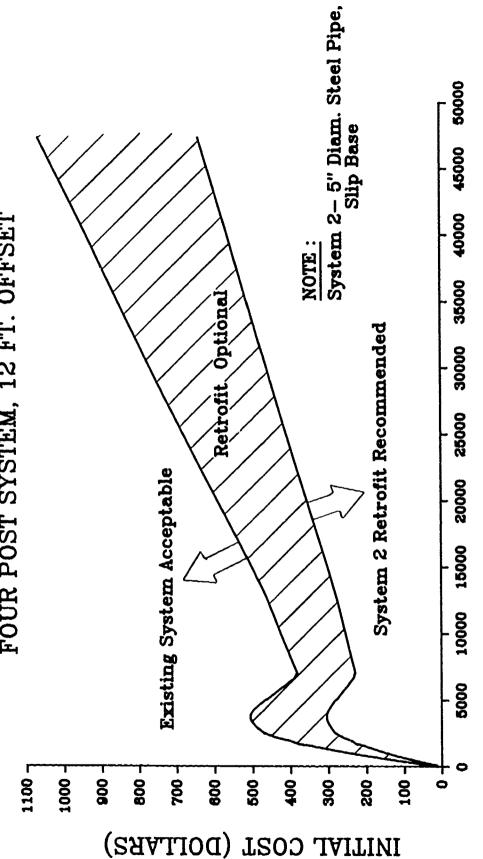
3 Lb/Ft U-POST-LONG LAP SPLICE THREE POST SYSTEM, 20 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE B9. U-POST RETROFIT GUIDELINES, PART I

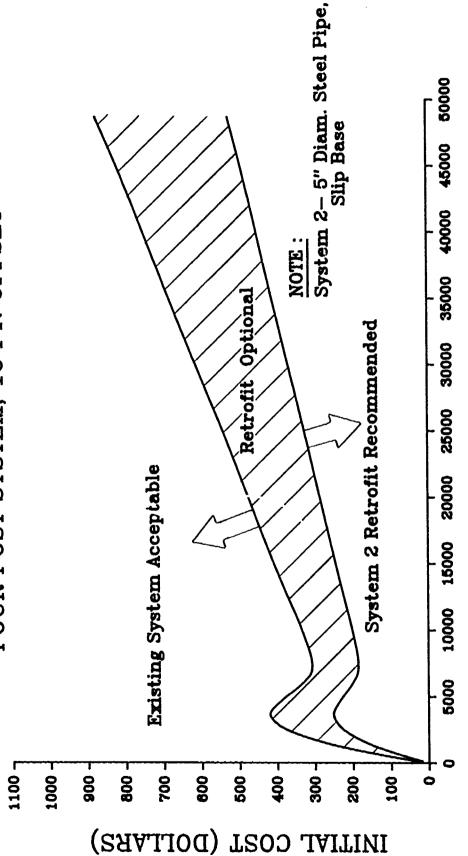
3 LB/FT U-POST-LONG LAP SPLICE FOUR POST SYSTEM, 12 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE B10. U-POST RETROFIT GUIDELINES, PART J

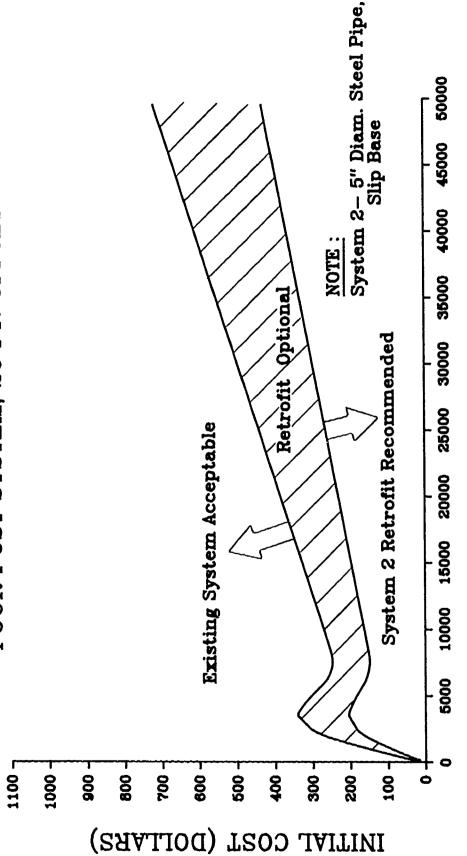
3 LB/FT U-POST-LONG LAP SPLICE FOUR POST SYSTEM, 16 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE BIL. U-POST RETROFIT GUIDELINES, PART K

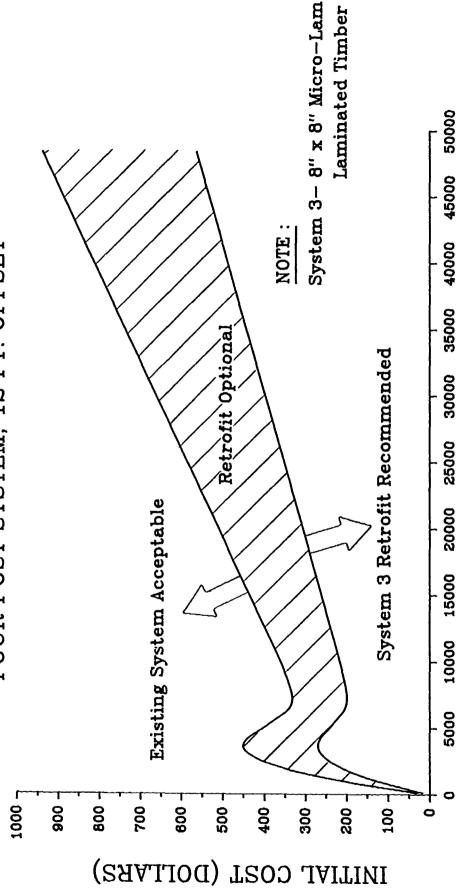
3 LB/FT U-POST-LONG LAP SPLICE FOUR POST SYSTEM, 20 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE B12. U-POST RETROFIT GUIDELINES, PART L

