



# **EVALUATION OF OFF-RAMP RIGHT TURN CONTROL AT SINGLE POINT URBAN INTERCHANGES WITHOUT FRONTAGE ROADS**

Final Report 556

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## SI\* (MODERN METRIC) CONVERSION FACTORS

APPROXIMATE CONVERSIONS TO SI UNITS					APPROXIMATE CONVERSIONS FROM SI UNITS				
Symbol	When You Know	Multiply By	To Find	Symbol	Symbol	When You Know	Multiply By	To Find	Symbol
<b><u>LENGTH</u></b>					<b><u>LENGTH</u></b>				
in	Inches	25.4	millimeters	mm	mm	millimeters	0.039	inches	in
ft	Feet	0.305	meters	m	m	meters	3.28	feet	ft
yd	Yards	0.914	meters	m	m	meters	1.09	yards	yd
mi	Miles	1.61	kilometers	km	km	kilometers	0.621	miles	mi
<b><u>AREA</u></b>					<b><u>AREA</u></b>				
in <sup>2</sup>	square inches	645.2	square millimeters	mm <sup>2</sup>	mm <sup>2</sup>	Square millimeters	0.0016	square inches	in <sup>2</sup>
ft <sup>2</sup>	square feet	0.093	square meters	m <sup>2</sup>	m <sup>2</sup>	Square meters	10.764	square feet	ft <sup>2</sup>
yd <sup>2</sup>	square yards	0.836	square meters	m <sup>2</sup>	m <sup>2</sup>	Square meters	1.195	square yards	yd <sup>2</sup>
ac	Acres	0.405	hectares	ha	ha	hectares	2.47	acres	ac
mi <sup>2</sup>	square miles	2.59	square kilometers	km <sup>2</sup>	km <sup>2</sup>	Square kilometers	0.386	square miles	mi <sup>2</sup>
<b><u>VOLUME</u></b>					<b><u>VOLUME</u></b>				
fl oz	fluid ounces	29.57	milliliters	mL	mL	milliliters	0.034	fluid ounces	fl oz
gal	Gallons	3.785	liters	L	L	liters	0.264	gallons	gal
ft <sup>3</sup>	cubic feet	0.028	cubic meters	m <sup>3</sup>	m <sup>3</sup>	Cubic meters	35.315	cubic feet	ft <sup>3</sup>
yd <sup>3</sup>	cubic yards	0.765	cubic meters	m <sup>3</sup>	m <sup>3</sup>	Cubic meters	1.308	cubic yards	yd <sup>3</sup>
NOTE: Volumes greater than 1000L shall be shown in m <sup>3</sup> .									
<b><u>MASS</u></b>					<b><u>MASS</u></b>				
oz	Ounces	28.35	grams	g	g	grams	0.035	ounces	oz
lb	Pounds	0.454	kilograms	kg	kg	kilograms	2.205	pounds	lb
T	short tons (2000lb)	0.907	megagrams (or "metric ton")	mg (or "t")	Mg	megagrams (or "metric ton")	1.102	short tons (2000lb)	T
<b><u>TEMPERATURE (exact)</u></b>					<b><u>TEMPERATURE (exact)</u></b>				
°F	Fahrenheit temperature	5(F-32)/9 or (F-32)/1.8	Celsius temperature	°C	°C	Celsius temperature	1.8C + 32	Fahrenheit temperature	°F
<b><u>ILLUMINATION</u></b>					<b><u>ILLUMINATION</u></b>				
fc	foot candles	10.76	lux	lx	lx	lux	0.0929	foot-candles	fc
fl	foot-Lamberts	3.426	candela/m <sup>2</sup>	cd/m <sup>2</sup>	cd/m <sup>2</sup>	candela/m <sup>2</sup>	0.2919	foot-Lamberts	fl
<b><u>FORCE AND PRESSURE OR STRESS</u></b>					<b><u>FORCE AND PRESSURE OR STRESS</u></b>				
lbf	Poundforce	4.45	newtons	N	N	newtons	0.225	poundforce	lbf
lbf/in <sup>2</sup>	poundforce per square inch	6.89	kilopascals	kPa	kPa	kilopascals	0.145	poundforce per square inch	lbf/in <sup>2</sup>

SI is the symbol for the International System of Units. Appropriate rounding should be made to comply with Section 4 of ASTM E380

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# **EXECUTIVE SUMMARY**

## **INTRODUCTION**

Single point urban interchanges (SPUIs) have become an integral part of managing traffic at the critical connections between freeway and arterial roadway systems. Although studies and debates continue as to where and how they should be applied, they do not discount their continued application. Based on this more widespread use, finer aspects of their operation are being considered and studied. This study focused on the control of the off-ramp right turn movement at SPUIs without frontage roads. The objective of this research project was to evaluate the safety and efficiency of traffic control for off-ramp right turns. For the purposes of this project, two common forms of off-ramp right turn traffic control were investigated: signal control and yield control.

## **SCOPE OF RESEARCH**

The process followed during this research focused on two main aspects of the off-ramp right turn movement: safety and operations. The project was composed of the following stages:

- Literature Review:** A literature review was conducted to provide the research team a broader perspective on other studies concerned with this aspect of SPUIs. The review was looking for the various traffic controls and interchange configurations that could particularly affect the safety and operation efficiency of off-ramp right turn movement.
- Safety Analysis:** Long-term trends in crash occurrences and short-term observations of conflicts at six study sites (12 off-ramp locations) were analyzed. Crash rates and conflict rates were determined in order to compare and contrast the two means of assessing safety as well as how they relate to the type of the traffic control used at the off-ramps.
- Operations Analysis:** Detailed traffic data collected at the study sites was used to calculate actual delays for off-ramp right turn movements at the study sites. This field data was also used to conduct simulations of interchange which supplemented the calculations based on the limited sample of study sites. The simulation models provided a means of testing different combinations of off-ramp right turn control types and overall interchange conditions in order to determine the effects of signal and yield control.

## FINDINGS

The review of relevant literature and research shows that there is some attention devoted to the operation and safety of SPUIs specifically pertaining to the off-ramp right turn movement. The literature review also revealed that there does not appear to be any past or present research/studies investigating the advantages and disadvantages of using one form of control over another for the off-ramp right turn movement. Most of the information reviewed pertained to the advantages and disadvantages of free/uncontrolled off-ramp right turn movements versus some type of control (i.e., stop sign, yield, or signal). Key concepts relating to the types of off-ramp right turn control that were discovered during the literature review and considered throughout the research included the effect of nearby downstream intersections, pedestrian/bicyclist activity at the interchange, increased clearance intervals with signal control, and other issues further discussed within the report.

The data collection effort and details obtained from observations and research allowed for actual calculations to be made concerning operations and safety. Interpretation of that data through the results of the calculations lends itself to determining interchange characteristics that influence operations and/or safety, but is subject to the limited number (6) of study interchanges evaluated. Qualitative observations and conclusions regarding the operations and safety of the study interchanges are presented within this report.

Delays, conflict rates, and crash rates were calculated from the data and observations at the six study sites. Average delays for off-ramp right turn vehicles at signal-controlled locations experienced about 20% to 30% more delay than the vehicles at locations with yield control. The overall conflict rates for the control-type groups were based on a recalculation of the conflict rate using the summed values for each sample site. An overall average of the crash rates calculated for each site was not deemed appropriate given the variability inherent to conflict observations based on the relatively short observation period as compared to crash rate calculations. The average conflict rate for the yield-controlled sites as a group is about 240% greater than the average rate for the signal-controlled group, but the yield-controlled sites have considerable variability in their rates. A statistical t-test indicates that because of this variability and despite the large difference in average rates, there is no significant difference ( $t_{\text{calc}} = 1.705$ ,  $t_{.05, v=10} = 1.812$ ) in the average conflict rates between the control groups. Overall crash rates for the control-type groups were the averaged values of the three-year average crash rate for each site in the group. The average crash rate for yield-controlled sites as a group is almost double the average crash rate for the signal-controlled sites. This ratio is comparable to the conflict rate relationship between the two groups. A statistical t-test was performed on the average crash rate data for the yield-controlled sites and the signal-controlled sites. All crash rates were considered, which resulted in no significant difference ( $t_{\text{calc}} = 1.510$ ,  $t_{.05, v=10} = 1.812$ ) in the average rates for each group.

The actual field data from the limited sample of study interchanges was supplemented with model simulation results that considered four control type scenarios—two variations on signal control and two on yield control. The signal control variations concern the

allotment of signal phasing to the off-ramp right turn traffic. One version only gives a green arrow indication to the off-ramp right turn movement during the adjacent cross street left turn phase. This was referred to as “Signal 1-phase” within this report. The other variation of the off-ramp right turn signal control type is when there are two phases that can provide the green arrow indication for the off-ramp right turn movement. This control variation is referred to as “Signal 2-phase” in this report.

The yield control type was split into two versions incorporating vehicle presence detection or just the standard yield sign with no vehicle detection. The off-ramp right turn control that uses yield signs and vehicle detection works similarly to the Signal 1-phase control, but without the signal head indications for the off-ramp right turn vehicles. Essentially the off-ramp right turn traffic would be acting as pseudo cross street left turn traffic. In this report, this control type is called “Yield With Detection.”

An iterative analysis process involving a range of off-ramp and interchange volume conditions was used to determine overall operational effectiveness of each control scenario. Data collected at several SPUI sites was used to calibrate a micro-simulation model (CORSIM) that was then used to evaluate numerous combinations of traffic volume conditions and off-ramp control types that would have not been possible to collect at actual SPUI locations. The results of the simulations were used in concert with the safety evaluation and conclusions to develop suggestions on appropriate control types for the off-ramp right turn movement.

The results indicated that in almost all volume scenarios, the “Yield Without Detection” control type (the basis for the comparisons) has the lowest overall interchange control delay. When comparing averaged interchange control delays, the other control type variations resulted in more delay. In the scenarios with one off-ramp right turn lane, the overall interchange delay for the “Yield With Detection” and “Signal 1-Phase” were not much greater (about 4 and 9 % more, respectively). The differences in interchange delay were more prominent in the two-lane off-ramp right turn scenarios due to modeling constraints, which caused the left hand lane of the two lane off-ramp right turn to experience more delay than necessary in the scenarios with signal control. Therefore, the magnitudes of the percent differences for the signal control types in this two-lane group of scenarios are exaggerated, yet they still reflect the same general relationship as the one-lane group of scenarios. Also, note that these percent differences apply for the normal ranges of interchange volumes and turning movements used in this project. Unusual situations may result in different results for each control type.

The efforts executed during this project had the goal of determining which control type would be best to use for off-ramp right turn movements at single-point urban interchanges without frontage roads. The data collected, both in the field and through the crash databases, were very detailed, beneficial, and used to their fullest. However, despite the efforts and underlying goal, the results from the safety and operations analyses appear to be contrary making it necessary to compare the two aspects using a common basis. Safety and operation can be measured in the common term of cost. Estimates of the overall yearly costs of operations and crashes associated with the off-

ramp right turn movement at yield and signal-controlled site were computed as a final means of determining the best control type.

The crash cost for each interchange is calculated from the number of crashes associated with the off-ramp right turn movement only. Thus, the total crash cost values are not representative of the total crash costs per interchange, but are valid for use in the comparison against interchange operational costs since the unknown crash cost component is assumed to be equal for all the interchanges. The costs are composed of several factors: medical costs, property damage loss, lost productivity (market and household), and other related costs. The average costs for crashes involving property damage only was \$4,812 (in 2004 dollars). Crashes involving injuries of varying degrees have an average cost of \$49,817. Crashes with any fatalities, which are about 75 times less likely to occur as other injury crashes, have an average cost of \$1,184,885 associated with them. The average yearly cost of crashes for the study interchanges, grouped by off-ramp right turn control type, indicates that interchanges using yield control for the off-ramp right turn movement are about \$384,000 (2004 dollars) more costly than the interchanges using signal control.

The user cost aspect considered in this project was the “value of time” (user delay costs), which accounts for a majority of the user costs in this project’s comparison of the control types for off-ramp right turn movements. The value of time is a function of the average hourly wage earned by the persons impacted by the delays (separated by passenger vehicles and trucks), the percentage of the hourly wage considered as the value of time (50% for passenger vehicles, 100% for trucks), and the average passenger occupancy (1.5 for passenger vehicles, 1.05 for trucks). The average yearly cost of delay for the study interchanges, grouped by off-ramp right turn control type, indicates that interchanges using signal control for the off-ramp right turn movement are about \$689,000 more costly.

For use in this comparison only, the total average yearly costs (crash costs + delay costs) for interchanges using signal control for the off-ramp right turn movement is estimated at \$2,100,000. Interchanges that have yield control for the off-ramp right turn movement have an average yearly cost estimate of \$1,800,000. Despite yield control sites appearing to have higher crash rates (although not statistically significant), their overall savings in user cost of delay offsets the increased costs of crashes. However, the difference in total costs does not appear to be substantial, at least not to a degree where the selection of a certain control type would be more convincing than the other.

# **CHAPTER 1**

## **LITERATURE REVIEW: OFF-RAMP RIGHT TURN CONTROL AT SINGLE POINT URBAN INTERCHANGES (SPUI) WITHOUT FRONTAGE ROADS**

### **OVERVIEW**

Although there are extensive studies concerning the effectiveness of single point urban interchanges (SPUIs), especially when compared to other interchange designs, most of this research has focused on the overall operation and safety of the interchange types. However, this investigation did not locate any past or current research specifically focused on the traffic control of the right turn movement from the major roadway associated with the SPUI and how it relates to operation and safety. The literature review did discover there are limited publications guidelines and protocols for how this movement should be controlled in specific conditions.

The SPUI has a unique characteristic, as compared to some other interchanges or intersection designs, where the major roadway right turn movement (hereafter referred to as the “off-ramp” right turn) can be accommodated by a dedicated right turn lane (or lanes) that could be operated without any traffic control (e.g., stop, yield, or signal). In this particular case, the off-ramp right turn is merged into the cross street traffic via a separate additional lane on the cross street. *NCHRP Report 345: Single Point Urban Interchange Design and Operations Analysis* by Messer, et al [1] found in its 1989 field survey that only about 25% of the SPUIs were designed to accommodate a “free” off-ramp right turn movement with a separate acceleration lane along the cross road. This layout for the off-ramp right turn is usually permissible based on the interchange operations, but is not always feasible. The report also states that right turns from the off-ramps are operationally more complex and typically have less capacity per lane.

Without a “free” (uncontrolled) situation, the off-ramp right turn movement has to be governed by some form of traffic control. The most common means of traffic control in these situations are stop control, signal control, merge (with yield), or yield control, which is the most prevalent [1]. The merge-type control is similar to the free right turn discussed above except that a separate additional lane is not provided to receive the off-ramp right turn traffic—instead a short acceleration or drop lane is provided necessitating a yield condition at the merge point. Stop control, yield without a merge situation, and signal control are typically implemented at the point where the right turn lane (or curvature of the right turn lane) starts to intersect with the cross street travel lanes. Yield control and signal control are the focus of this literature search and research as a whole.

## **OPERATIONAL ASPECTS**

Several of the sources examined in this review provided information on off-ramp right turn control as it related to operational characteristics and effects. Much of the information focused on the advantages and disadvantages of a free/uncontrolled off-ramp right turn versus a controlled situation (e.g., signal or yield/stop control). Although this particular interest is different from the purpose of this research, it does provide some insight into the benefits of one control type over another.

### **General Characteristics of Off-Ramp Right Turns**

There are several components to the design and operation of the off-ramp right turn movement that are independent of the type of traffic control employed. NCHRP 345 [1] points out a few of these. Geometrically speaking, some overall characteristics that affect off-ramp right turn operations are the magnitude of the turn radius, the presence of an auxiliary acceleration lane at the end of the turn, and whether the off-ramp right turn lane is exclusive. Larger turn radii can promote better off-ramp right turn operations, but at the cost of making the movement more complex and requiring more space. Locations where the off-ramp left turns and right turns do not have exclusive lanes will be inefficient due to the difference in traffic controls (i.e., the respective turn lane queues may block one another), as well as when both movements are signalized.

NCHRP 345 [1] mentions some factors that determine how well an off-ramp right turn movement operates, what its capacity limit is, and its safety. The characteristics include the geometry of the turn path, complexity of the entrance maneuver, capacity of the maneuver, and type of traffic control in place. The report continues by stating, “[the] right turn maneuver is significantly affected by the type of traffic control, e.g., stop, yield, etc., the number of conflicting signalized movements, and the signal timing of the conflicting movements.” (p. 24)

The complexity of the entrance maneuver can affect the efficiency and safety of the off-ramp right turn operations. One point of complexity involves the off-ramp right turn driver’s perception of potential conflicting traffic. Due to the signal phasing used at SPUIs, off-ramp right turn traffic is faced with alternating sequences of high and low traffic flows where they enter the cross street. This is not all that uncommon at interchanges/intersections, but the distances related to a SPUI layout complicate the decision for the driver. Another characteristic mentioned in NCHRP 345 [1] that complicates the off-ramp right turn movement is the angle of entry and physical requirements necessary to confirm a safe point to enter the cross street traffic stream.

The capacity of an off-ramp right turn movement is dependent on the type of traffic control used. According to NCHRP 345 [1], if a stop or yield control is in place, the off-ramp right turn capacity is dependent on the availability of gaps in the conflicting traffic stream (with most of them being generated artificially by the overall SPUI signal operations). Capacity at signal controlled off-ramp right turn movements is based on the portion of the overall SPUI signal cycle length devoted to the off-ramp right turn movement plus available gaps for right turn on red.