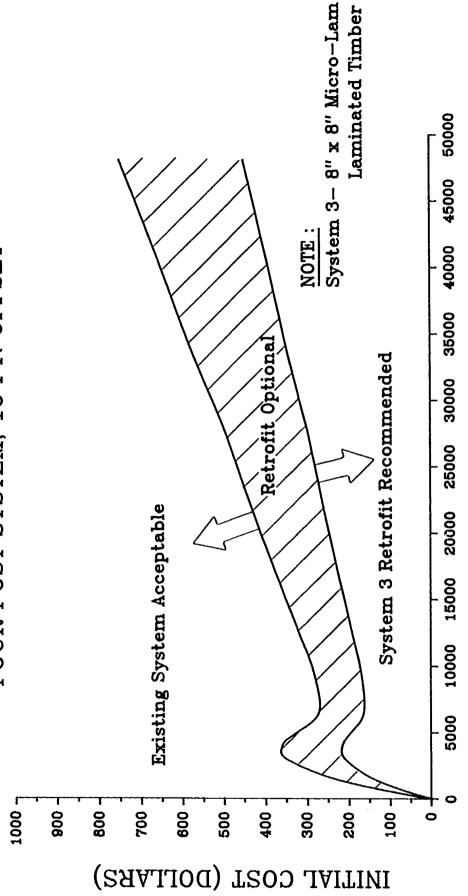
3 Lb/Ft U-POST-LONG LAP SPLICE FOUR POST SYSTEM, 12 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE B13. U-POST RETROFIT GUIDELINES, PART M

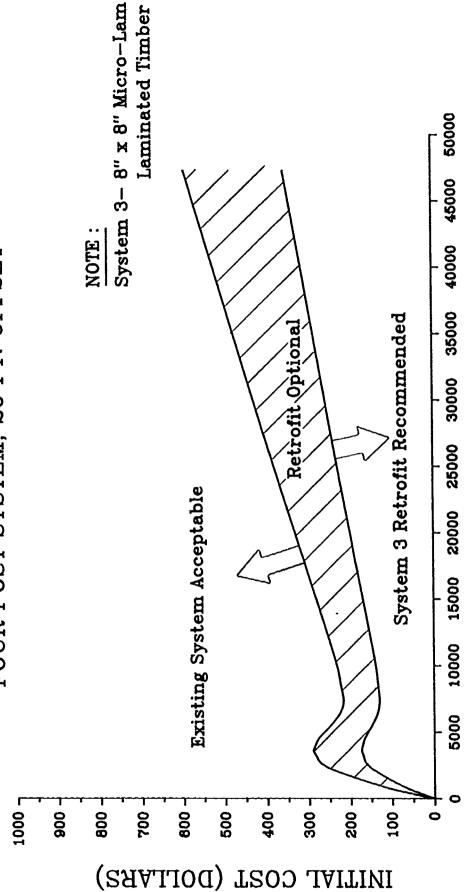
3 Lb/Ft U-POST-LONG LAP SPLICE FOUR POST SYSTEM, 16 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE 814. U-POST RETROFIT GUIDELINES, PART N

3 Lb/Ft U-POST-LONG LAP SPLICE FOUR POST SYSTEM, 20 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE B15. U-POST RETROFIT GUIDELINES, PART O

A P P E N D I X C RETROFIT GUIDELINES FOR P2 POSTS

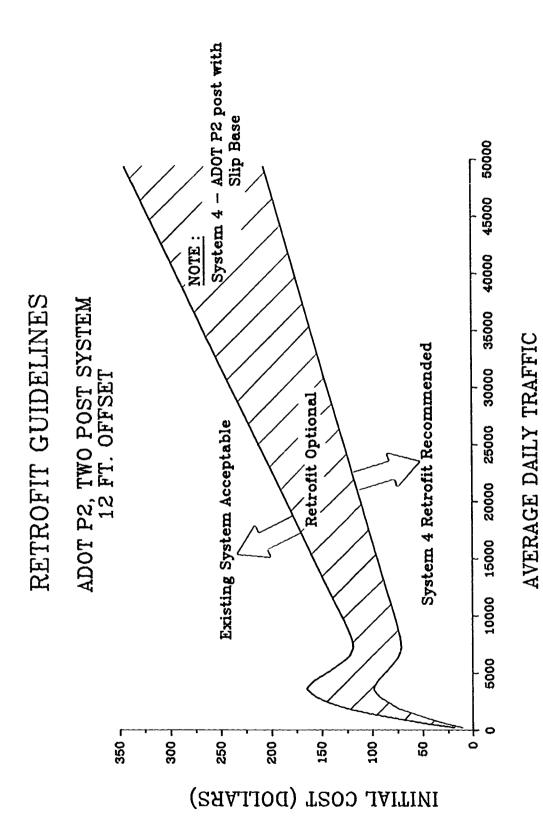
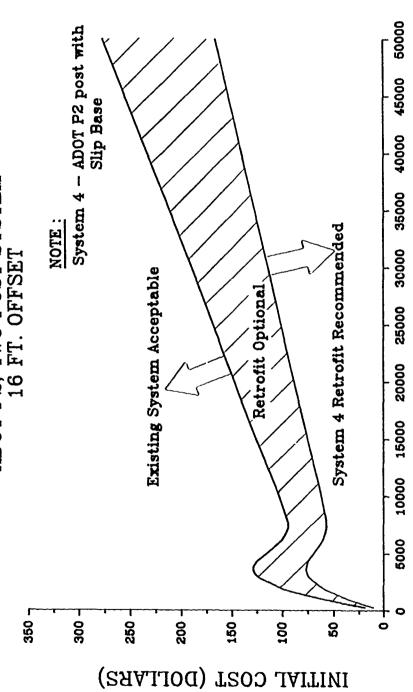