## Free/Uncontrolled Off-Ramp Right Turns

The *California Single Point Interchange Planning, Design, and Operations Guidelines* [2] mentions off-ramp right turn movements with free control. The *Guidelines* claims that “free right turn moves at the exit ramps are a basic feature of the typical SPI [i.e., SPUI]. Lack of a free right can negatively impact operational efficiency.” (p. 9)

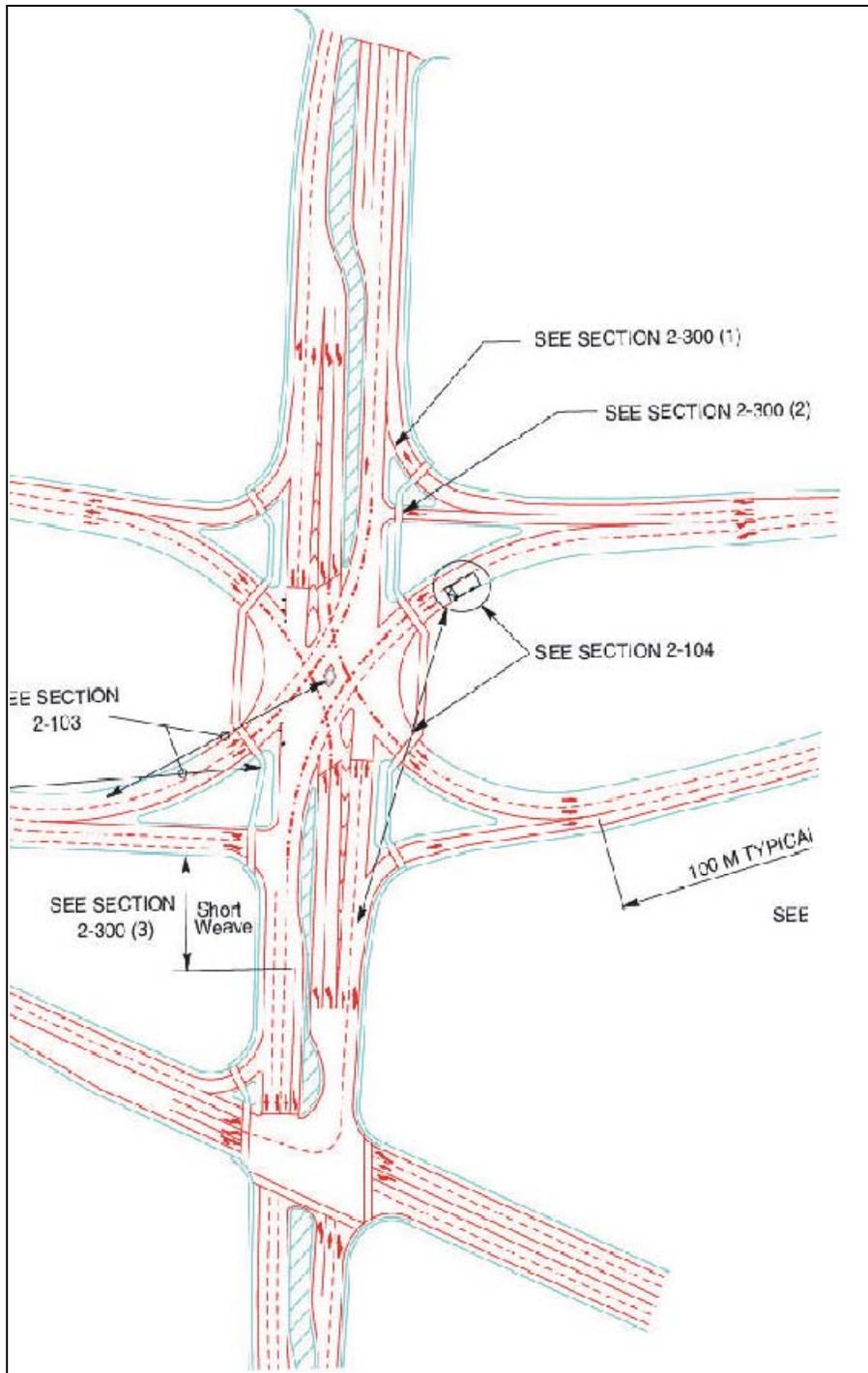
California views the use of SPUIs (SPIs) as a means to move large volumes of traffic, and therefore they should be designed to allow for free right turns when possible. This preference is reiterated in the *California Highway Design Manual* [3] where it states in Index 504.3(2):

“Where a separate right turn lane is provided at ramp terminals, the turn lane should not continue as a free right unless pedestrian volumes are low, the right turn lane continues as a separate full width lane for at least 60 m [200 ft] prior to merging, and access control is maintained for at least 60 m [200 ft] past the ramp intersection. Provision of the free right should also be precluded if left turn movements of any kind are allowed within 125 m [410 ft] of the ramp intersection.”

Despite this foundation of design philosophy, the *Guidelines* also mentions that “often free right turn moves at exit ramps can not be provided due to close proximity of adjacent intersections.” (p.3) Close proximity of downstream intersections would not allow for sufficient weave and merge lengths with a free right turn from the off-ramp.

*A Policy on Geometric Design of Highways and Streets* by AASHTO (the “Green Book”) [4] provides further support for the use of free off-ramp right turns. On pages 748 and 787 the Green Book states “all right turns into and out of ramp approaches are generally free flow...and only the left turns must pass through the signalized intersection.” The Green Book also provides guidance on when free off-ramp right turns should be implemented, “the design of the free right turns should include an additional lane on the cross street beginning at the free right-turn lane for at least 60 m [200 ft] before being merged. Free-flow right turns from the exit ramp to an arterial cross road are not desirable when the nearest intersection on the cross road is within 150 m [500 ft] because there may be inadequate weaving distance between the exit ramp and the adjacent intersection.” The California *Guidelines* [2] criteria are quite similar with the additional criterion of access control being maintained for at least 200 feet beyond the ramp intersection. The Green Book still accounts for the possibility of the off-ramp right turn being a controlled movement despite the details pertaining to free right turn situations.

The *Minnesota Department of Transportation Roadway Design Manual* [5] also is a proponent of free off-ramp right turn movements. It states that “left and right turn movements at single point diamond interchanges (SPDI) should be physically separated, and moreover allow the right turns to flow independent of the signal.” (p. 6-1(3)) The basis for this statement is that any portion of the signal cycle length devoted to the off-ramp right turn movement increases the overall interchange delay.



**Figure 1. Common Right Turn Lane Configurations at Exit Ramps**  
 (California Single Point Interchange Planning, Design, and Operations Guidelines [2])

NCHRP 345 [1] states that “in general, the right-turn maneuver will operate more safely and efficiently if a right-turn bay and auxiliary lane are provided” (p. 24) because the traffic flows are physically separated. However, the design guidelines presented in the report state that “an acceleration lane for off-ramp traffic onto the cross arterial is not necessarily recommended unless sufficient distance (greater than 1,200 feet) is available to the next downstream [signalized] intersection. Direct entry merging for this maneuver provides good operation in restricted designs.” (p. 99)

### **Controlled Off-Ramp Right Turns**

Despite the emphasis placed on free off-ramp right turns by the preceding sources, the same sources as well as others provide some detail pertaining to controlled off-ramp right turns. Primarily, the controlled movement aspect is concerned with signalization, although some discussion is provided as it relates to yield and stop control types.

#### *Signal Control*

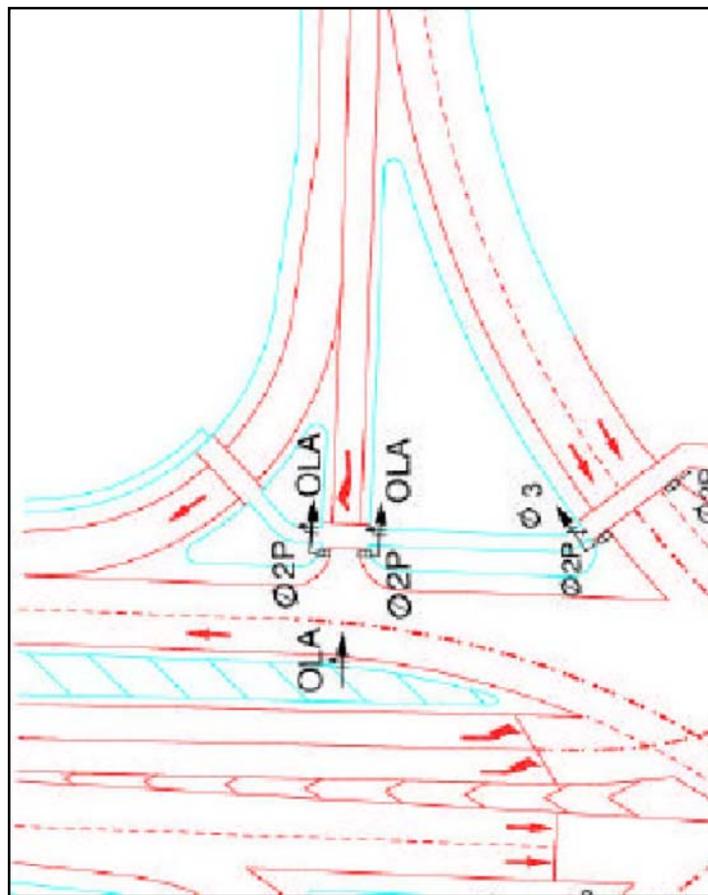
The California *Guidelines* [2] qualify its preference for free right turns with the provision that when volumes are too high for one exit ramp right turn lane it is sometimes reasonable to add and signalize another exit ramp lane exclusively for right turn movements. This situation, as well as other approaches to off-ramp right turn movement control in California, is shown in Figure 1 as Item 2-300(2).

The *Guidelines* also contends “in some situations this configuration of a combination free right/signalized right turn layout can mitigate short weaves and merges related to close spacing of the ramp and adjacent local intersections.” (p. 10) According to the *Guidelines*, signalization of the off-ramp right turn is considered when the spacing between the ramp and the adjacent intersection is too short and/or there is a large proportion of right turn traffic from the exit ramp attempting to weave across the cross street to turn left at the adjacent intersection. This situation is depicted in Figure 1 as Item 2-300(3).

Page 113 of the *NCHRP Report 420: Impacts of Access Management Techniques* by Gluck, et al [6] notes that signalization of the off-ramp right turn can be used to alleviate (to some degree and dependent on progression considerations) congestion at downstream signals sometimes caused by free or yield-controlled off-ramp right turns. The signalization of the off-ramp right turns also can assist motorists with shorter weave/merge lengths or to accommodate a heavy left-turn demand at the downstream location. The report also cautions that the signalization of the off-ramp right turns may cause an increase in the queue length, which must be minimized to avoid spillback onto the freeway mainline. The AASHTO Green Book [4] also provides this same advice, but applies it to possibly blocking access to the off-ramp left turn lanes (or through movement if the SPUI has frontage roads). The Utah Department of Transportation (UDOT) currently has a project in design at this time to signalize most of these off-ramp right turns in Salt Lake County [7]. UDOT cites problems with traffic queues extending back onto the mainlines. They feel that replacing their current stop controls (they do not have yield control) with signal control will allow for traffic to still turn right after

stopping when the signal is red, but will more importantly “flush out” the traffic queue via a green signal indication when no conflicting movements are operating.

When signal control is utilized, the California Guidelines [2] states that right turns on red should be allowed when practical and should have a sign stating they are allowed or not allowed. According to the Guidelines, the use of the sign “will reduce the risk of driver confusion on the nature of this movement and in enforcement.” The typical location of the off-ramp right turn movement signals is shown in Figure 2 (Note: “OLA” refers to the phasing being an overlap of the corresponding cross street left turn phase). The Guidelines also points out that U-turns from the cross street are not allowed in this situation since any U-turns would conflict with the off-ramp right turn movement phase that is overlapped with the cross street left turn movement.



**Figure 2. Signal Head Placement for Exit Ramp Right Turns (California Single Point Interchange Planning, Design, and Operations Guidelines [2])**

It is interesting to note that the California *Guidelines*, as shown in Figure 2, depicts the signalization of the off-ramp right turn movement as a separate lane group apart from the “free” off-ramp right turn movement. One possible reason for this design relates to promoting an efficient operation of the signalized movement and safety of the vehicles involved. By having the signalized off-ramp right turn movement intersect the cross street

at a right angle, the sight distance is not affected by the curvature of the typical off-ramp right turn lane layout. The *Guidelines* suggests that interchange/off-ramp right turn operations are affected by inadequate sight distance because “if drivers in a queue cannot see approaching vehicles, each driver may tend to slow and creep into the intersection, thus reducing the capacity of the ramp and hindering the operation of the intersection.” (p. 8)

NCHRP 345 [1] notes that signalized off-ramp right turn movements tend to work quite efficiently during their green phase, but revert to stop-and-go situations for the red phase. During this portion of the signal cycle, the flow rate for the operation is much lower, which highlights the driver’s need to verify safe gaps to enter the cross street. Based on observations presented in the report, off-ramp right turns controlled by signals “appeared to operate about as efficiently as yield control.” (p. 27) The off-setting efficiencies of the movement during the green and red phases were cited as the reason.

There is a method of addressing insufficient off-ramp right turn capacity without resorting to signalization as detailed in NCHRP 345 [1]. Since the off-ramp right turn movement does not have a “parent” phase to provide a protected entry, sometimes the off-ramp right turns will not have adequate yield-entry merging capacity during high-volume conditions. Usually this will only occur at SPUI sites with only one lane devoted to the off-ramp right turn movement. The report describes the use of a queue detector, located in the off-ramp right turn lane with yield control that is connected to the adjacent (i.e., overlapping) cross street left turn phase:

“This delayed-call queue detector should be located perhaps 50 feet upstream from the stop line (to detect the presence of the second or third vehicle stopped in queue). A delayed call of perhaps 6 seconds would be adequate for a normal 6-foot by 6-foot inductive loop detector design. If the queue remains over the loop for 6 seconds or more during the cross street left turn red, a call is placed for the left turn phase to provide ‘protected’ right turns. If the left turn phase is already green, the ‘delay inhibit’ or defeat feature of the detector-controlled system should be used to turn off the delay feature during green, so that the right turn calls are immediately recognized to extend the cross-street left turn phase until gap out. These features will provide additional movement capacity only when needed by just monitoring the queuing status of the right turn. Single vehicles stopping in line to make a right turn will still enter under yield control.” (p. 70)

The Design Guidelines presented in NCHRP 345 [1] state that “signalizing the off-ramp right turn operations should be avoided. Delayed-call right turn queue detection should be provided for high-volume conditions having fairly balanced traffic patterns. Right turn volumes from the off-ramp exceeding the complementary cross street volume by 100 vehicles per hour per lane, vphpl, should warrant this detector treatment when the right turn volume exceeds 300 vphpl.” (p. 99)

## *Yield Control*

NCHRP 345 [1] provides many of the details pertaining to yield controlled off-ramp right turn movements. It states that yield control “has the advantages of being relatively efficient in terms of traffic performance and right-of-way need.” (p. 26) The main reason for its efficiency is because it only requires the off-ramp turn traffic to stop when it cannot safely enter the cross street traffic stream. Therefore, the movement is able to make maximum use of opportunities to enter with a minimum amount of delay. The capacity of an off-ramp right turn movement under yield control is highly sensitive to the amount of conflicting traffic. Later in the report, the following statement is made, “observation...suggests that yield control for the off-ramp right turn movement can be an efficient and cost-effective control mode.” (p. 27)

## **SAFETY ASPECTS**

The method of controlling the off-ramp right turn movement at SPUIs can also affect the safety of the interchange. Several sources offered information supporting certain types of off-ramp right turn control from a safety perspective. The safety concern highlighted in the literature usually is associated with pedestrians and bicycles, but the type of off-ramp right turn control can also affect vehicular safety.

## **PEDESTRIAN & BICYCLIST SAFETY**

Pedestrians and/or bicyclists attempting to cross the off-ramp approach of a SPUI are faced with unique conditions which warrant particular attention to ensuring that there is a mutual understanding of the traffic situations by both the driver and the pedestrian/bicyclist. The off-ramp right turn movement is of particular concern due to this being one of the first points of potential conflict at the interchange.

The AASHTO Green Book [4] points out that heavy pedestrian traffic can diminish the desirability of free right-turn lanes by adding a potential conflict with non-controlled vehicular traffic. This situation is of particular concern when the off-ramp right turn lane(s) are curved in such a way as to promote a speed sufficient for merging with the cross street and yet obscure the intervisibility between the driver and pedestrian. *NCHRP Synthesis of Highway Practice 139* [8], which provides general information regarding expressway ramps intersecting local streets, states that, “...vehicles are still traveling at a relatively high rate of speed when they pass through the intersection or merge with surface street traffic.” (p. 38) The report continues by indicating motorists also may be unaware of pedestrians because they are focused on looking for upstream traffic. This behavior would probably be evident regardless of the traffic control in place since the driver is either anticipating a gap for a right turn on red (or at a stop control) or timing a gap for a yield or free right turn/merge situation. Based on this situation, NCHRP 139 [8] also states that “...pedestrian safety can be severely threatened at intersections where freeway off-ramps intersect with local streets, because of the high-speed traffic mixing with crossing pedestrians.” (p. 39)

The report followed up on this idea with the following:

Situations where high-speed expressway ramps intersect with local streets were identified as having lessened adverse effects when:

- pedestrian volumes and local traffic volumes are relatively low and good roadway designs are used
- suitable traffic control devices are used at the local street and/or grade separation (where appropriate)

The conditions listed as possibly harmful include:

- High traffic volumes and/or speeds on the off-ramp
- Moderate to high pedestrian volumes crossing at the intersection
- Insufficient traffic controls at the intersection (e.g., off-ramp traffic controlled by yield signs only)
- High-speed traffic on ramp having poor sight distance and/or an unexpected intersection

The conclusions drawn from the report suggest that the hazards to pedestrians can be mitigated by using proper intersection design, utilizing grade separation, and/or implementing adequate traffic control devices (e.g., signals and signs). The effects of these items are reductions in vehicle speeds and increased pedestrian/motorist awareness. NCHRP 345 [1] suggests that signaling the off-ramp right turn movements would reduce the capacity of the SPUI as a whole. Also, the capacity of the off-ramp right turn movement would be similar to that of a yield-controlled movement because the increased efficiency of operation during the green phase is partially offset by the reduced efficiency during the red phase. Furthermore, the report mentions observations from its associated field study which showed “pedestrian behavior...indicated that pedestrians were able to cross the ramp junctions safely and with little confusion as to when it was safe to cross during the cycle.” (p. 32)

The California *Guidelines* [2] had some limited safety information concerning bicyclists. The *Guidelines* promotes only one lane being dedicated as a free right turn from the exit ramp “so bicyclists need to cross only one lane of uncontrolled traffic.” (p. 11) Also, the use of stop control for the exit ramp right turn traffic is mentioned as a means of adequately accommodating bicyclists in some situations. Furthermore, the *Guidelines* states that if an exit ramp right turn lane is anticipated to be signaled in the future or if the SPUI is larger than a “compact” SPUI as defined by the *Guidelines*, then a separate bicycle facility (i.e., overpass or underpass) should be incorporated into the SPUI design.

### **Vehicular Safety**

None of the literature sources reviewed had specific information pertaining to the crashes associated with the off-ramp right turn movement. Data and conclusions pertaining to the off-ramp as a whole were evident. The *Minnesota Department of Transportation Roadway Design Manual* [5] states “the predominant crash type at SPDI [SPUI] is rear-

end crashes on the off-ramp.” (p. 6-1(5)) This conclusion is further supported by the Cheng article, “*Accident Analysis for Single Point Urban Interchange*” [9] which states the predominant type of crash is rear-ends on the off ramps with a reported percentage of at least 40%. This paper advises that improvements in advance warning signs, visibility, location of signal and stop bar, and skid resistance could reduce off-ramp rear-end crashes.

The radius of the off-ramp right turn lanes also contributes to the safety of the movement. NCHRP 345 [1] found that almost all stop, yield, and traffic-signal controlled off-ramp right turn movements had radii of less than 100 feet. Radii of this size or smaller promote better visibility for off-ramp right turn motorists as they look back to their left to assess cross street traffic conditions. However, the assessment of potential vehicular traffic conflicts complicates any off-ramp right turn movement regardless of turn radius or traffic control (except possibly free right/merge). The report emphasizes this with the following statement, “...the greater distance and unique phasing create a complex flow pattern by releasing a second platoon a few seconds after the through phase. This second platoon may surprise right turning drivers who expect to enter freely after the end of the cross-road through phase.” (p. 27) The origin and sequencing of the conflicting traffic streams is not consistent with the expectancy of a driver making the off-ramp right turn maneuver. This could be the basis for the right-angle and rear-end collisions associated with the off-ramp right turn. One form of mitigation would be to separate the entry point farther from the interchange via a merge control. Usually, this is not feasible due to space constraints and/or the proximity of a downstream signalized intersection where left turns are permitted.

The California *Guidelines* [2] focuses on safety by describing desirable visibility conditions. The *Guidelines* promotes intervisibility and claims that this will improve safety conditions and operational conditions.

## CONCLUSIONS

The literature and research documented above show that there is some attention devoted to the operation and safety of SPUIs specifically pertaining to the off-ramp right turn movement. The literature also revealed that there does not appear to be any past or present research/studies investigating the advantages and disadvantages of using one form of control over another for the off-ramp right turn movement.

With regards to operational/design effects, this research paper should focus on several key points. The intersection downstream of the off-ramp right turn movement is important to the selection of the traffic control used at the off-ramp right turn. The information reviewed showed that free right turns are a common practice, but are constrained by the downstream intersection location. Signal control at the off-ramp right turn can “meter” the off-ramp right turn traffic and help with shorter weaving distances and congestion at the downstream intersection. NCHRP 345 [1] promotes a distance between the SPUI and the downstream intersection that provides enough room to store stopped cross street traffic as well as provide additional room to accommodate lane

changes/weaving in advance of the stopped cross street traffic. The report recommends a desirable downstream signalized intersection separation of at least 1,200 feet from the off-ramp entry point. Spillback from a close downstream signalized intersection can affect the efficiency and safety of the off-ramp right turn movement.

The information reviewed described situations where pedestrian, bicyclist, and motorist safety can be affected by the type of control used for the off-ramp right turn movement. Other factors such as geometric design, sight conditions, pedestrian/bicyclist activity, and vehicle speeds also play significant roles, but the traffic conditions in which these all interact can be exacerbated or enhanced from a safety perspective based on the control type in place for the off-ramp right turn movement.

The review of information also indicated some concepts that will assist in the evaluation tasks of this project. Most of the information from the research papers by Follmer and Janson [10] and Bonneson [11] concern the evaluation of signal operations at SPUIs. For instance, the Follmer/Janson paper proposes an alternative to using the simple Highway Capacity Manual (HCM) estimate for right turn on red (RTOR) capacities. The concept is that a motorist attempting to turn right on red at a signalized intersection from an exclusive right turn lane will encounter similar conflicting traffic flows to a motorist attempting to turn right at an unsignalized intersection.

Another concept related to SPUIs with signalized off-ramp right turn movements is clearance time. The Bonneson paper [11] defines the clearance interval as the “interval [that] follows the yellow warning interval at the end of each signal phase. It is intended to provide sufficient time for those vehicles entering during the yellow to safely clear the intersection conflict area before the start of the next phase.” (p. 11) When the off-ramp right turn movement is signalized, essentially the interchange has “grown” to incorporate a larger conflict area. Thus, the clearance time has to be longer based on this increased potential conflict area. As Bonneson [8] puts it, “Longer clearance intervals lead to longer delays for the motorist because all-red time represents time that is not available to serve traffic demand.” (p. 6) This important point is emphasized in NCHRP 345 [1] which claims this situation as a “major disadvantage of signal control for the off-ramp right-turn movement.” (p. 27) However, this facet of SPUI operation does not preclude the use of a signalized off-ramp right turn movement; it merely means the “...designer’s goals should be to minimize the length of the clearance paths while still providing a geometric design that meets or exceeds minimum design standards.” [11]



## **CHAPTER 2**

### **EXISTING CONDITIONS AT STUDY SITES: OFF-RAMP RIGHT TURN CONTROL AT SINGLE POINT URBAN INTERCHANGES (SPUI) WITHOUT FRONTAGE ROADS**

#### **INTRODUCTION**

This study is concerned with the evaluation of off-ramp right turn control options at single point urban interchanges (SPUIs) without frontage roads. The off-ramp right turn control employed at a SPUI can affect the interchange as a whole as well as the specific off-ramp right turn movement. This study is concerned with operational efficiency (also referred to as “operations” within this report) and safety. The analysis of each component will be compared on equal terms in order to determine the advantages and disadvantages of certain off-ramp right turn traffic controls. The first step in this evaluation process is the data collection effort.

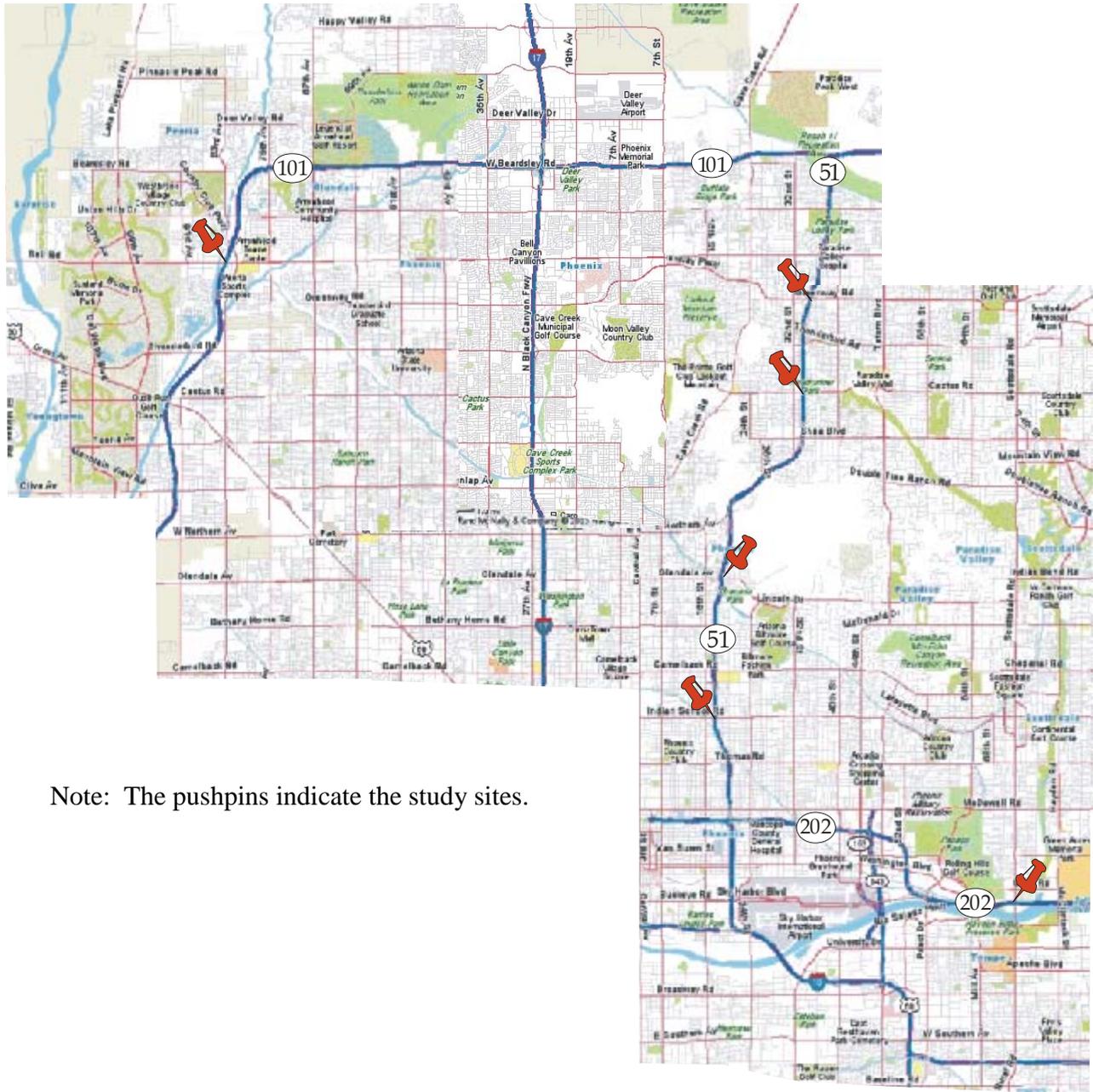
Ideally traffic data relating to volumes, operations, and safety would be readily at-hand for any number of subject sites. Without this luxury, some concessions had to be made in order to conduct this study. The number of interchange sites to be studied was limited by the funding available with acknowledgment that the more sites that were studied, the more useful and applicable the information would be. In order to supplement this constraint, the data collection effort was geared towards providing information that could be used to calibrate a micro-simulation model (CORSIM) that could then be used to evaluate a myriad of hypothetical SPUIs with varied traffic volumes/distributions and off-ramp traffic controls. Although these interchanges technically would not exist, their operation and subsequent evaluation would be a derivative of actual data collected as described in this chapter

#### **DATA COLLECTION EFFORT**

The data collection activities were related to the two main aspects being evaluated in this study: off-ramp right turn operations and safety. All operational data were collected in-field over the course of several weeks in early 2004. Some of the in-field safety data were obtained through engineers’ observations and recordings, but a significant portion of the safety-related data was from historical crash records. The following subsections describe the data collection process while subsequent sections report findings and calculations based on the data obtained.

#### **Study Site Selection**

Six SPUI sites were selected for study in this research project. There were several criteria that controlled which sites would be viable. First and foremost, the SPUI had to be a “three-phase” (referring to the signal phasing necessary) configuration meaning it did not have frontage roads incorporated into its operation. The second criterion was that



Note: The pushpins indicate the study sites.



Figure 3. Study Sites

the SPUI had to have sufficient crash history data available, i.e., be fully operational for at least three years. Applying these two criteria resulted in 17 potential sites in the Phoenix metropolitan area. The next level of filtering was based on the type of off-ramp right turn control used at the potential sites. Five of the seventeen sites had signalized off-ramp right turn controls, the remainder used yield control for the off-ramp right turn movement. The final selection of the six study sites was determined by the technical advisory committee (TAC) which relied on lane configuration information, pedestrian/bicycle activity, and local knowledge of the interchanges. The resulting study sites listed below provide a mixture of operation types and configurations commonly found in the Phoenix area:

- State Route 51 (SR 51) & Indian School Road
- State Route 51 (SR 51) & Glendale Avenue
- State Route 51 (SR 51) & Cactus Road
- State Route 51 (SR 51) & Greenway Road
- Loop 101 (Agua Fria Freeway) & Bell Road
- Loop 202 (Red Mountain Freeway) & Rural (Scottsdale) Road

Figure 3 shows the general location of the interchanges while Figures 4 through 9 are aerial photographs of each interchange. Table 1 shows the pertinent characteristic data for each interchange.

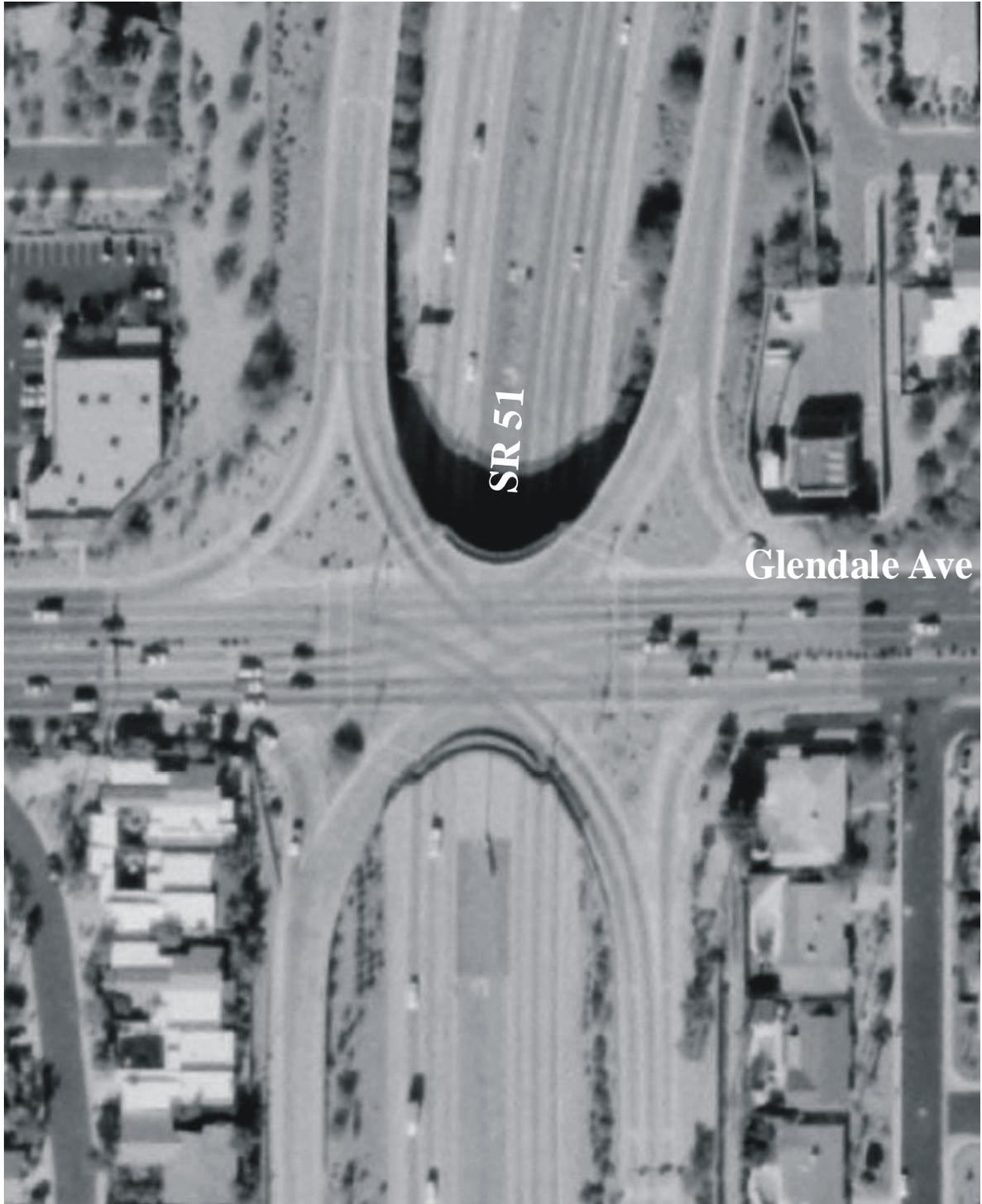
There are some important aspects to keep in mind when reviewing data, analysis, and findings concerning the selected study sites. Although a majority of the interchange sites were oriented with the freeway aligned north-south, the Loop 202/Rural Road interchange has the freeway aligned east-west. Also, the Loop 101/Bell Road and SR 51/Greenway Road interchanges have a skewed configuration, although the freeway generally aligns north-south. The freeway alignment could potentially affect driver vision caused by sun glare. Another difference between the interchanges that could factor into inherent interchange characteristics is the method of separating the freeway from the cross road. The interchange could be configured with the freeway passing over the cross road (an overpass interchange) or the freeway passing under the cross road (an underpass interchange). Either configuration may have advantages and disadvantages relating to interchange operations and safety. Half of the study sites selected were of the overpass interchange variety with two of these three sites also having signalized off-ramp right turn movements. Yet another variation was present at the SR 51/Glendale Avenue interchange where the northbound off-ramp right turn movement was controlled by a traffic signal and the southbound off-ramp right turn movement was yield-controlled. This mixture of off-ramp right turn traffic controls prompts particular attention to the analysis of the overall interchange operation while also providing a microcosm to potentially compare the two methods of control. Ideally the study site selection would have attempted to minimize, if not eliminate, these characteristic variables through consistency, but given the availability of potential study sites meeting the primary selection criteria stated previously, this was not possible.