FIGURE C1. P2 POST RETROFIT GUIDELINES, PART A

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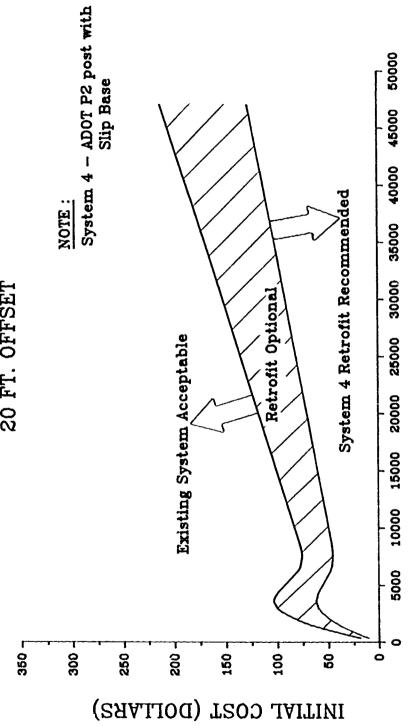
ADOT P2, TWO POST SYSTEM 16 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE C2. P2 POST RETROFIT GUIDELINES, PART B

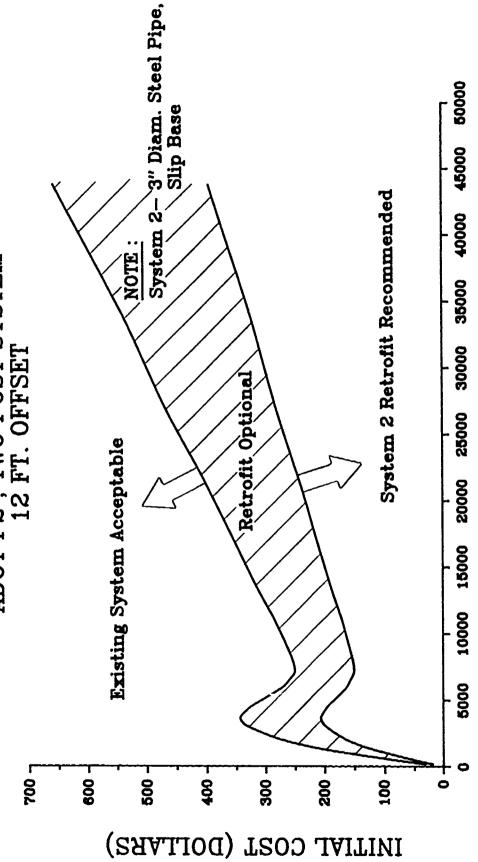
ADOT P2, TWO POST SYSTEM 20 FT. OFFSET



P2 POST RETROFIT GUIDELINES, PART C. AVERAGE DAILY TRAFFIC

FIGURE C3.

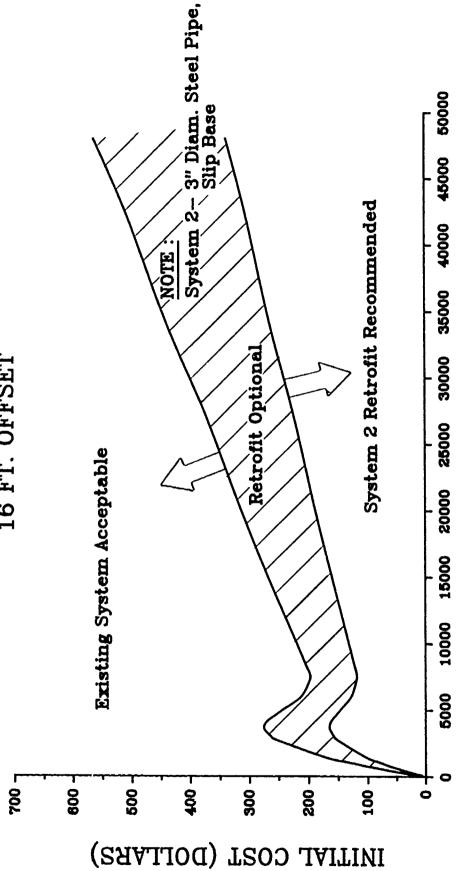




AVERAGE DAILY TRAFFIC

FIGURE C4. P2 POST RETROFIT GUIDELINES, PART D

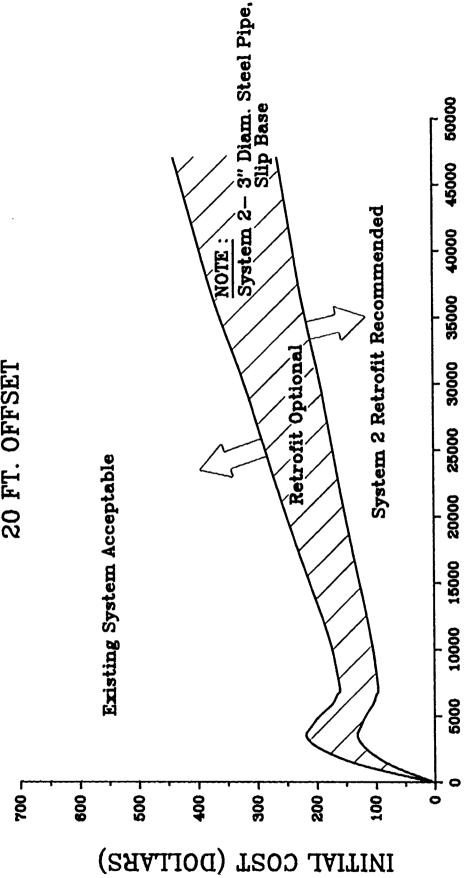
ADOT P2, TWO POST SYSTEM 16 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE C5. P2 POST RETROFIT GUIDELINES, PART E

ADOT P2, TWO POST SYSTEM 20 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE C6. P2 POST RETROFIT GUIDELINES, PART F

System 3-8" x 8" Micro-Lam Laminated Timber NOTE: ADOT P2, TWO POST SYSTEM 12 FT. OFFSET RETROFIT GUIDELINES Retrofit Optional Existing System Acceptable 450 J 350 -1004 300 280 . 2002 (DOLLARS) INITIAL COST

System 3 Retrofit Recommended

P2 POST RETROFIT GUIDELINES, PART G AVERAGE DAILY TRAFFIC FIGURE C7.