  
1' = 100'

*Photo source: Maricopa County Assessor's Office  
Photo date: December 2002*

**Figure 4. State Route 51 & Indian School Road Aerial Photograph**



  
1' = 100'

*Photo source: Maricopa County Assessor's Office*  
*Photo date: December 2002*

**Figure 5. State Route 51 & Glendale Avenue Aerial Photograph**



  
1' = 100'

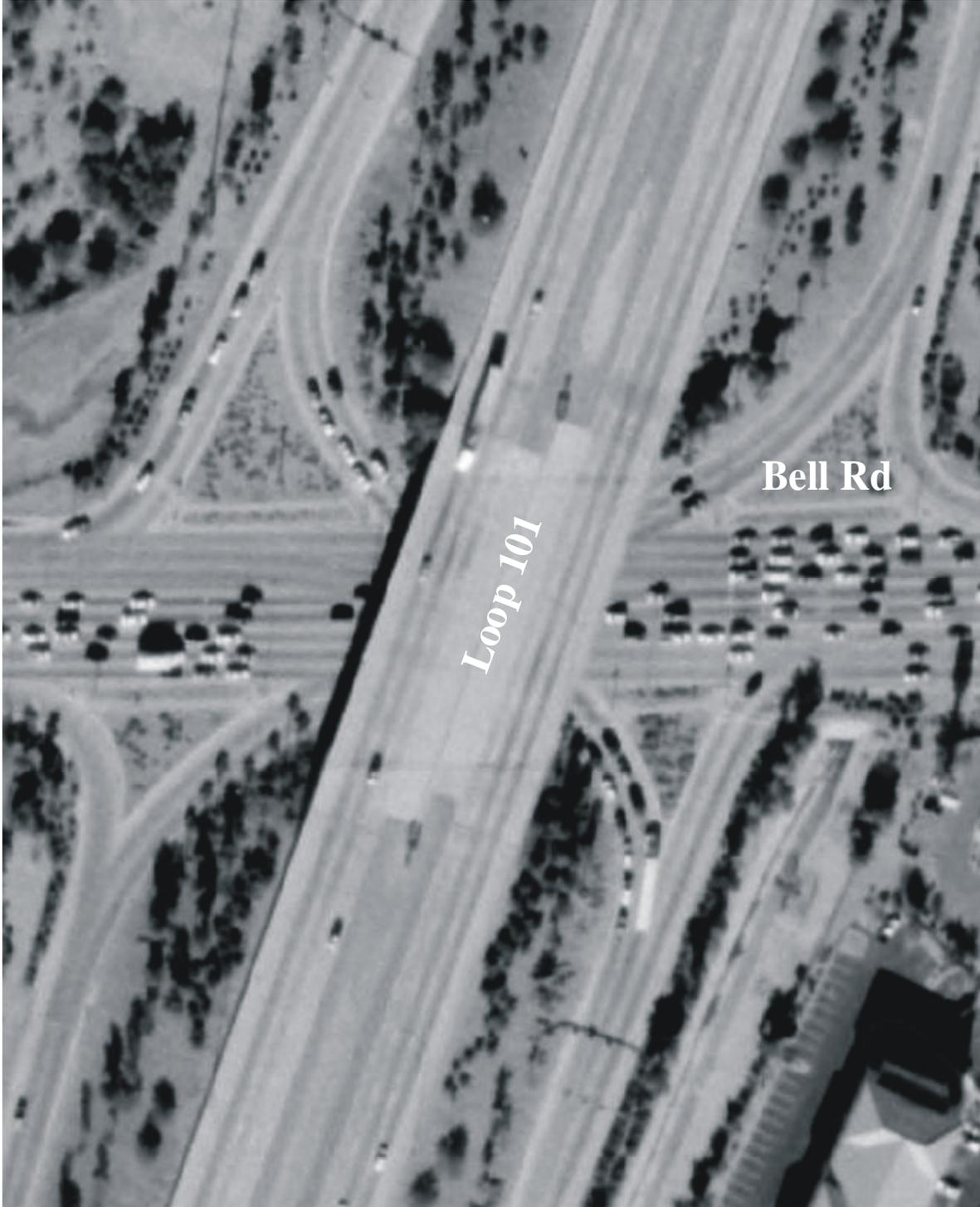
*Photo source: Maricopa County Assessor's Office  
Photo date: December 2002*

**Figure 6. State Route 51 & Cactus Road Aerial Photograph**



*Photo source: Maricopa County Assessor's Office  
Photo date: December 2002*

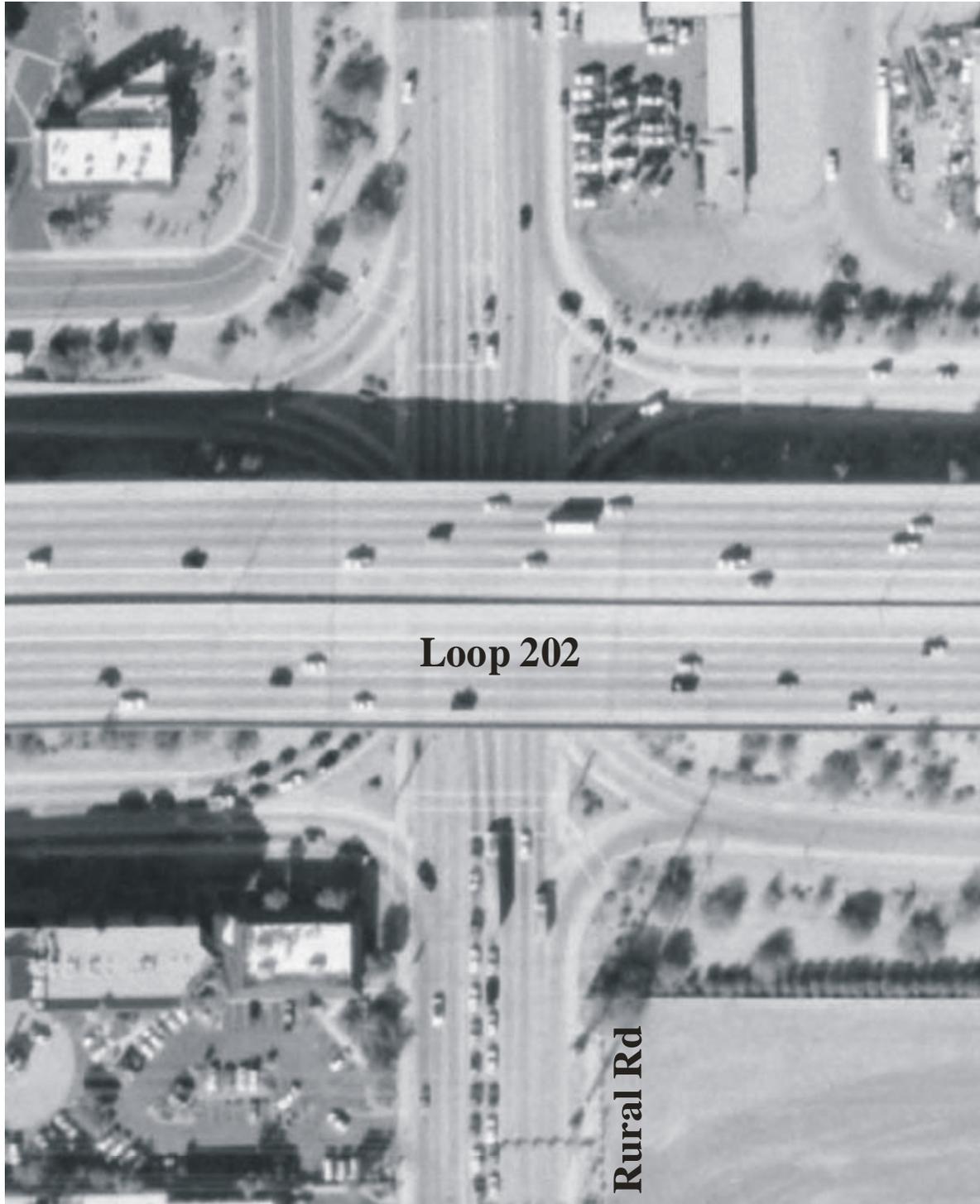
**Figure 7. State Route 51 & Greenway Road Aerial Photograph**



  
1' = 100'

*Photo source: Maricopa County Assessor's Office  
Photo date: December 2002*

**Figure 8. Loop 101 (Agua Fria Freeway) & Bell Road Aerial Photograph**



**Loop 202**

**Rural Rd**



1' = 100'

*Photo source: Maricopa County Assessor's Office  
Photo date: December 2002*

**Figure 9. Loop 202 (Red Mountain Freeway) & Rural Road Aerial Photograph**

Table 1. SPUI Study Sites Characteristic Data

Intersection Location (Freeway Orientation)	Distance Between Cross Road Stop Lines (ft)	Off-Ramp Right Turn Control Type	Speed Limit (mph)	Lane Assignments (Left-Thru-Right)	Turn Bay Lengths (ft)	Right Turn Radius (ft)	Angle of Ramp Approach/Departure to Mainline (degrees)	Cross Road Thru & Off-Ramp Right Turn Loop Detectors	Loop Detector Distance from Stop Line/Crosswalk	Cross Road & Off-Ramp Left Turn Loop Detectors	Loop Detector Distance from Stop Line/Crosswalk
SR 51 and Indian School Rd (Overpass) EB Indian School Road WB Indian School Road NB Off-ramp SB Off-ramp	315	Signal Signal	35 35 n/a n/a	2-3-1 2-3-1 2-0-1 2-0-1	L = 300, R = 250 L = 250, R = 300 360 325	60 60 75 75	90 90 90 90	2 x 30 (10' gap) 2 x 30 (10' gap) 2 x 30 (10' gap) 2 x 30 (10' gap)	6' downstream 6' downstream 6' downstream 6' downstream	2 x 30 (10' gap) 2 x 30 (10' gap) 2 x 30 (10' gap) 2 x 30 (10' gap)	6' downstream 6' downstream 6' downstream 6' downstream
SR 51 and Cactus Rd (Underpass) EB Cactus Road WB Cactus Road NB Off-ramp SB Off-ramp	240	Yield Yield	n/a 40 40 n/a n/a	2-3-0 2-3-0 2-0-1 2-0-1	L = 200, R = n/a L = 300, R = n/a 125+150 150+125	70 60 80 90	85 85 90 95	2 x 40 (10' gap) 2 x 40 (10' gap) n/a n/a	5' upstream 5' upstream	2 x 40 (10' gap) 2 x 40 (10' gap) not detectable not detectable	5' upstream 5' upstream
SR 51 & Glendale Rd (Underpass) EB Glendale Road WB Glendale Road NB Off-ramp SB Off-ramp	215	Signal Yield	40 40 n/a n/a	2-2-1 2-3-0 2-0-1 2-0-1	L = 275, R = 250 L = 275, R = n/a 350 375	75 100 60 125	85 85 90 95	2 x 40 (12' gap) 2 x 40 (12' gap) 1 x 30 n/a	6' downstream 6' downstream 5' downstream	2 x 40 (12' gap) 2 x 40 (12' gap) 1 x 60* 1 x 60*	6' downstream 6' downstream 10' downstream 10' downstream
SR 51 and Greenway Rd (Underpass) EB Greenway Road WB Greenway Road NB Off-ramp SB Off-ramp	305	Yield Yield	45 45 n/a n/a	2-3-1 2-3-0 2-0-1 2-0-1	L = 250, R = 225 L = 250, R = n/a 160+100 200+100	100 100 50 65	75 75 115 115	2 x 40 (12' gap) 2 x 40 (12' gap) n/a n/a	at stop line at stop line	2 x 40 (12' gap) 2 x 40 (12' gap) 1 x 40 1 x 40	at stop line at stop line at crosswalk at crosswalk
Loop 101 and Bell Road (Overpass) EB Bell Road WB Bell Road NB Off-ramp SB Off-ramp	335	Signal Signal	40 40 n/a n/a	2-3-1 2-3-1 2-0-2 2-0-2	L = 525, R = 500+ L = 525, R = 100 I = 200, O = 600 I = 150, O = 700	80 80 30 100	75 75 115 120	not detectable not detectable not detectable not detectable	not detectable not detectable not detectable not detectable	not detectable not detectable not detectable not detectable	not detectable not detectable not detectable not detectable
Loop 202 and Rural Rd (Overpass) NB Rural Road WB Rural Road EB Off-ramp WB Off-ramp	315	Yield Yield	40 40 n/a n/a	2-3-1 2-3-1 2-0-2 2-0-1	L = 250, R = 150 L = 225, R = 150 550 450	60 80 70 120	90 90 90 90	not detectable not detectable n/a n/a	not detectable not detectable 3 x 6 (0' gap)** not detectable	not detectable not detectable not detectable not detectable	10' upstream

Notes:  
 1. Lane Assignments - A zero for the right turn lane is a shared through-right turn lane.  
 2. Turn Bay Lengths - L=left, R=right, I=inside right turn lane, O=outside right turn lane, XXX = XXX (exclusive right turn storage area + Shared storage area with left turn lane)  
 3. Loop detectors that were not detectable during field inspections were assumed to be pre-formed installations implemented during roadway construction. Assumed size/configuration was commensurate with other interchange locations  
 n/a = not applicable  
 \* bridge deck seams interrupt loop pattern -  
 equivalent size shown  
 \*\* southern LT lane only

## **Operations-Related Data**

The data on interchange/off-ramp right turn operations has three elements: traffic volume, interchange signal timing/phasing, and off-ramp right turn specific delays. The procedures used to collect data on each of these elements are described below and the resulting information presented accordingly.

### *Traffic Volumes*

Data relating to traffic volumes was fundamental to the evaluation of the study sites. Two-way daily traffic volumes were collected at each interchange with a majority of the other data collection efforts occurring simultaneously. The daily traffic volumes were collected using automatic traffic recorders (ATRs) which consist of a counter and pneumatic tube placed at selective locations within the interchange area. Specific volumes for the movements through the interchange were also recorded by data collectors for a one and a half hour period in the morning and evening. The resulting volumes are shown in Figures 10 through 15.

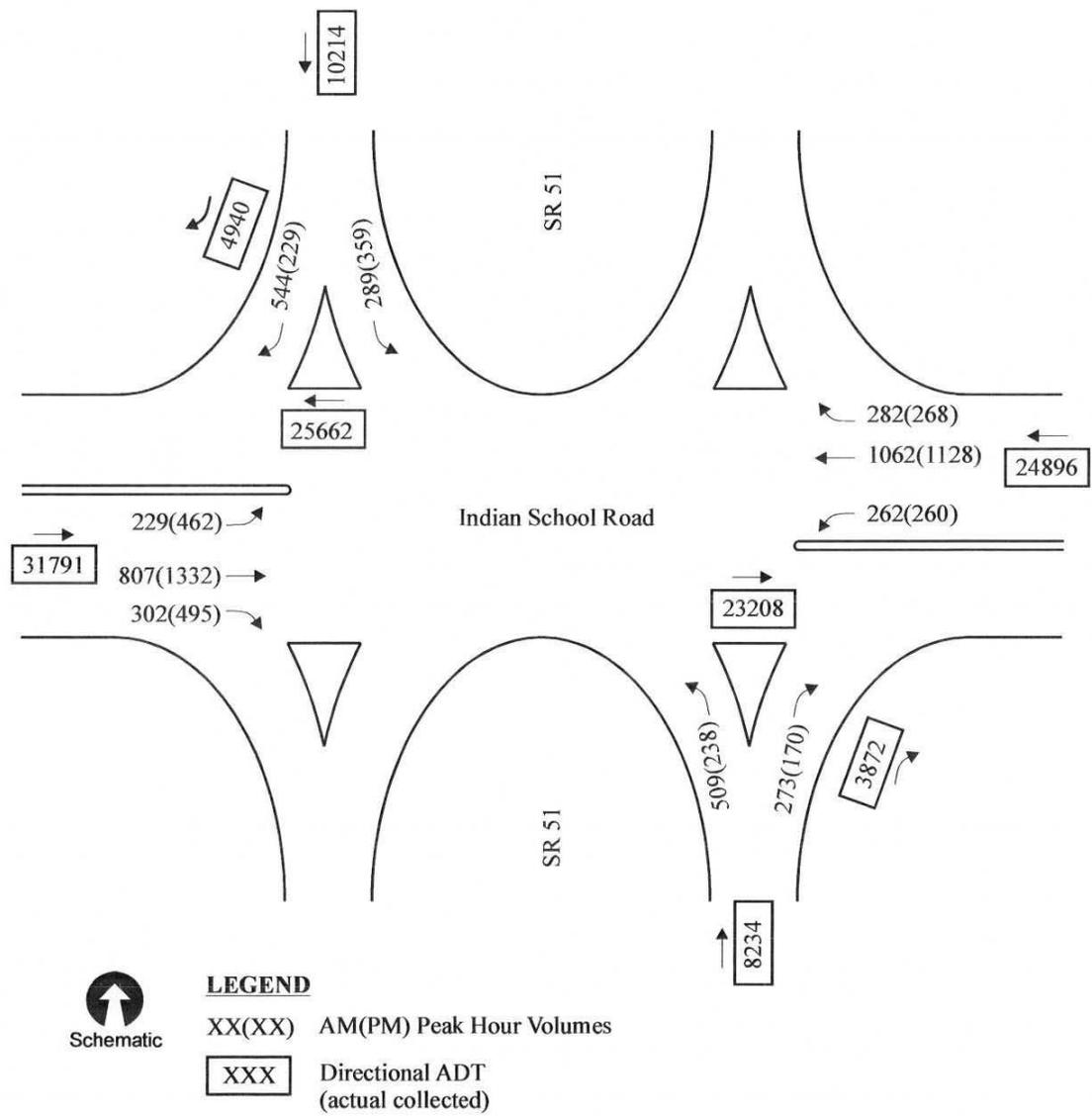
The number of right turns made on red from the off-ramp was recorded. Additionally, the number of heavy trucks was noted and used to calculate truck percentages for the interchange. The raw data from the turning movement and daily traffic collections are contained in the Appendix A.

The traffic volume data was collected in January 2004. Review of Arizona Department of Transportation (ADOT) seasonal adjustment factors revealed that January is one percent higher than the annual average month for the Phoenix area. Therefore, the volumes presented previously were adjusted downward by 1% prior to any computations being performed.

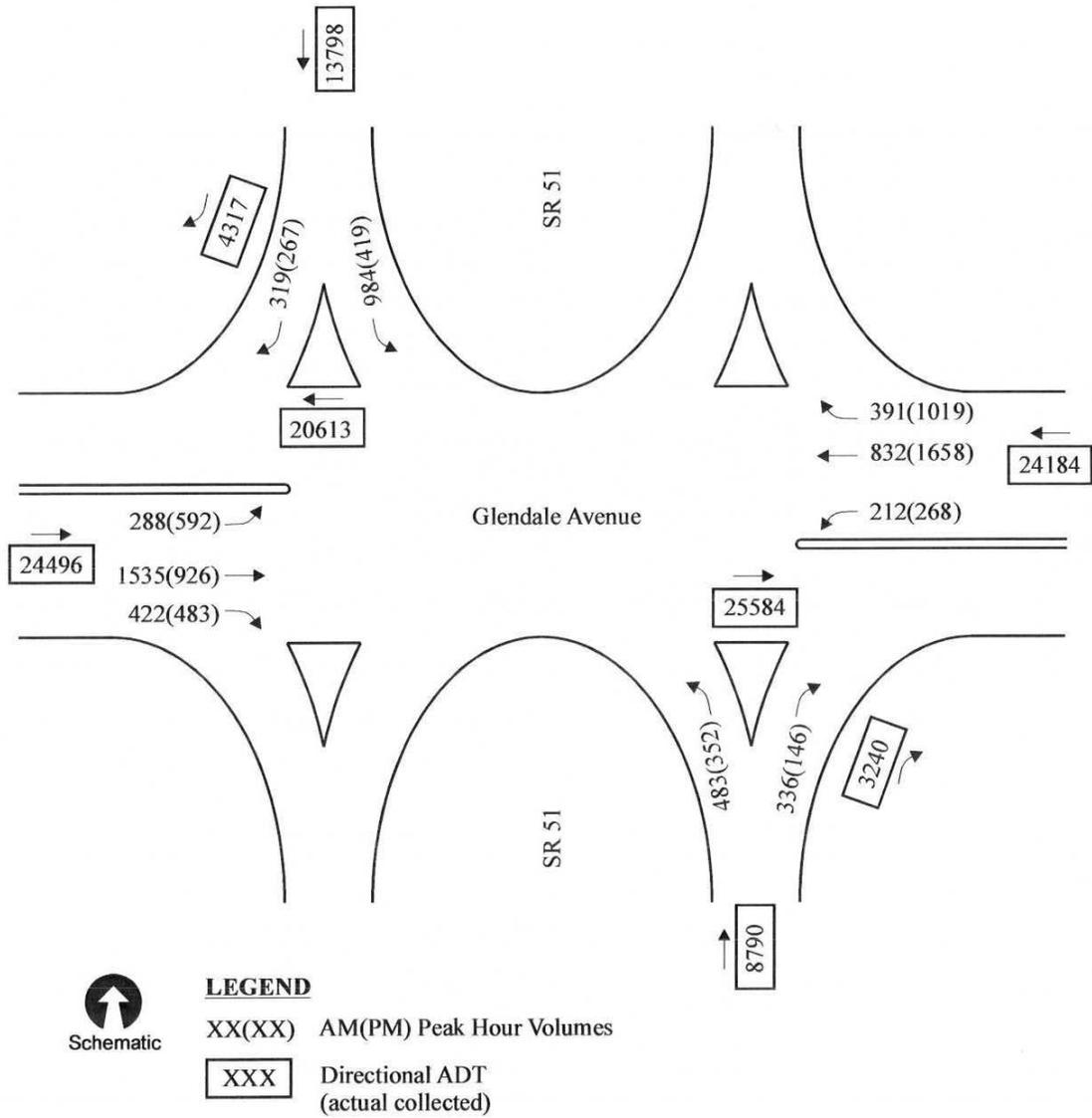
### *Interchange Signal Timings*

Even though this study is specifically focused on the operations and safety related to the off-ramp right turn movements at SPUIs, the control employed at the off-ramp right turn can have an effect on the overall interchange efficiency. To account for this, signal timing information was required so that the entire interchange could be evaluated from an operations standpoint.

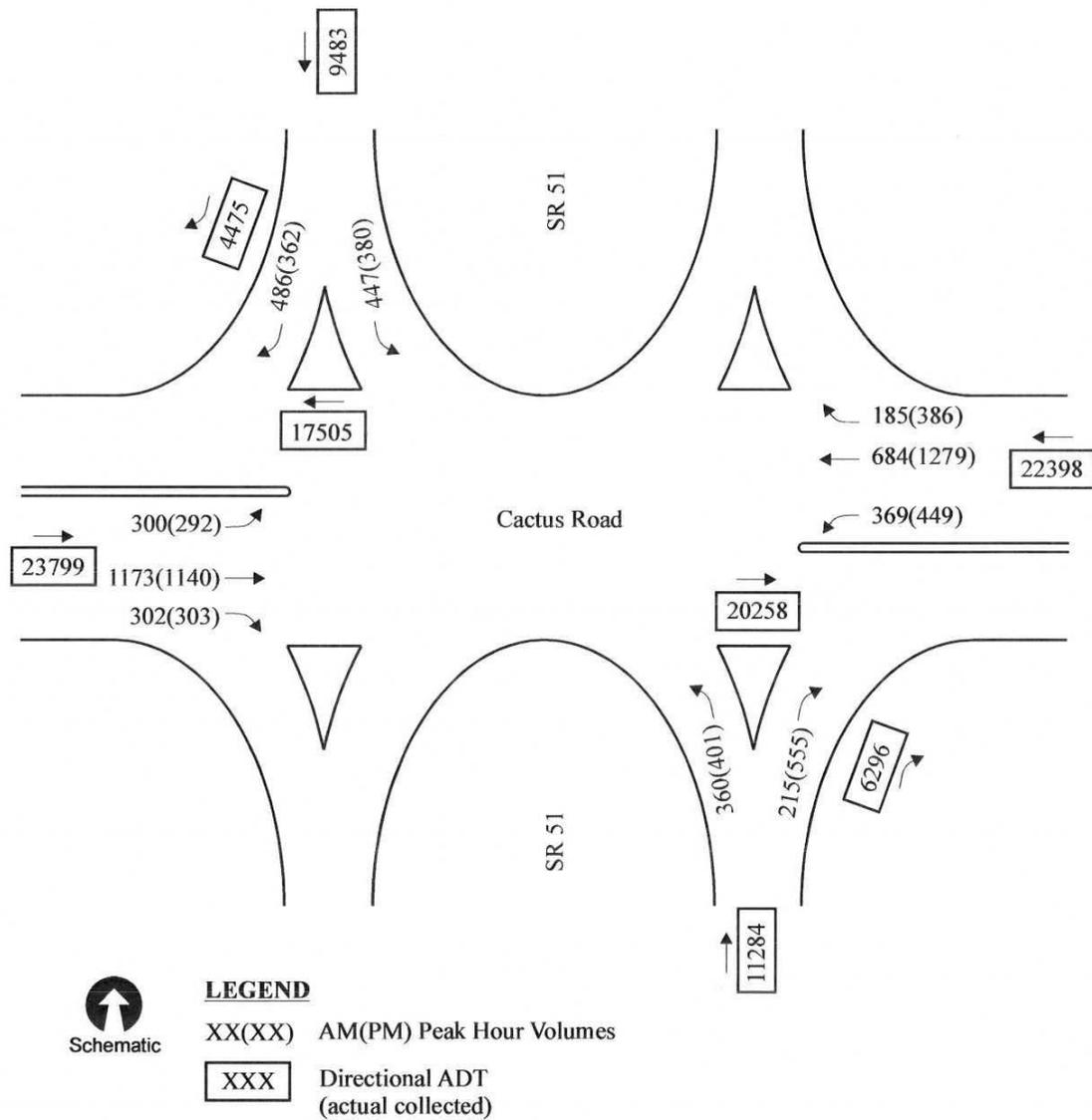
The overall interchange signal timing/phasing and the specific timing/phasing associated with the off-ramp right turn movement were collected from the governing agencies. Actual signal timing samples were recorded in the field in order to verify, to a certain degree, the information provided by the agencies. Generally, the in-field timing samples concurred with supplied timing information which was then used for calculations relating to delay and overall interchange operations via the CORSIM modeling.

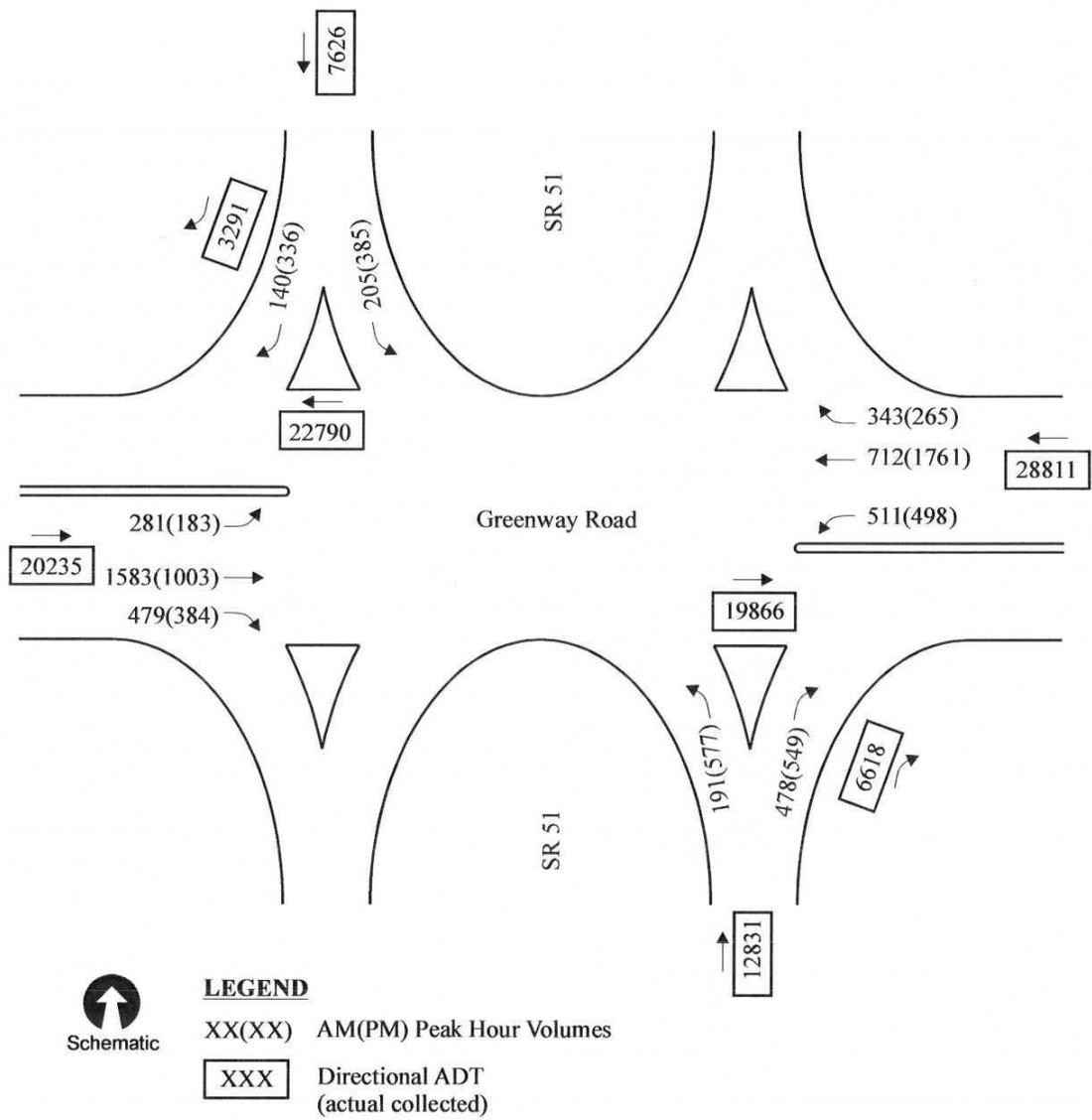


**Figure 10. Existing 2004 Volumes SR 51 & Indian School Road**



**Figure 11. Existing 2004 Volumes SR 51 & Glendale Avenue**





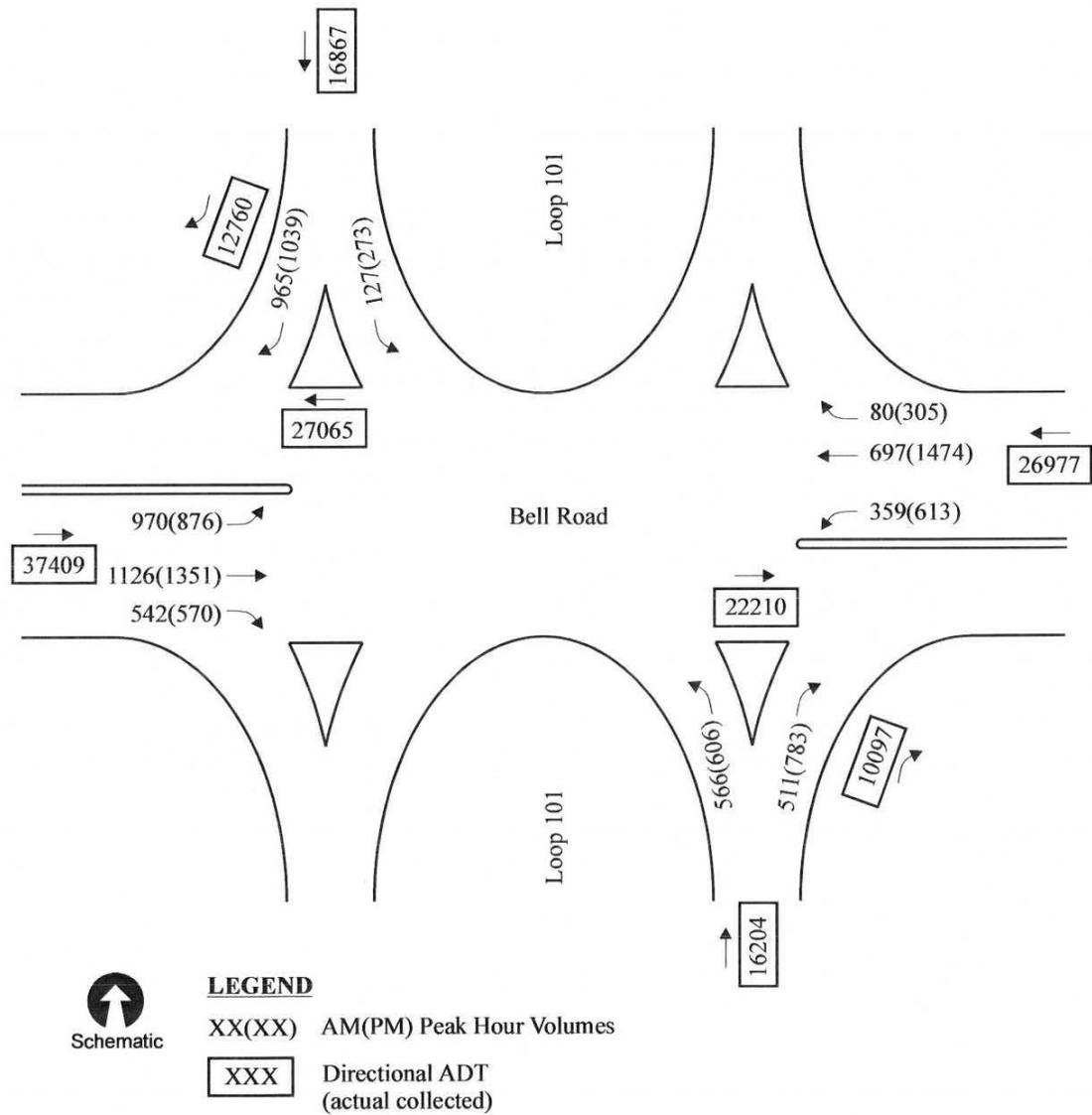
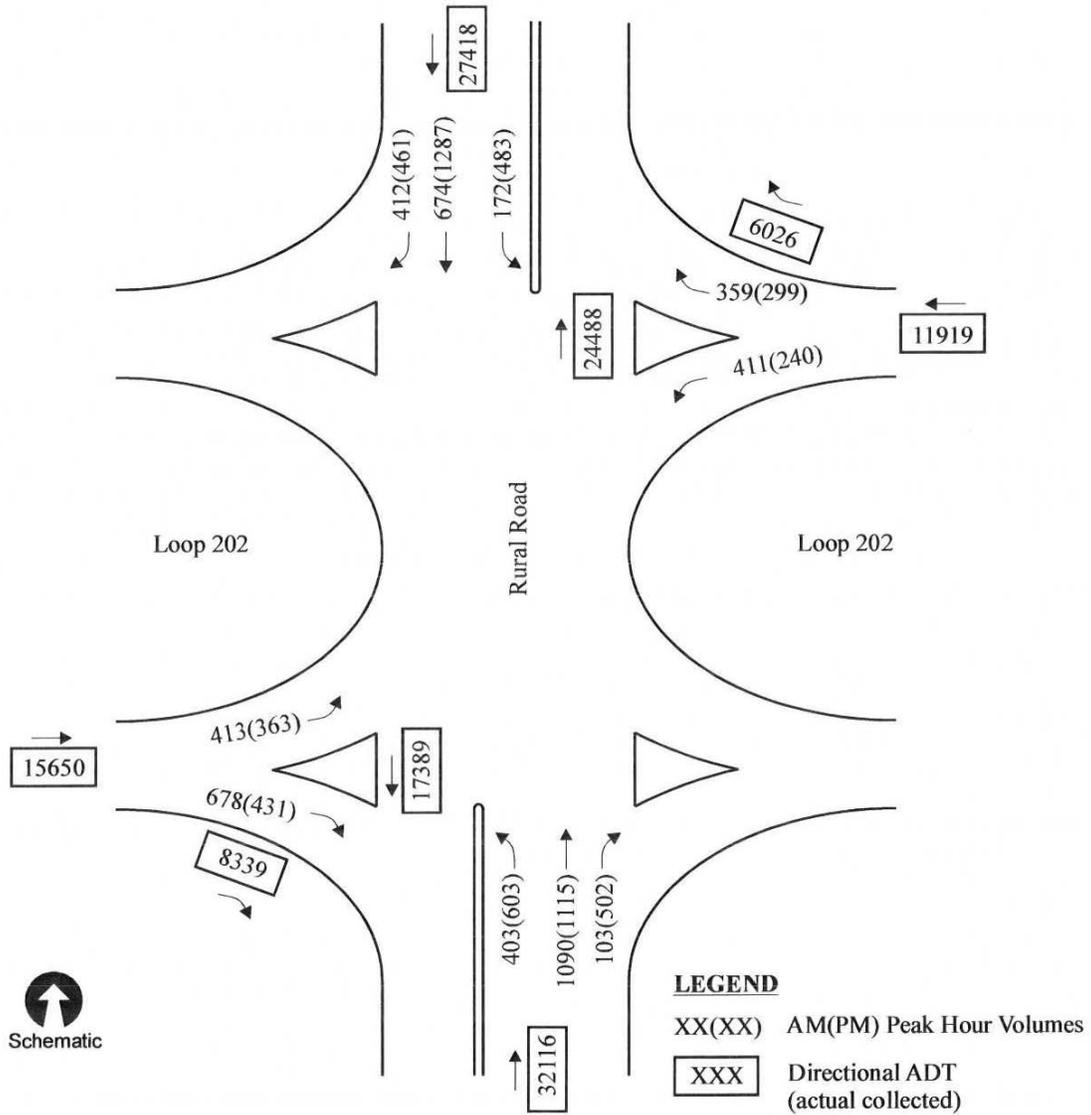


Figure 14. Existing 2004 Volumes Loop 101 & Bell Road



**Figure 15. Existing 2004 Volumes Loop 202 & Rural Road**

### *Off-Ramp Right Turn Delay*

Particular attention was devoted to the off-ramp right turn movement operations during the data collection tasks. The primary indication of operational efficiency for the off-ramp right turn movement is the delay incurred by the motorist due to the control device, whether a signal or yield sign, and prevailing traffic conditions. In order to determine this average delay per vehicle, a data collection procedure from the *Highway Capacity Manual* (HCM) [12] was used as a guide. The procedure is primarily dependent on three components of traffic data: volume over a specified period of time, number of vehicles stopping during that time, and number of vehicles considered part of a queue in the off-ramp right turn traffic flow. The data collected was for the one and one-half hour peak periods in the morning and evening.