45000

40000

35000

30000

15000 20000 25000

10000

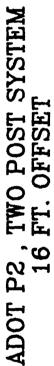
5000

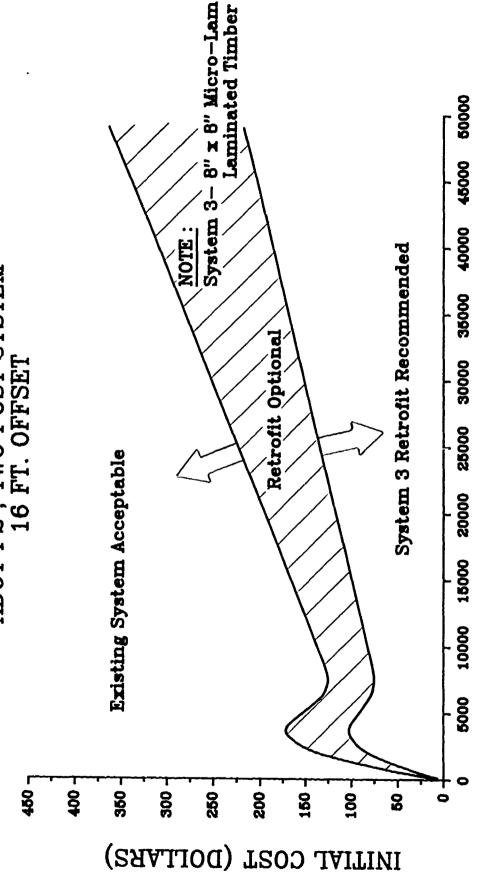
T 0

. 2

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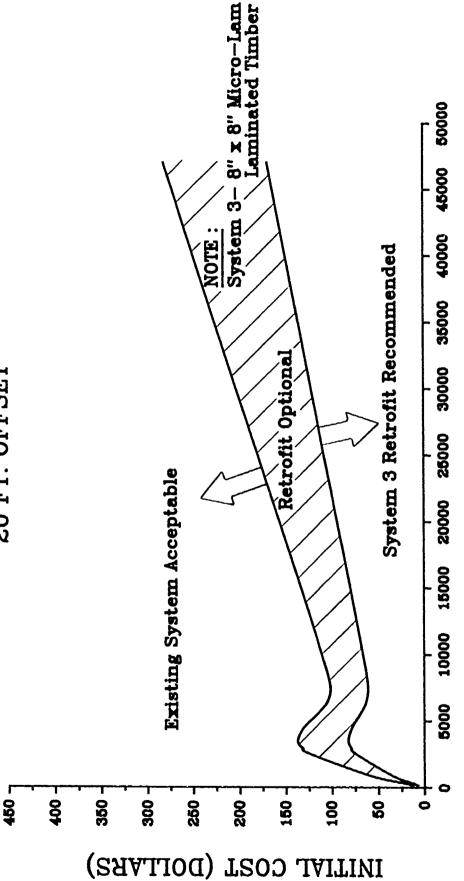


AVERAGE DAILY TRAFFIC

P2 POST RETROFIT GUIDELINES, PART H

FIGURE C8.

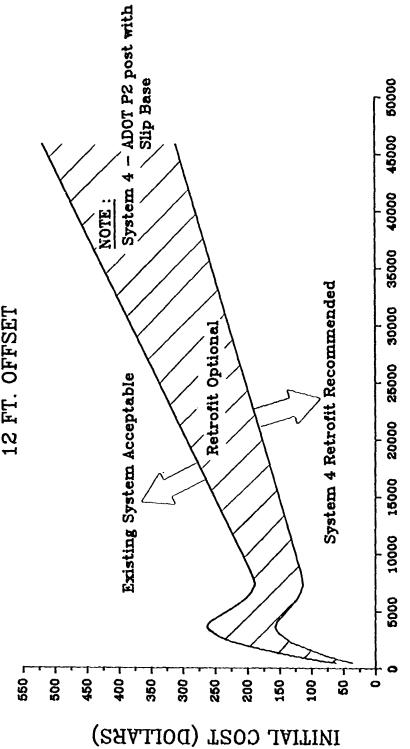
ADOT P2, TWO POST SYSTEM 20 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE C9. P2 POST RETROFIT GUIDELINES, PART I

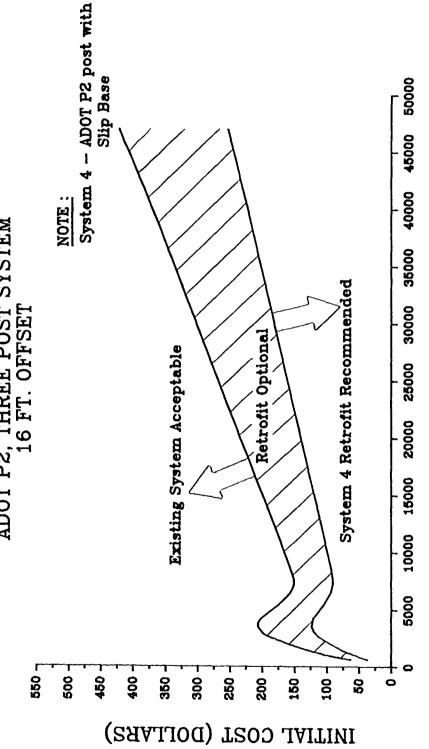
ADOT P2, THREE POST SYSTEM 12 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE C10. P2 POST RETROFIT GUIDELINES, PART J

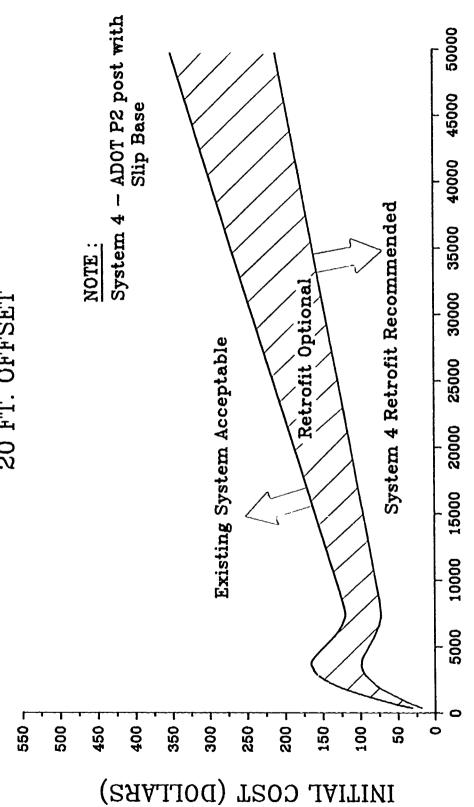
ADOT P2, THREE POST SYSTEM 16 FT. OFFSET