The traffic volume for the off-ramp right turn movement was collected in conjunction with the turning movements for the entire site. The number of off-ramp right turn vehicles that were counted as a vehicle that stopped was based on observing a vehicle come to a full stop at any point along the length of the off-ramp right turn lane(s) up to and including the junction point with the cross road. If the same vehicle stopped multiple times, it was only recorded as one stopped vehicle in the count total. Vehicles counted as being part of a traffic queue constituted any vehicle within one vehicle length of another vehicle, whether one or both vehicles were moving or stopped. Additionally, one off-ramp right turn vehicle waiting to turn right onto the cross road was considered a queue of one. These vehicle-in-queue determinations were assessed every 19 seconds per the HCM data collection guidelines, which require the interval to be any value up to 20 seconds so long as the interval does not divide evenly into the cycle length for the interchange. Nineteen seconds was selected as the observation interval because this interval value would not divide evenly into any of the signal cycle lengths used at the study sites. Observing/recording the traffic queues in this manner results in a random sample of values, which were then used in the calculation of the control delay for the movement.

### **Safety-Related Data**

The analysis of the safety implications related to the off-ramp right turn control type was supported by data collected concerning conflicts observed and crash history investigations. Conflict observations were conducted by experienced traffic engineers, one positioned at each off-ramp right turn area, during the AM and PM peak periods. Crash histories were obtained for each interchange that has the off-ramp right movement only. Each of these data sets were then used in conjunction with the traffic volume data to determine both conflict and crash rates specifically related to the off-ramp right turn movement.

### *Conflict Observations*

Although traffic crash records provide the most direct measure of safety for a roadway section, adequate data may not be available for analysis. Moreover, some crashes are not reported or records may be only available for a time period that does not represent current conditions at the study area. Therefore, conflict data specifically pertaining to the off-ramp right turn movements was obtained for the AM and PM peak periods at the study sites.

For the purposes of this study, a conflict was considered to be a traffic event involving two or more road users (i.e., vehicles, pedestrians, bicyclists, etc.), in which one or more user performs an abnormal or unusual action causing another or others to execute an abrupt or evasive maneuver to avoid a collision. The most common avoidance maneuver related to the off-ramp right turn movement is either abrupt braking or swerving to avoid a collision.

The decision concerning what traffic occurrence/situation constitutes a traffic conflict is subjective to some degree. In an attempt to minimize observer subjectivity, only experienced engineers conducted the conflict observations. The same two engineers were used at every study site location. The observation positions were chosen on a site-by-site basis based on whichever position provided the best vantage point to observe conflicts involving off-ramp right turn traffic interacting along the off-ramp or at the ramp junction with the cross road where the cross road traffic could also be involved. The following guidelines were used in identifying traffic conflicts:

- Secondary conflicts caused by an initial or primary conflict were possible at the study sites. If this occurred, a maximum of one secondary conflict was recorded and tabulated as a separate traffic conflict.
- Unusual occurrences due to the presence of ambulances, fire trucks, or police vehicles were identified but not included in the conflict observation tally.
- Example of non-conflict occurrence: a driver performing normal braking due to the presence of a yellow/red signal or resulting traffic queue.
- Example of a conflict occurrence: a driver who brakes abruptly to avoid a collision with a vehicle slowing for a yellow/red signal because they anticipated following the vehicle through the signal.

In order to assist with the observation and recording of traffic conflicts, a schematic key map was developed to identify the location of conflicts. The key map is shown in Figure 16 below. The numbered location areas are intended to be general in nature and to cover all areas of potential conflicts, although observations found that most conflicts were confined to one or two main areas. This same key map was also used for the crash history investigations.

When conflicts were observed, four items of information were recorded: the time, the location (per the key map), the types of road users and their associated movement, the avoidance actions taken, and a more detailed account (if necessary). Observed conflicts were recorded on standardized sheets used at each study site.

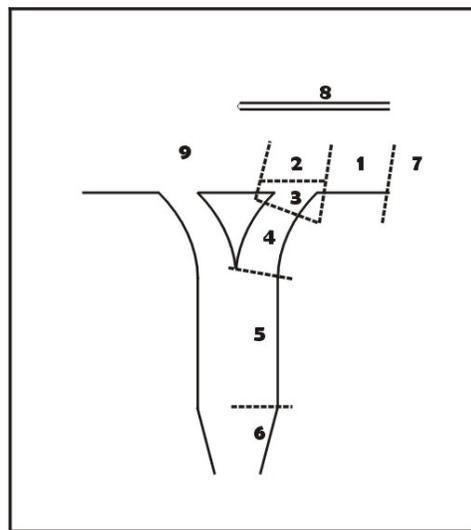
The data collected from the conflict observations will be presented in the Calculations section of this chapter in conjunction with the calculated conflict rate values.

### *Crash History Investigation*

The crash history investigations used data from ADOT's Accident Location Identification Surveillance System (ALISS) database, which was queried for the most recent three-year

period of crash information (August 1, 2000 through July 31, 2003) at the time of the request. The query consisted of any crashes occurring specifically in the right turn lane(s) on the off-ramp or at the crossroad. Crashes reported as occurring on the cross road involving an off-ramp right turn vehicle were also included in the query request. The effective distance for the query was set at 300 feet from the off-ramp right turn/cross road junction point. The resulting number of crash records returned from the query was about 650 for the six interchanges (twelve off-ramps) for the three-year period.

The listing of crash records was then used to retrieve the actual crash reports from ADOT's Traffic Records Section. The actual crash reports were reviewed by traffic engineers to determine their applicability to the off-ramp right turn movement. Overall, only a small percentage (~2%) were found to be inapplicable and were thus removed from the crash record listing for the respective study site location. During the review of the crash reports, the location of the crash was noted according to the key map shown as Figure 16. This determination was somewhat subjective since the crash reports usually provided a sketch of the crash location relative to geometric aspects of the interchange. Generally, Area 3 was reserved for crashes occurring within one to two vehicle lengths of the junction point with the cross road in addition to crashes involving cross road traffic in the curb lane. Area 4 was reserved for other crashes occurring farther away, bounded by the gore point on the off-ramp. Tables 2-13 present the crash data totals.



**Figure 16. Schematic Conflict/Crash Location Key Map**

Table 2. SR 51/Indian School Road - Southbound Off-Ramp Right Turn Related Crashes

Signal Controlled (08/01/00 - 07/31/03)	No Injury	Possible Injury	Non- Incapacitating Injury	Incapacitating Injury	Fatality	Unknown
<b>TOTAL</b>	45	8	2	0	0	0
<b>Rear-End</b>	45	8	2	0	0	0
* Location 1						
Location 2						
Location 3	45	8	2			
Location 4						

\* Locations refer to Figure 16

Table 3. SR 51/Indian School Road - Northbound Off-Ramp Right Turn Related Crashes

Signal Controlled (08/01/00 - 07/31/03)	No Injury	Possible Injury	Non- Incapacitating Injury	Incapacitating Injury	Fatality	Unknown
<b>TOTAL</b>	21	8	0	0	0	0
<b>Rear-End</b>	21	8	0	0	0	0
* Location 1						
Location 2						
Location 3	21	8				
Location 4						

\* Locations refer to Figure 16

Table 4. SR 51/Glendale Avenue - Southbound Off-Ramp Right Turn Related Crashes

Yield Controlled (08/01/00 - 07/31/03)	No Injury	Possible Injury	Non- Incapacitating Injury	Incapacitating Injury	Fatality	Unknown
<b>TOTAL</b>	55	11	3	0	0	0
<b>Rear-End</b>	51	11	2	0	0	0
* Location 1						
Location 2						
Location 3	51	10	1			
Location 4		1	1			
<b>Sideswipe (same dir.)</b>	1	0	0	0	0	0
Location 1						
Location 2						
Location 3	1					
Location 4						
<b>Single Vehicle</b>	2	0	0	0	0	0
Location 1						
Location 2						
Location 3						
Location 4	2					
<b>Angle</b>	1	0	0	0	0	0
Location 1						
Location 2						
Location 3	1					
Location 4						
<b>Pedestrian-Involved</b>	0	0	1	0	0	0
Location 1						
Location 2						
Location 3			1			
Location 4						

\* Locations refer to Figure 16

**Table 5. SR 51/Glendale Avenue - Northbound Off-Ramp Right Turn Related Crashes**

Signal Controlled (08/01/00 - 07/31/03)	No Injury	Possible Injury	Non- Incapacitating Injury	Incapacitating Injury	Fatality	Unknown
<b>TOTAL</b>	9	1	0	0	0	0
<b>Rear-End</b>	9	1	0	0	0	0
* Location 1						
Location 2						
Location 3	8					
Location 4		1				
Location 6	1					

\* Locations refer to Figure 16

**Table 6. SR 51/Cactus Road - Southbound Off-Ramp Right Turn Related Crashes**

Yield Controlled (08/01/00 - 07/31/03)	No Injury	Possible Injury	Non- Incapacitating Injury	Incapacitating Injury	Fatality	Unknown
<b>TOTAL</b>	8	2	0	0	0	0
<b>Rear-End</b>	8	2	0	0	0	0
* Location 1						
Location 2						
Location 3	8	2				
Location 4						

\* Locations refer to Figure 16

**Table 7. SR 51/Cactus Road - Northbound Off-Ramp Right Turn Related Crashes**

Yield Controlled (08/01/00 - 07/31/03)	No Injury	Possible Injury	Non- Incapacitating Injury	Incapacitating Injury	Fatality	Unknown
<b>TOTAL</b>	70	28	2	0	0	0
<b>Rear-End</b>	65	28	2	0	0	0
* Location 1						
Location 2						
Location 3	57	24	2			
Location 4	8	4				
<b>Sideswipe (same dir.)</b>	2	0	0	0	0	0
Location 1						
Location 2						
Location 3	2					
Location 4						
<b>Single Vehicle</b>	2	0	0	0	0	0
Location 1						
Location 2						
Location 3	2					
Location 4						
<b>Backing</b>	1	0	0	0	0	0
Location 1						
Location 2						
Location 3						
Location 4	1					

\* Locations refer to Figure 16

**Table 8. SR 51/Greenway Road - Southbound Off-Ramp Right Turn Related Crashes**

Yield Controlled (08/01/00 - 07/31/03)	No Injury	Possible Injury	Non- Incapacitating Injury	Incapacitating Injury	Fatality	Unknown
<b>TOTAL</b>	3	0	0	0	0	0
<b>Rear-End</b>	3	0	0	0	0	0
* Location 1						
Location 2						
Location 3	3					
Location 4						

\* Locations refer to Figure 16

**Table 9. SR 51/Greenway Road - Northbound Off-Ramp Right Turn Related Crashes**

Yield Controlled (08/01/00 - 07/31/03)	No Injury	Possible Injury	Non- Incapacitating Injury	Incapacitating Injury	Fatality	Unknown
<b>TOTAL</b>	46	17	2	0	0	0
<b>Rear-End</b>	46	17	2	0	0	0
* Location 1						
Location 2						
Location 3	41	16	1			
Location 4	5	1	1			

\* Locations refer to Figure 16

**Table 10. Loop 101/Bell Road - Southbound Off-Ramp Right Turn Related Crashes**

Signal Controlled (08/01/00 - 07/31/03)	No Injury	Possible Injury	Non- Incapacitating Injury	Incapacitating Injury	Fatality	Unknown
<b>TOTAL</b>	34	7	3	0	0	0
<b>Rear-End</b>	30	6	3	0	0	0
* Location 1						
Location 2						
Location 3	27	5	3			
Location 4	3	1				
<b>Sideswipe (same dir.)</b>	3	0	0	0	0	0
Location 1						
Location 2	2					
Location 3	1					
Location 4						
<b>Backing</b>	1	0	0	0	0	0
Location 1						
Location 2						
Location 3	1					
Location 4						
<b>Pedestrian-Involved</b>	0	1	0	0	0	0
Location 1						
Location 2						
Location 3		1				
Location 4						

\* Locations refer to Figure 16

Table 11. Loop 101/Bell Road - Northbound Off-Ramp Right Turn Related Crashes

Signal Controlled (08/01/00 - 07/31/03)	No Injury	Possible Injury	Non- Incapacitating Injury	Incapacitating Injury	Fatality	Unknown
<b>TOTAL</b>	35	11	5	0	0	0
<b>Rear-End</b>	33	11	4	0	0	0
* Location 1						
Location 2						
Location 3	27	9	4			
Location 4	6	2				
<b>Sideswipe (same dir.)</b>	1	0	0	0	0	0
Location 1						
Location 2						
Location 3	1					
Location 4						
<b>Single Vehicle</b>	0	0	0	0	0	0
Location 1						
Location 2						
Location 3						
Location 4						
<b>Backing</b>	1	0	0	0	0	0
Location 1						
Location 2						
Location 3	1					
Location 4						
<b>Bicyclist-Involved</b>	0	0	1	0	0	0
Location 1						
Location 2						
Location 3			1			
Location 4						

\* Locations refer to Figure 16

**Table 12. Loop 202/Rural Road - Westbound Off-Ramp Right Turn Related Crashes**

<b>Yield Controlled (08/01/00 - 07/31/03)</b>	<b>No Injury</b>	<b>Possible Injury</b>	<b>Non- Incapacitating Injury</b>	<b>Incapacitating Injury</b>	<b>Fatality</b>	<b>Unknown</b>
<b>TOTAL</b>	75	19	2	2	0	1
<b>Rear-End</b>	70	19	2	2	0	0
* Location 1		1				
Location 2						
Location 3	67	18	2	2		
Location 4	3					
<b>Sideswipe (same dir.)</b>	2	0	0	0	0	0
Location 1						
Location 2						
Location 3	2					
Location 4						
<b>Single Vehicle</b>	2	0	0	0	0	1
Location 1						
Location 2						
Location 3	2					
Location 4						1
<b>Angle</b>	1	0	0	0	0	0
Location 1						
Location 2						
Location 3	1					
Location 4						

\* Locations refer to Figure 16

**Table 13. Loop 202/Rural Road - Eastbound Off-Ramp Right Turn Related Crashes**

Yield Controlled (08/01/00 - 07/31/03)	No Injury	Possible Injury	Non- Incapacitating Injury	Incapacitating Injury	Fatality	Unknown
<b>TOTAL</b>	79	23	4	2	1	0
<b>Rear-End</b>	70	22	4	2	0	0
* Location 1						
Location 2						
Location 3	68	20	4	2		
Location 4	2	2				
<b>Sideswipe (same dir.)</b>	6	0	0	0	0	0
Location 1						
Location 2						
Location 3	5					
Location 4	1					
<b>Single Vehicle</b>	2	0	0	0	0	0
Location 1						
Location 2						
Location 3						
Location 4						
Location 8/9	2					
<b>Angle</b>	1	1	0	0	0	0
Location 1						
Location 2						
Location 3	1	1				
Location 4						
<b>Bicyclist-Involved</b>	0	0	0	0	1	0
Location 1						
Location 2						
Location 3					1	
Location 4						

\* Locations refer to Figure 16

If a particular type of crash was not listed in the above tables, then no crashes of that type were found to have occurred in the three-year assessment period. The crash types listed are based on the information noted by the officer on the actual crash report. The above data was used in conjunction with the volume data (or derivatives thereof) to calculate the crash rates for the off-ramp right turn movements at the study sites. These calculations along with other calculations pertaining to the data described and presented previously are explained and contained in the following section.

## **CALCULATIONS**

This section presents the calculations performed using the operational and safety data. The operational data collected specifically for the off-ramp right turn movement is used to calculate the control delay (i.e., the portion of overall delay that results when a vehicle slows or stops due to the presence of a traffic control like a signal or yield sign) for the movement in the AM/PM peak periods/hours. Conflict observations are used with volume data to determine conflict rates for existing conditions. The volume data and past projections of volumes are also used with the crash history data to determine off-ramp right turn movement crash rates for the three-year assessment period.

## Control Delay Calculations

The calculation of the control delay for the off-ramp right turn movement is fairly complicated, relying on several factors and values supplied by tables in the HCM [12]. The general description of the calculation is shown below with the detailed description and an example provided in the Appendix B. The main components which are used to calculate the average control delay value per vehicle are:

- (1) Time-in-Queue per Vehicle (seconds) =  
$$\frac{\text{Count Interval [19 seconds]} * (\text{Sum of Vehicles Observed in Queue} / \text{Total Off-Ramp Right Turn Volume}) * 0.9 \text{ [HCM correction factor]}}{1}$$
- (2) Number of Vehicles Stopping Per Lane Per Cycle Length (vehicles) =  
$$\frac{\text{Number of Vehicles that Stopped One or More Times}}{\text{Number of Signal Cycles Observed} * \text{Number of Off-Ramp Right Turn Lanes}}$$
- (3) Acceleration/Deceleration Correction Delay Value (seconds) =  
$$\frac{\text{Ratio of Off-Ramp Right Turn Vehicles That Stopped} * \text{Acceleration/Deceleration Factor [from HCM table—either +2 or +5 in this study based on the Equation 2 results and free-flow speed range estimate]}}{1}$$
- (4) Average Control Delay per Vehicle (seconds) =  
Equation 1 + Equation 3

These calculation procedures were performed for each off-ramp right turn movement at the study sites regardless of the traffic control in place. Even though there was not a portion of the signal cycle length devoted to the off-ramp right turn movements where yield control was in place, the cycle length value for the interchange was still assumed in the control delay calculations. This assumption is based on the yield control operation being a derivative of gap acceptance in the cross road traffic stream for off-ramp right turn traffic. These gaps are created by the traffic pattern fluctuations and by the cycling of the overall interchange signal control. Control delay calculations for off-ramp right turn movements at signal and yield control sites are similar since most of the delay is generated as a function of gap acceptance: right turn on red at the signal control sites and yielding right-of-way at the yield control sites.

The calculated delay results are shown in Table 14 (p.44). Please note that calculations are provided for the peak *period* and the peak *hour*. Since the data component pertaining to number of vehicles that stopped one or more times was collected only for the peak period (i.e., the 1 ½ hour observation period), the peak hour value was pro-rated based on the proportion of time. Since the peak period and peak hour durations were relatively close, this assumption should not have a significant effect on the peak hour delay calculations. Other data collected and used in the control delay calculation was specified as to whether it pertained to the peak period and peak hour.

**Table 14. Control Delay for Off-Ramp Right Turn Movements**

	Off-Ramp Right Turn Signal Control						Off-Ramp Right Turn Yield Control							
	Indian School Road		Bell Road		Glendale Ave		Cactus Road		Greenway Road		Rural Road		Glendale Ave	
	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT
<b>ORRT Control Type</b>	Signal	Signal	Signal	Signal	Signal	Signal	Signal	Signal	Signal	Signal	Signal	Signal	Signal	Signal
<b>Overlap w/ Cross Road LT</b>	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes	yes
<b>Sum of Vehicles in Queue<sup>1</sup></b>	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT
AM Period (7-8:30)	698	218	1224	559	1319		217	412	91	872	296	601	181	
PM Period (4:30-6)	173	175	2512	1075	133		424	613	695	488	249	473	306	
AM Peak (actual)	474	186	874	421	945		159	303	61	716	258	457	119	
PM Peak (actual)	119	109	1686	696	85		321	452	509	296	179	250	183	
<b>Vehicles That Stopped 1+ Times</b>	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT
AM Period (7-8:30)	488	223	674	423	456		230	293	81	413	282	530	223	
PM Period (4:30-6)	141	157	1194	744	115		298	378	393	476	262	403	293	
AM Peak (pro-rated)	325	149	449	282	304		153	195	54	275	188	353	149	
PM Peak (pro-rated)	94	105	796	496	77		199	252	262	317	175	269	195	
<b>Total ORRT Vehicles</b>	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT
AM Period (7-8:30)	804	388	1375	718	486		649	301	203	681	468	987	513	
PM Period (4:30-6)	336	290	1551	1167	204		502	805	498	841	458	647	431	
AM Peak	544	273	965	511	336		486	215	140	478	359	678	319	
PM Peak	229	170	1039	783	146		362	555	336	549	299	431	267	
<b>Control Delay/Vehicle Computations</b>	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT
Number of ORRT Lanes	1	1	2	2	1		1	1	1	1	1	2	1	
Count Interval (s)	19	19	19	19	19		19	19	19	19	19	19	19	
<b>Calculations for Peak Period</b>	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT
Control Delay/Total Vehicles (s) - AM	16.50	10.68	16.91	14.79	51.57		6.35	26.01	8.52	24.33	12.02	11.57	6.70	
PM	9.78	11.47	30.77	17.50	12.39		16.05	14.47	26.52	11.02	10.33	13.89	13.49	
Control Delay/Stopped Vehicles (s) - AM	27.18	18.57	34.50	25.11	54.96		17.93	26.72	21.35	40.12	19.94	21.55	15.42	
PM	23.31	21.18	39.97	27.45	21.97		27.03	30.81	33.60	19.48	18.06	22.30	19.84	
<b>Calculations for Peak Hour</b>	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT	SB ORRT	NB ORRT
Control Delay/Total Vehicles (s) - AM	16.56	12.95	17.21	15.65	53.44		6.22	26.78	8.28	28.46	13.65	12.81	7.09	
PM	9.87	12.18	30.83	16.89	11.06		16.85	15.47	28.78	10.24	11.37	11.02	13.02	
Control Delay/Stopped Vehicles (s) - AM	27.71	23.72	36.98	28.37	59.06		19.75	29.52	21.46	49.47	26.07	24.60	15.17	
PM	24.05	19.72	40.24	26.66	20.97		30.65	34.08	36.91	17.74	19.43	17.66	17.83	
<b>Note</b>	Indian School Road	Bell Road	Glendale Ave	Indian School Road	Bell Road	Glendale Ave	Cactus Road	Greenway Road	Rural Road	Cactus Road	Greenway Road	Rural Road	Glendale Ave	Glendale Ave

1 - Sum of queued vehicles sampled at 19-second intervals. Vehicle stopped more than 19 seconds would be counted more than once.

**KEY**

ORRT - Off-Ramp Right Turn; OR - Off-Ramp; LT - Left Turn; NB - Northbound; SB - Southbound; WB - Westbound; EB - Eastbound

## Conflict Rate Calculations

The conflict rate for the off-ramp right turn movement is the ratio of the number of conflicts occurring and the volume of traffic that could potentially be involved in the conflicts. The volume component is comprised of the cross road traffic (both through volume, and volume generated by the opposing off-ramp left turn movement) and the off-ramp right turn traffic. The conflicts and volume are summed for the same period of time and the resulting ratio is multiplied by 1,000 to equate the value of the rate to typical crash rate values. The calculation is shown below:

$$(5) \text{ RTCV} = (\text{CO} / \text{TCV}) * 1000$$

where:

RTCV = Rate per thousand conflicting vehicles

CO = Conflicts observed

TCV = Total potentially conflicting vehicle volume

Table 15 presents the conflict data collected, the calculated conflict rates, and details concerning the locations of the conflicts observed.

**Table 15. Conflict Data and Rate Computations for Off-Ramp Right Turn Movements**

	Interchange/Off-Ramp	Conflicts Observed AM & PM	Period Traffic		Conflict Rate (RTCV*)	by location (see key map)				Involving			
			Off-Ramp	Cross Rd		1	2	3	4	ped/veh	bike/veh	veh/veh	other
Off-Ramp Right Turn Signal Control	Indian School/SR51 SB Off-Ramp	1	1033	3607	0.216	0	0	1	0	0	1	0	0
	Indian School/SR51 NB Off-Ramp	0	558	3161	0.000	0	0	0	0	0	0	0	0
	Glendale Road/SR51 NB Off-Ramp	2	690	5687	0.314	1	1	0	0	0	0	2	0
	Bell Road/L101(W) SB Off-Ramp	3	2926	4843	0.386	0	0	2	1	0	0	3	0
	Bell Road/L101(W) NB Off-Ramp	4	1885	4181	0.659	0	0	4	0	1	0	3	0
	<b>All Signal Control Off-Ramps</b>	<b>10</b>	<b>7092</b>	<b>21479</b>	<b>0.350</b>	<b>1</b>	<b>1</b>	<b>7</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>8</b>	<b>0</b>
Off-Ramp Right Turn Yield Control	Glendale Road/SR51 SB Off-Ramp	3	944	4802	0.522	0	0	3	0	0	0	3	0
	Cactus Road/SR51 SB Off-Ramp	2	848	2724	0.560	0	0	2	0	0	0	2	0
	Cactus Road/SR51 NB Off-Ramp	7	770	3140	1.790	3	0	4	0	0	0	7	0
	Greenway Road/SR51 SB Off-Ramp	1	476	3241	0.269	0	0	1	0	0	0	1	0
	Greenway Road/SR51 NB Off-Ramp	1	1027	3176	0.238	1	0	0	0	0	0	1	0
	Rural Road/L202 WB Off-Ramp	3	658	2981	0.824	1	0	2	0	0	0	3	0
	Rural Road/L202 EB Off-Ramp	7	1109	2612	1.881	3	0	4	0	0	0	7	0
	<b>All Yield Control Off-Ramps</b>	<b>24</b>	<b>5832</b>	<b>22676</b>	<b>0.842</b>	<b>8</b>	<b>0</b>	<b>16</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>24</b>	<b>0</b>

\* RTCV = rate per thousand conflicting vehicles

Separate conflict rates for the AM and PM periods were not calculated due to limited sample size. Instead, the conflict observation totals were combined and applied against the total volume exposure over that collective duration. The conflict rate for the group of off-ramp right turn movements segregated by control type was based on the aggregate values of conflicts and volume instead of a simple average of the conflict rate values for each individual off-ramp right turn movement. By doing this, the average for the group is not biased as much by the variability of the conflict rate values caused by the relatively small sample sizes.

**Table 16. Crash Data and Rate Computations for Off-Ramp Right Turn Movements**

		Yearly Crash Rate (per MEV)				
		8/1/00 to 7/31/01	8/1/01 to 7/31/02	8/1/02 to 7/31/03	All Years Mean	Total Crashes
Off-Ramp Right Turn Signal Control	Interchange/Off-Ramp					
	Indian School/SR51 SB Off-Ramp	1.62	1.84	1.86	1.77	55
	Indian School/SR51 NB Off-Ramp	1.26	1.21	0.74	1.07	29
	Glendale Road/SR51 NB Off-Ramp	0.28	0.28	0.38	0.31	10
	Bell Road/L101(W) SB Off-Ramp	1.09	0.97	1.22	1.10	44
	Bell Road/L101(W) NB Off-Ramp	1.64	1.20	1.86	1.57	51
<b>Mean Yearly Crash Rate, per MEV (All Signal Control, All Years)</b>					<b>1.16</b>	
Off-Ramp Right Turn Yield Control	Glendale Road/SR51 SB Off-Ramp	1.38	3.54	2.50	2.47	69
	Cactus Road/SR51 SB Off-Ramp	0.59	0.36	0.24	0.40	10
	Cactus Road/SR51 NB Off-Ramp	2.91	2.87	4.14	3.31	100
	Greenway Road/SR51 SB Off-Ramp	0.10	0.00	0.20	0.10	3
	Greenway Road/SR51 NB Off-Ramp	1.34	1.86	3.22	2.14	65
	Rural Road/L202 WB Off-Ramp	3.34	2.67	3.33	3.11	99
	Rural Road/L202 EB Off-Ramp	4.43	4.64	3.40	4.16	109
	<b>Mean Yearly Crash Rate, per MEV (All Yield Control, All Years)</b>					<b>2.24</b>

		Total Crashes	% of Total Crashes Occurring at*:				
			Loc. 1	Loc. 2	Loc. 3	Loc. 4	Loc. Other
Off-Ramp Right Turn Signal Control	Interchange/Off-Ramp						
	Indian School/SR51 SB Off-Ramp	55			100.0%		
	Indian School/SR51 NB Off-Ramp	29			100.0%		
	Glendale Road/SR51 NB Off-Ramp	10			80.0%	10.0%	10.0%
	Bell Road/L101(W) SB Off-Ramp	44		4.5%	86.4%		9.1%
	Bell Road/L101(W) NB Off-Ramp	51			84.3%	15.7%	

\*refer to location key map

Off-Ramp Right Turn Yield Control	Glendale Road/SR51 SB Off-Ramp	69			94.2%	5.8%	
	Cactus Road/SR51 SB Off-Ramp	10			100.0%		
	Cactus Road/SR51 NB Off-Ramp	100			85.0%	13.0%	2.0%
	Greenway Road/SR51 SB Off-Ramp	3			100.0%		
	Greenway Road/SR51 NB Off-Ramp	65			89.2%	10.8%	
	Rural Road/L202 WB Off-Ramp	99	1.0%		92.9%	4.0%	1.0%
	Rural Road/L202 EB Off-Ramp	109			94.5%	4.6%	1.8%

\*refer to location key map

		Total Crashes	% of Total Crashes by Crash Type						
			Rear-End	Side-swipe	Single Vehicle	Backing	Angle	Ped/Bike	Other
Off-Ramp Right Turn Signal Control	Interchange/Off-Ramp								
	Indian School/SR51 SB Off-Ramp	55	100.0%						
	Indian School/SR51 NB Off-Ramp	29	100.0%						
	Glendale Road/SR51 NB Off-Ramp	10	100.0%						
	Bell Road/L101(W) SB Off-Ramp	44	88.6%	6.8%		2.3%		2.3%	
	Bell Road/L101(W) NB Off-Ramp	51	94.1%	2.0%		2.0%		2.0%	

Percentages may not add up to 100% within sections due to rounding

Off-Ramp Right Turn Yield Control	Glendale Road/SR51 SB Off-Ramp	69	91.3%	2.9%	1.4%		2.9%	1.4%
	Cactus Road/SR51 SB Off-Ramp	10	100.0%					
	Cactus Road/SR51 NB Off-Ramp	100	95.0%	2.0%	2.0%	1.0%		
	Greenway Road/SR51 SB Off-Ramp	3	100.0%					
	Greenway Road/SR51 NB Off-Ramp	65	100.0%					
	Rural Road/L202 WB Off-Ramp	99	90.9%	4.0%	2.0%		1.0%	1.0%
	Rural Road/L202 EB Off-Ramp	109	90.8%	5.5%	1.8%		1.8%	0.9%

Percentages may not add up to 100% within sections due to rounding

		Total Crashes	% of Total Crashes by Injury				
			None	Possible	NIC	IC	Fatal
Off-Ramp Right Turn Signal Control	Interchange/Off-Ramp						
	Indian School/SR51 SB Off-Ramp	55	80.0%	16.4%	3.6%		
	Indian School/SR51 NB Off-Ramp	29	72.4%	27.6%			
	Glendale Road/SR51 NB Off-Ramp	10	90.0%	10.0%			
	Bell Road/L101(W) SB Off-Ramp	44	77.3%	15.9%	6.8%		
	Bell Road/L101(W) NB Off-Ramp	51	68.6%	21.6%	9.8%		

NIC - Non-incapacitating; IC - Incapacitating

Off-Ramp Right Turn Yield Control	Glendale Road/SR51 SB Off-Ramp	69	79.7%	15.9%	4.3%		
	Cactus Road/SR51 SB Off-Ramp	10	80.0%	20.0%			
	Cactus Road/SR51 NB Off-Ramp	100	70.0%	28.0%	2.0%		
	Greenway Road/SR51 SB Off-Ramp	3	100.0%				
	Greenway Road/SR51 NB Off-Ramp	65	70.8%	26.2%	3.1%		
	Rural Road/L202 WB Off-Ramp	99	75.8%	19.2%	2.0%	2.0%	1.0%
	Rural Road/L202 EB Off-Ramp	109	72.5%	21.1%	3.7%	1.8%	0.9%

NIC - Non-incapacitating; IC - Incapacitating

## **Crash Rate Calculations**

The crash rate computations are similar to the conflict rate calculations, but are based on a more robust time and sample. One difference in the rate computations is that the resulting ratio of crashes to exposed volume is multiplied by one million rather than one thousand to account for the greater volume considered over the longer assessment period (in this case three years). Therefore, the crash rate is based on one million “entering” vehicles (MEV) with “entering” constituting off-ramp right turn traffic volumes and the traffic volume on the cross road immediately in front of the off-ramp right turn junction area. Table 16 presents the calculated crash rates and corresponding data summary.

In order to calculate specific yearly crash rates for each off-ramp right turn movement, additional volume data was obtained. Historical average daily traffic (ADT) volumes were researched from governing city and state resources. Usually data was available for the cross road on both sides of the interchange. Occasionally ADT data would only be available for the cross road on one side of the interchange. These data ranged in age from one to five years. All study sites had data pertaining to multiple years and so the most recent years were used to formulate an average growth (or decline) rate. The data collected in-field as part of this project served as the most recent value in the determination.

The calculated growth rates for the study sites ranged from about -3% to about 4% per year. The growth rate was applied to both the off-ramp right turn volume and cross road volume immediately in front of the off-ramp. This included actually increasing the volumes when projecting past yearly volume totals if the growth rate was a negative value. Representations of volumes for previous years were generated from applying the growth or decline rates to the existing volume data collected in 2004.

## **CONCLUSIONS**

The data collected and the details obtained from observations and research allowed for the calculations to be made concerning operations and safety. The interpretation of that data through the results of the calculations lends itself to determining interchange characteristics that influence operations and/or safety. One of these characteristics is the traffic control for the off-ramp right turn movement and is the focus of this study. Therefore, all of the calculation results have presented values that were grouped by the individual off-ramp traffic control device—either signal or yield. The presentation of the information in this manner allows trends specifically related to the traffic control used to surface. The following subsections provide interpretation of the previously presented data and highlight any trends and perspectives.