AVERAGE DAILY TRAFFIC

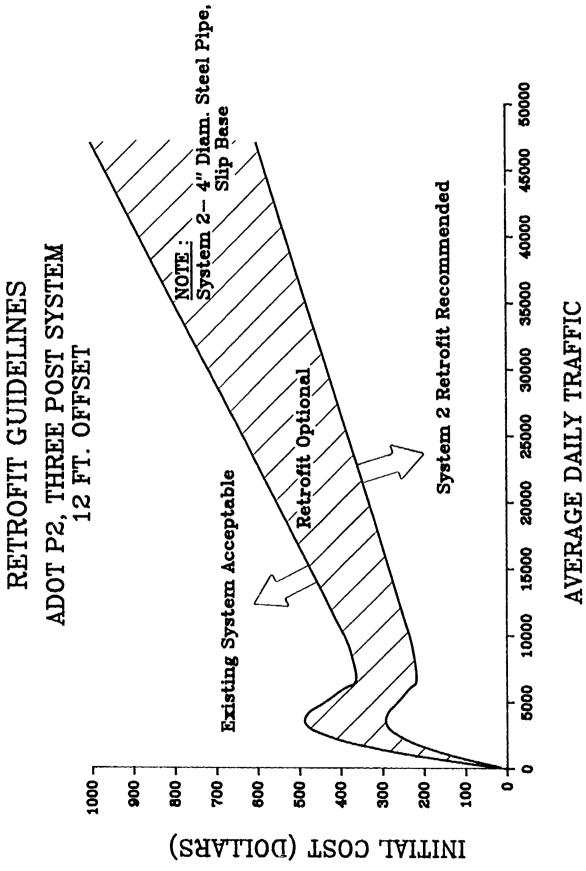
FIGURE C11. P2 POST RETROFIT GUIDELINES, PART K

ADOT P2, THREE POST SYSTEM 20 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE C12. P2 POST RETROFIT GUIDELINES, PART L

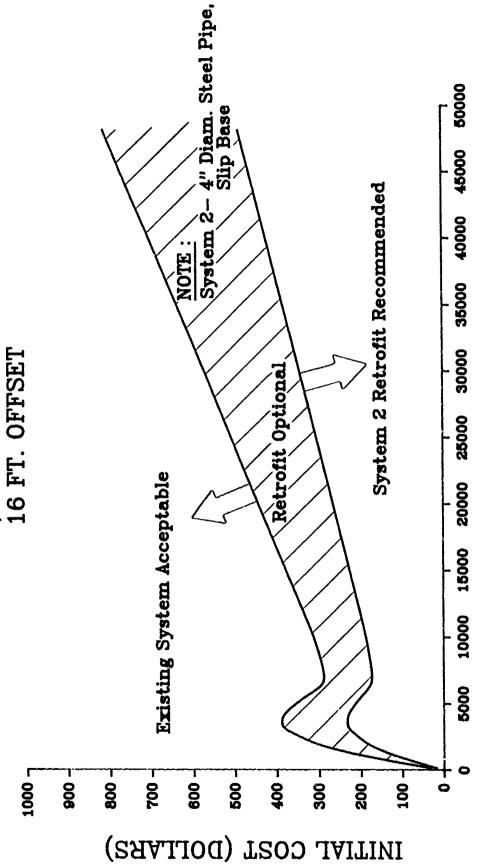


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FIGURE C13. P2 POST RETROFIT GUIDELINES, PART

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RETROFIT GUIDELINES ADOT P2, THREE POST SYSTEM 16 FT. OFFSET



AVERAGE DAILY TRAFFIC

P2 POST RETROFIT GUIDELINES, PART N

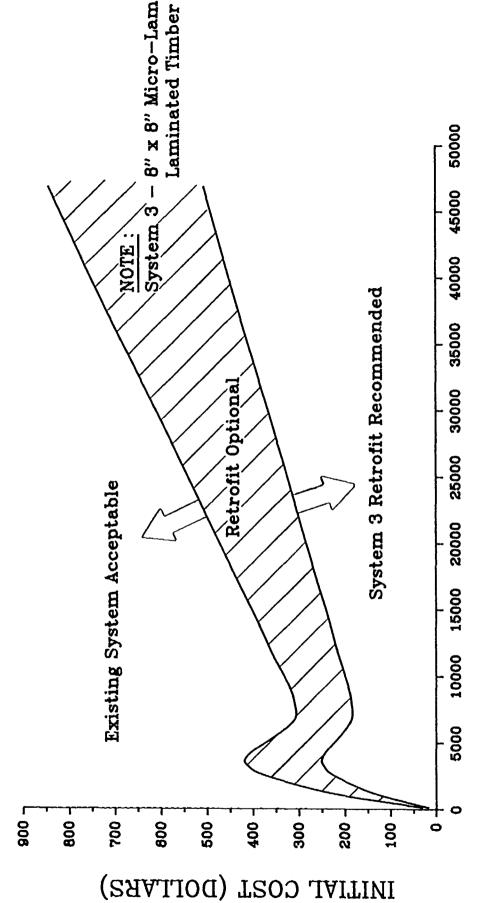
FIGURE C14.

NOTE: System 2- 4" Diam. Steel Pipe, Slip Base System 2 Retrofit Recommended ADOT P2, THREE POST SYSTEM 20 FT. OFFSET Retrofit Optional Existing System Acceptable - 006 – - 002 (DOLLARS) INITIAL COST

AVERAGE DAILY TRAFFIC

FIGURE C15. P2 POST RETROFIT GUIDELINES, PART 0

ADOT P2, THREE POST SYSTEM 12 FT. OFFSET

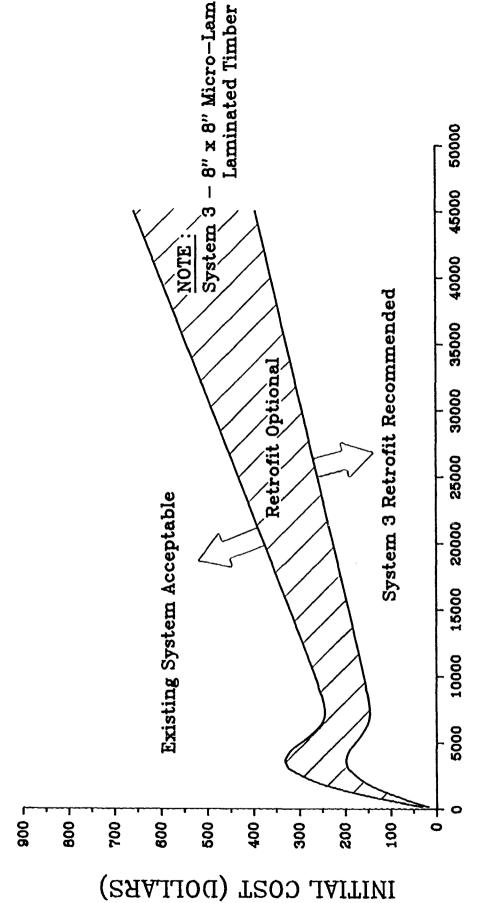


AVERAGE DAILY TRAFFIC

FIGURE C16. P2 POST RETROFIT GUIDELINES, PART

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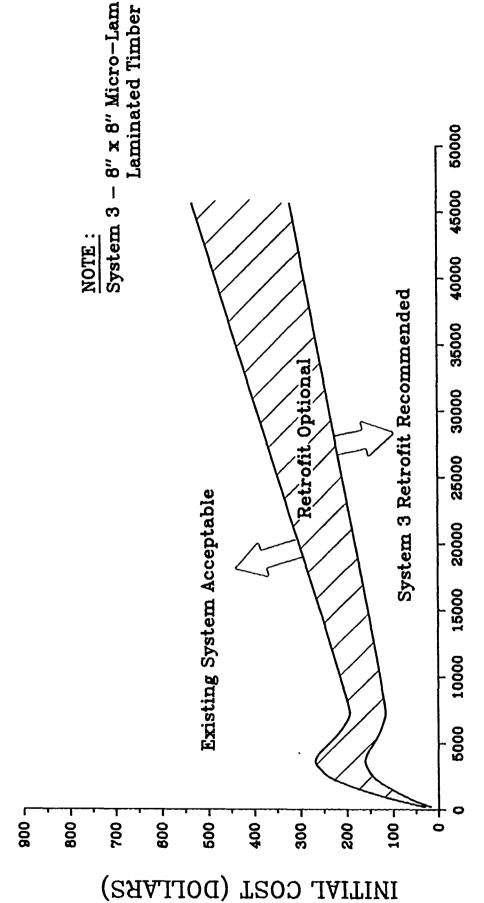
ADOT P2, THREE POST SYSTEM 16 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE C17. P2 POST RETROFIT GUIDELINES, PART Q

ADOT P2, THREE POST SYSTEM 20 FT. OFFSET



P2 POST RETROFIT GUIDELINES, PART AVERAGE DAILY TRAFFIC FIGURE C18.

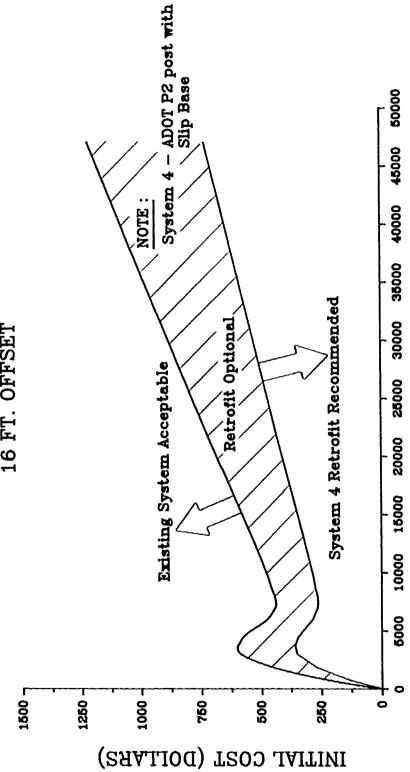
œ

NOTE: System 4 - ADOT P2 post with Slip Base 50000 10000 15000 20000 25000 30000 35000 40000 45000 ADOT P2, FOUR POST SYSTEM 12 FT. OFFSET RETROFIT GUIDELINES System 4 Retrofit Recommended Retrofit Optional Existing System Acceptable 2000 1500 ¬ 750 -200 -250 -1250 -1000 Ö (DOLLARS) INITIAL COST

FIGURE C19. P2 POST RETROFIT GUIDELINES, PART S

AVERAGE DAILY TRAFFIC

ADOT P2, FOUR POST SYSTEM 16 FT. OFFSET

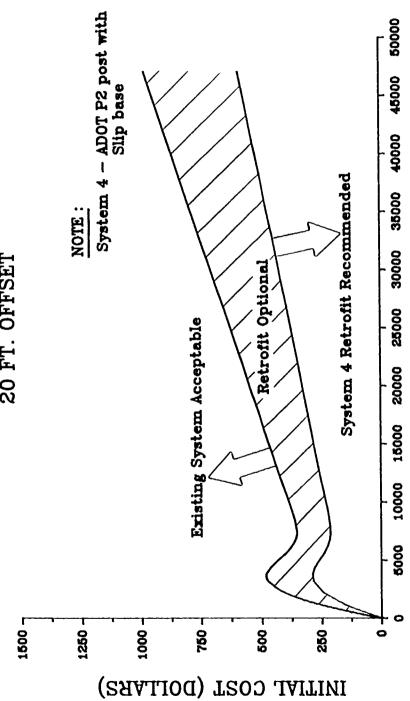


AVERAGE DAILY TRAFFIC

FIGURE C20. P2 POST RETROFIT GUIDELINES, PART T

86

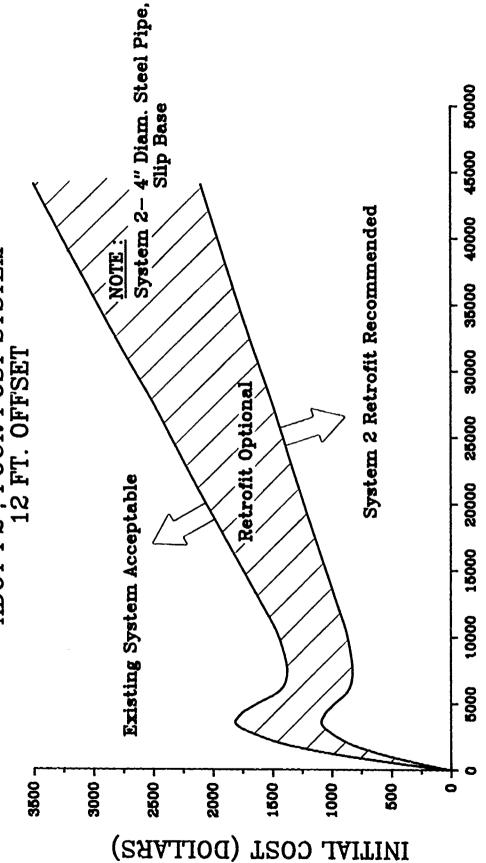
ADOT P2, FOUR POST SYSTEM 20 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE C21. P2 POST RETROFIT GUIDELINES, PART U

ADOT P2, FOUR POST SYSTEM 12 FT. OFFSET RETROFIT GUIDELINES 3500 -



AVERAGE DAILY TRAFFIC

PAKT V

NOTE: System 2- 4" Diam. Steel Pipe, Slip Base ADOT P2, FOUR POST SYSTEM 16 FT. OFFSET RETROFIT GUIDELINES Retrofit Optional Existing System Acceptable 3500 -3000 2500 - 0002 1500 -(DOLLARS) COZL

AVERAGE DAILY TRAFFIC

50000

45000

40000

5000

Ö

200

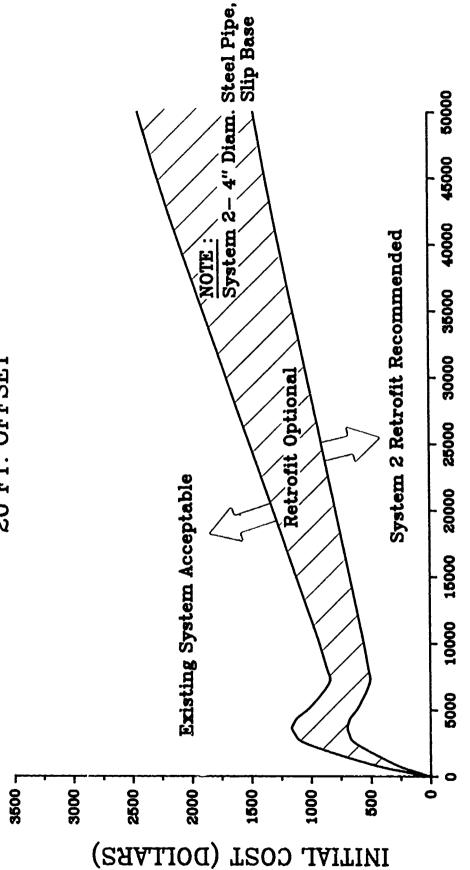
JAITINI

800

System 2 Retrofit Recommended

FIGURE C23. P2 POST RETROFIT GUIDELINES, PART W

RETROFIT GUIDELINES ADOT P2, FOUR POST SYSTEM 20 FT. OFFSET

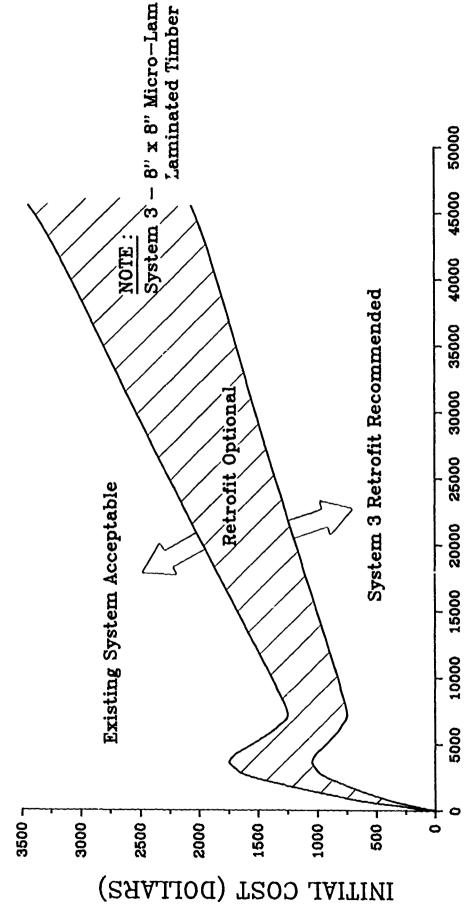


AVERAGE DAILY TRAFFIC

P2 POST RETROFIT GUIDELINES, PART X

FIGURE C24.

ADOT P2, FOUR POST SYSTEM 12 FT. OFFSET

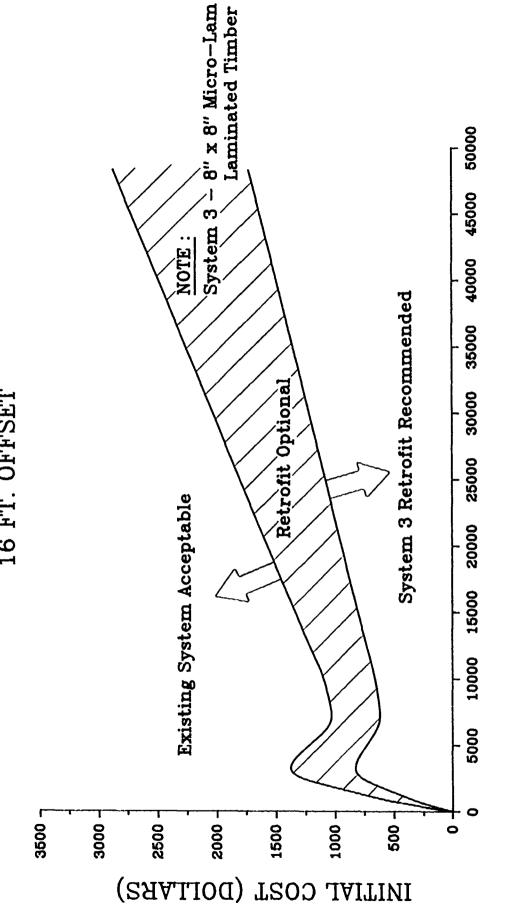


AVERAGE DAILY TRAFFIC

FIGURE C25. P2 POST RETROFIT GUIDELINES, PART Y

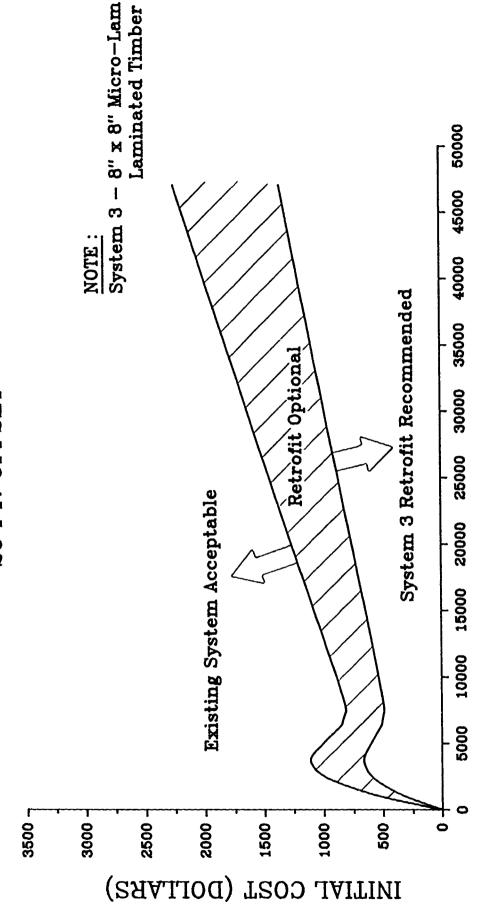
91

RETROFIT GUIDELINES ADOT P2, FOUR POST SYSTEM 16 FT. OFFSET



AVERAGE DAILY TRAFFIC

ADOT P2, FOUR POST SYSTEM 20 FT. OFFSET



AVERAGE DAILY TRAFFIC

FIGURE C27. P2 POST RETROFIT GUIDELINES, PART AA

APPENDIX D GUIDELINES FOR NEW INSTALLATIONS

NEW INSTALLATIONS

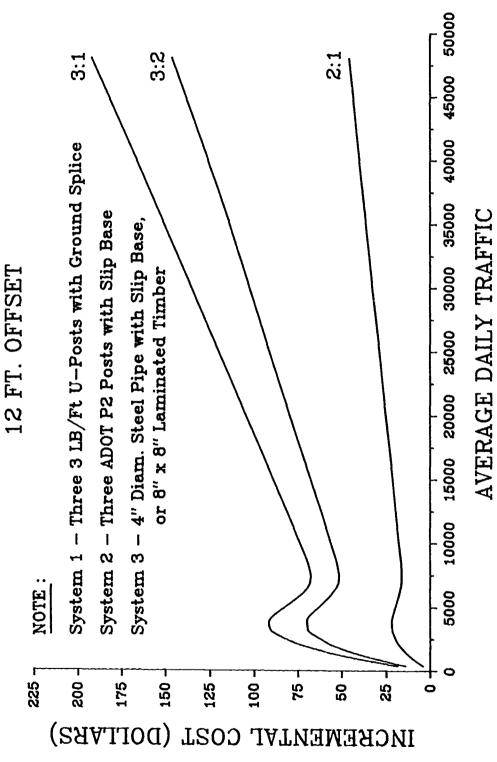


FIGURE D1. NEW INSTALLATION GUIDELINES, PART A

NEW INSTALLATIONS

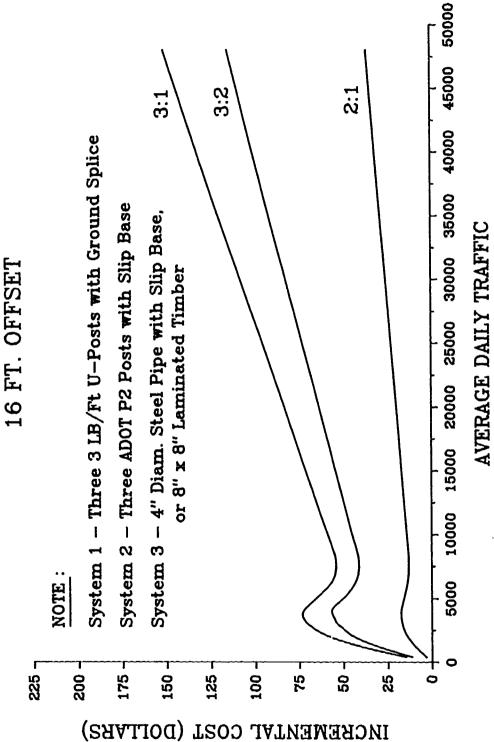


FIGURE D2. NEW INSTALLATION GUIDELINES, PART B

NEW INSTALLATIONS 20 FT. OFFSET

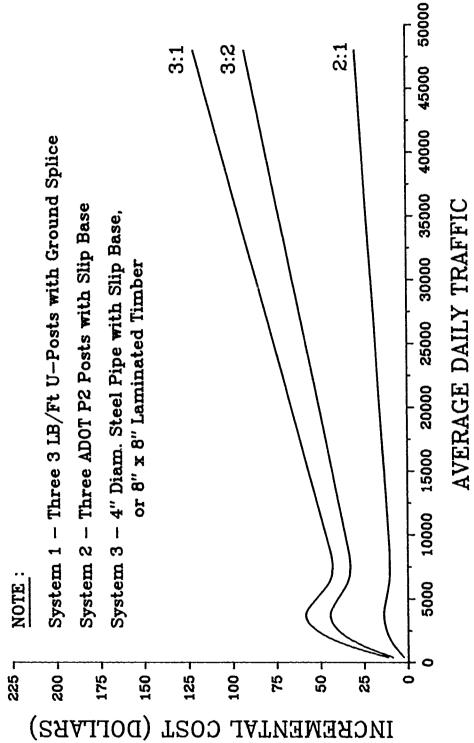


FIGURE D3. NEW INSTALLATION GUIDELINES, PART C