### **General Operations**

Qualitative observations of off-ramp right turn traffic operations were facilitated through the collection of conflict data. Other opportunities to observe and assess traffic operations were possible during the data collection effort for the off-ramp right turn

control delay study. The following list highlights some important points relating to either operation or safety (or both) for the study sites as a group:

- Motorists disregard the requirement to fully stop at a red signal indication when a signal control is used for the off-ramp right turn movement.
- Due to this motorist disregard, the only significant difference in the off-ramp right turn operations between signal control with right-turn-on red and yield control occurs during the limited portion of the overall interchange cycle length when the off-ramp right turn signal has a green arrow indication.
- The advantage of the green arrow phase associated with a signal-controlled off-ramp right turn movement was perceived to be minimal as compared to a yield-controlled off-ramp right turn movement since a fair amount of motorists were observed not paying attention to the green arrow indication either by 1) looking upstream along the cross road (away from the signal indication) or 2) stopping/slowing in advance of the cross road (in preparing to look upstream) despite the green arrow indication.
- Motorists' tendencies to look upstream along the cross road while advancing towards or being at the junction area (for either signal or yield controlled off-ramp right turn movements) causes hardship on pedestrians attempting to cross the off-ramp right turn lane(s), particularly when crossing from the motorist's right side. This is especially evident at sites using dual off-ramp right turn lanes.
- Pedestrian signal indications can be hazardous when the WALK indication is given to a pedestrian crossing the off-ramp right turn lane(s) from the right of the motorist since off-ramp right turn vehicles are either attempting to turn right on red or yield which is dependent on gaps in the cross road traffic flow. To assess these gaps, the motorist must look in the opposite direction from the pedestrian. This is especially evident at signalized off-ramp right turn locations where the WALK indication is given as soon as the cross road traffic receives its green indication. The width of the interchange coupled with start-up time losses for the cross road through traffic results in the creation of a sufficient gap for off-ramp right turn traffic to enter the cross road on red at the same time the pedestrian WALK indication is given.
- Generally, queue lengths for the off-ramp right turn and off-ramp left turn movements were not long enough to block access to either movement's lane(s). If blockage occurred, it was usually the build-up of off-ramp left turn vehicles blocking the off-ramp right-turn vehicles, which could then usually pass the queue by using the paved shoulder area existing outside of the lane line.
- Heavy off-ramp right turn conditions, primarily at signalized off-ramp right turn locations, would prompt frustrated motorists to try to take every opportunity to enter the cross road by turning during the limited change interval duration between interchange signal phases. This would occasionally lead to off-ramp turn vehicles turning onto the cross road during the end of (or after) the change interval time and narrowly in front of an advancing platoon of vehicles from the cross road through movement or opposing off-ramp left turn movement.
- Some motorists showed the tendency to want to follow the actions of the vehicle immediately in front of them which led to or had the potential to lead to the lag vehicle entering the cross road during insufficient gaps and/or without looking upstream along the cross road.

- Data showed that motorists tended to use the outside (curb) lane about twice as much as the inside lane at the study sites that had dual off-ramp right turn lanes.
- There were some observations of motorists blatantly disregarding the red signal indication at signalized off-ramp right turn locations when they approached the junction area immediately after the yellow arrow phase. Perhaps these motorists were taking advantage of the longer clearance interval at the interchange (as compared to a typical intersection).
- Due to the approach angle of some off-ramp right turn lanes, vehicle deflection (and subsequent speed reduction) were not as enhanced leading to motorist tendencies to continue at their off-ramp speed rather than slowing down to assess the cross road traffic conditions.
- Regularly, off-ramp right turn vehicle queuing would block pedestrian access to the crosswalk across the off-ramp right turn lane(s). This is especially evident at sites using dual off-ramp right turn lanes since the outside lane vehicle must pull closer to the cross road in order to try to see around the off-ramp right turn vehicle occupying the inside off-ramp right turn lane.
- U-turns from the cross road left turn lane could and did conflict with some off-ramp right turn vehicles attempting to turn at the same time. Traffic signs explicitly restricting U-turns from the cross road left turn lane were not observed at any of the sites.

## **Control Delay**

Review of the information and results shown in Table 14 yields some interesting observations. The following are some of the key points derived from the review of the information when considering the different off-ramp right turn traffic control types:

- No discernable trends of increased control delay per vehicle associated with a particular peak time or particular direction when considering all sites.
- Average Control Delay per Vehicle for the AM & PM Peak Period (and Hour)
  - Number of signalized off-ramp right turn movements with average delay of 30+ seconds: 2 off-ramps (2 off-ramps)
  - Number of yield-control off-ramp right turn movements with average delay of 30+ seconds: 0 off-ramps (0 off-ramps)
- Longest Control Delay per Vehicle by Control Type
  - Northbound Off-Ramp Right Turn at SR 51/Glendale Avenue (signal control)—AM Peak Period (and Hour): 51.57 seconds (53.44 sec.)
  - Southbound Off-Ramp Right Turn at SR 51/Greenway Road (yield control)—PM Peak Period (and Hour): 26.52 seconds (28.78 sec.)
- Non-weighted Traffic Control Group Averages of Control Delays for Combined Peaks
  - Signal Control – per Total Vehicles, Peak Periods (and Hours):  
19.24 sec. (19.66 sec.)
  - Signal Control – per Stopped Vehicles, Peak Period (and Hours):  
29.42 sec. (30.75 sec.)
  - Yield Control – per Total Vehicles, Peak Periods (and Hours):  
14.38 sec. (15.00 sec.)

- Yield Control – per Stopped Vehicles, Peak Periods (and Hours):  
23.87 sec. (25.74 sec.)

### **Conflict Rate Comparison**

Conflict data was presented in Table 15 which also included the calculated rates. The overall rates for the control type groups were based on a recalculation of the conflict rate using the summed values for each sample site. An overall average of the crash rates calculated for each site was not deemed appropriate given the variability inherent to conflict observations based on the relatively short observation period as compared to crash rate calculations. The following list remarks on the findings:

- Conflict rates for yield-controlled sites as a group are about 240% greater than the overall rate for the signal-controlled group. However, a statistical t-test reveals that this difference is not significant ( $t_{\text{calc}} = 1.705$ ,  $t_{.05, v=10} = 1.812$ ) because of the variability of the conflict rates at the yield control sites and the small sample size.
- Thirty-two of the thirty-four total conflicts involved two or more vehicles while the remaining two conflicts involved vehicles and bicycles/ pedestrians, which were only observed at signalized off-ramp right turn sites (representing 20% of the conflicts observed at signalized locations).
- Most conflicts occur in Area 3 (refer to Figure 16) regardless of the off-ramp right turn control type. However one-third of the conflicts observed at yield-controlled sites occurred in Area 1.
- The highest conflict rates (per thousand conflicting vehicles) calculated for the individual sites were for the eastbound off-ramp right turn at Loop 202/Rural Road (1.881) and the northbound off-ramp right turn at SR 51/Cactus Road (1.790). These sites also had the largest number of occurrences outside of Area 3, which were in Area 1.

### **Crash Rate Comparison**

Crash data is more robust than the data used to calculate conflict rates. Therefore, the results and conclusions drawn from the crash data should be more indicative of longer-term trends and conditions at the site. The crash data and conflict should be used simultaneously to draw conclusions concerning a particular site and what cause(s) might be contributing to them. The crash information per site was presented in Tables 2 through 13 with a summarization and calculated crash rates shown in Table 16. Overall crash rates for the control type groups were the averaged values of the three-year average crash rate for each site in the group. The following conclusions were developed from the review of this information:

- The average crash rate for yield-controlled sites as a group is almost double the average crash rate for the signal-controlled sites. This ratio is comparable to the conflict rate relationship between the two groups.

- A statistical t-test was performed on the average crash rate data for the yield-controlled sites and the signal-controlled sites. All crash rates were considered, which resulted in no significant difference in the average rates for each group.
- The two sites with the highest average crash rate over the three-year assessment period are also the two sites with the highest conflict rates (eastbound off-ramp right turn at Loop 202/Rural Road and northbound off-ramp right turn at SR 51/Cactus Road).
- The proportion of crashes occurring in Area 3 support the conflict observations that showed this being the most prevalent location for conflict occurrence. However the crash data does show a fair number of more crashes occurring in Area 4 than conflicts observed in the same area.
- The percentage of serious injury crashes (non-incapacitating or worse) for the signal control group (5.3%) is similar to the percentage for the yield-control group (4.0%). However, crashes involving incapacitating injuries and fatalities were found to have occurred at yield-controlled sites only.
- Rear-end crashes are dominant at sites with either control type. There was greater variety of the remaining crash types found to occur at the yield-controlled sites.
- There were a couple of anomalies that were noticed upon reviewing the crash data and rates:
  - The rates on southbound off-ramp right turn movements from SR 51 at Greenway and Cactus Roads were very low when compared to other similar sites (0.10 and 0.40 crashes per million entering vehicles, respectively). It was determined that although these interchanges had been fully operational for three years, the connectivity of SR 51 to the north was limited during this span of time (i.e., the freeway terminated at Bell Road, one mile north of Greenway Road). The crash rates for these sites were calculated on a projection of previous off-ramp right turn traffic using current volume data. However, the current off-ramp right turn volumes are substantially different now as compared to the three-year assessment period since SR 51 extends past Bell Road and connects with Loop 101 today. For the three-year assessment period there was probably very little demand to exit off of southbound SR 51 since the motorist would have just entered onto the freeway one or two miles north of these sites. A sensitivity analysis of the projected off-ramp right turn volume used in the crash rate calculations revealed that drastic reductions in off-ramp right turn volumes to represent the past year conditions only cause minimal increases in the three-year crash rate average for the sites (no change in average at the Greenway interchange and a 0.05 increase at the Cactus interchange).
  - The next anomaly concerns the extreme difference between the average crash rate for the signalized northbound off-ramp right turn at Glendale Avenue and the other signal-controlled off-ramp right turn sites. The average crash rate for the northbound off-ramp right turn site at Glendale Avenue is about 77% lower than the average of the other four signal-controlled off-ramp right turn sites. It does not appear that the lower rate can be attributable to under-reporting of crashes by the responsible law enforcement because the southbound off-ramp right turn crash rate at Glendale Avenue is about

average for the yield-controlled sites. A more likely theory is based on this off-ramp right turn movement being the most congested of all the study sites in the AM peak period. The congestion is caused in part by the cross road only having two eastbound through lanes—the only study site to have such configuration (the others have three through lanes on the cross road). The off-ramp right turn congestion would cause the overall speeds along the off-ramp right turn lane to be reduced due to extensive queuing, which in turn promotes a longer time to react to potential conflicts, namely abrupt braking since all crashes at this site were rear-ends. The cross road congestion also virtually eliminates the opportunities for off-ramp right turn vehicles to turn right on red, so that the off-ramp right turn traffic is usually only turning during times of least potential conflict.

# **CHAPTER 3**

## **OPERATIONAL ASSESSMENT OF DIFFERENT TYPES OF OFF-RAMP RIGHT TURN CONTROL AT SINGLE POINT URBAN INTERCHANGES (SPUI) WITHOUT FRONTAGE ROADS**

### **INTRODUCTION**

This chapter describes the approach, process steps, and analysis results from the operational assessment of different off-ramp right turn controls at SPUIs. The results of this assessment will be used in concert with the safety evaluation and conclusions previously presented to develop suggestions on appropriate control types for off-ramp right turn movement. This information will be presented as a final summarization chapter as part of this report. Four control type scenarios were examined during the process—two variations on signal control and two involving yield control. An iterative analysis process involving a range of off-ramp and interchange volume conditions was used to determine overall operational effectiveness of each control scenario. Data collected at several SPUI sites provided actual data that was used to calibrate a micro-simulation model (CORSIM) that was then used to evaluate numerous combinations of traffic volume conditions and off-ramp control types that would have not been possible to collect at actual SPUI locations.

### **CALIBRATION OF CORSIM MODEL**

In order to effectively use CORSIM to simulate actual traffic conditions, it is best to use actual data to calibrate the software parameters governing the model so that it returns results in line with actual traffic conditions. The data collection undertaken to provide this data was described in the previous chapter. The base CORSIM model representing a SPUI (without frontage roads) was calibrated to create six new models representing the six study sites. The latest version of CORSIM, version 5.1 [13], was used to simulate the interchange operations because it can produce measures of effectiveness (MOEs), like control delay, for each movement on a particular link of the network representing the SPUI. This was particularly important for this project since the evaluation of signalized off-ramp right turn operations would involve a network link accommodating both the off-ramp left and right turn movements. Previous versions would not produce output results for control delay by movement. To promote subsequent comparisons that will be particularly focused on the effects of the off-ramp right turn control type, all six interchanges were represented by the same arrangement of network links, except for any network components intended to vary in order to represent the particular off-ramp right turn control types.

## **Modeling of Off-Ramp Right Turn Lanes**

### *Signal Control*

The off-ramp right turn lanes for a particular SPUI model were designed differently depending on the traffic control used for the movement. For signalized off-ramp right turn control, the right turn movement shares a network link with the off-ramp left turn movements. These movements could not be separated onto distinct entry links because CORSIM is limited to five entry links for the single signal controlling the interchange model—separate links for the off-ramp right turn movements would have created six entry links.

### *Yield Control*

In the case of yield controlled off-ramp right turn movements, there was another adjustment to the model network that was necessary. CORSIM does not allow yield (or other sign control) and signal controlled movements to operate at the same node (in these models, the interchange signal control is located at the central node for the network where all entry and exit links connect). Therefore, to represent yield control of the off-ramp right turn movement, separate links and nodes were created to represent the off-ramp right turn lane(s). This accommodation then brought about another item to address. A separate link serving the off-ramp right turn movement would not be able to be positioned at its realistic location with respect to the interchange because the relatively close proximity to the center of the interchange would create a short upstream link as part of the cross street. Gap-acceptance movements (e.g., yield and right turn on red) in the simulation are driven by CORSIM's interpretation of acceptable gaps in the traffic immediately upstream from the intersection node—in this case, the off-ramp right turn movement intersection node with the cross street. Very short upstream links are interpreted by CORSIM as a very large gap when no traffic is present on that link. If this is the case, then the off-ramp right turn movement would have an unrealistically high movement rate. The only recourse to solve this issue is to orient the off-ramp right turn movement link so that it intersects with the cross street a sufficient distance away from the central interchange node. A separation distance of 610 feet was used and was determined by calculating the equivalent distance for normal gap acceptance behavior within the CORSIM environment when considering the cross street traffic traveling at 45 miles per hour.

### *Dual Off-Ramp Right Turn Movements*

When a dual turn movement is provided in CORSIM, the program attempts to balance traffic volumes between the two lanes making up the turn movement. However, in actual field conditions, drivers may tend to prefer one lane over another based on future downstream turn movements, convenience, or preference for turning right from the right-most lane, especially with right turns on red. Field observations showed that about twice as many drivers tended to use the right-hand lane when turning right from the off-ramp that had two right turn lanes regardless of off-ramp right turn control. To account for this behavior, the two off-ramp right turn lanes were assigned as a “right” lane (for the right-hand lane) and a “through” lane (for the left-hand lane) although both only allowed for a

right turn movement onto the cross street. This convention allowed the proportioning of the right turn traffic volume between the two lanes according to the field data. The drawback to this approach is that the left-hand right turn lane would not be permitted to turn right on red in the signal control scenarios (the yield control scenarios were not adversely affected), which is not too far removed from actual driver behavior when faced with a right on red from the left-hand lane of a two lane approach.

## **CORSIM Parameters and Distributions**

For the most part, the default traffic flow parameters used in CORSIM were determined to provide a reasonable representation of traffic flow at the six modeled interchanges. A few changes were implemented to further refine the model operations in their simulation of actual conditions and results. These changes are outlined below.

### *Turn Speeds*

Left turn movement speeds at SPUIs typically are fairly high as compared to a normal intersection, so the maximum available speed of 44 feet per second was used as the turning speed for left turn traffic. A right turn speed of 19 feet per second was input for right turns that shared a network link with left turns. The CORSIM-determined right turn speeds for links that only accommodated right turn traffic were not modified.

### *Speed Distribution*

A symmetric speed distribution was used in the simulation of the study interchanges in place of the default distribution typically used in the CORSIM software. This alternative distribution was used based on a previous study of single point urban interchanges [14]. The mean speed entered for a particular link of the network comprising the simulated interchange was the 85<sup>th</sup> percentile speed (posted speed observed in the field review) divided by the previously observed standard deviation:

$$(6) \quad v_{mean} = \frac{v_{85}}{1.13}$$

Using this input information, CORSIM then proceeded to assign speeds to the vehicles in the simulation using the correct 85<sup>th</sup> percentile speed.

### *Traffic Arrival Type*

The arrival type for all vehicles was assumed to be random, which was reasonable for the off-ramp traffic flows. However, the cross street at each interchange is coordinated and thus would tend to have a more predictable arrival pattern. Without specific data available or collected for the upstream traffic signals, it was not possible to make any assumptions about cross street traffic arrival type. Even if an arrival type could be determined, CORSIM would only allow one arrival type for all vehicles in the simulation, which would unrealistically affect the off-ramp traffic flows.

## **Field Data Inputs**

The field data pertaining to traffic conditions and interchange signal timing obtained from the data collection effort was input into the models to determine how well they simulated actual traffic conditions. The simulation with these inputs was observed and examined in order to determine further adjustments to the model/software to yield realistic results.

### *Traffic Conditions*

Traffic volumes, truck percentages, and turn percentages composed the available data to enter into the interchange simulation. Turn percentages in CORSIM are limited to the nearest percentage, so simulated turn volumes do not exactly match the field observations. Specifically entering the actual turn volumes obtained from the field was possible, but rejected because even with the actual volumes entered the simulated results would not match the field results exactly and because the future application of the models for later stages of analysis would have been made more cumbersome using this method of volume input.

### *Signal Timings and Coordination*

The six study interchanges are all currently part of coordinated signal systems. As a result, each of the six sites has a fixed signal cycle length. Since a fully actuated intersection does not have a fixed cycle length, unless the cycle length is constrained, they were modeled using the time-based coordination feature in CORSIM. The coordination for each simulated interchange was programmed using the phase times and splits from the interchange timing sheets obtained during the overall data collection task for the project. The “offset” value associated with time-based coordination was not applicable and thus set to zero for simplicity since no other data was available for other upstream coordinated traffic signals.

A few adjustments were necessary to allow CORSIM to accept the actual timings and splits used in the field. For example, many movements had no minimum green times shown on the timing sheets, but zero or some very small value (such as one second) could either not be entered or would produce unrealistically short phase durations, respectively. Therefore, for the phases that did not have a specified minimum green time, 8 seconds was used instead and appeared to provide the best compromise between the controller settings and the observed phase durations in the field.

Phases 2 and 6 represented westbound and eastbound through movements, respectively, at five of the six interchanges. However, at the Loop 202/Rural Road interchange the off-ramp movements are oriented westbound and eastbound and thus do not have a through movement nor Phases 2 and 6, which caused a mismatching of phase numbering and interchange movement type. In order to keep the same phase assignments for all of the interchanges, the phase structure of the Loop 202/Rural Road interchange was “reassigned” so that Phases 2

and 6 were the through movements on Rural Road rather than Phases 4 and 8. All of the other phases at this interchange were altered accordingly, as shown in Figure 17.

Original Phases			Reassigned Phases		
Off-Ramps	Rural Road		Off-Ramps	Rural Road	
1	3	4	3	1	2
5	7	8	7	5	6

**Figure 17. Reassigned Phases for Loop 202/Rural Road Interchange**

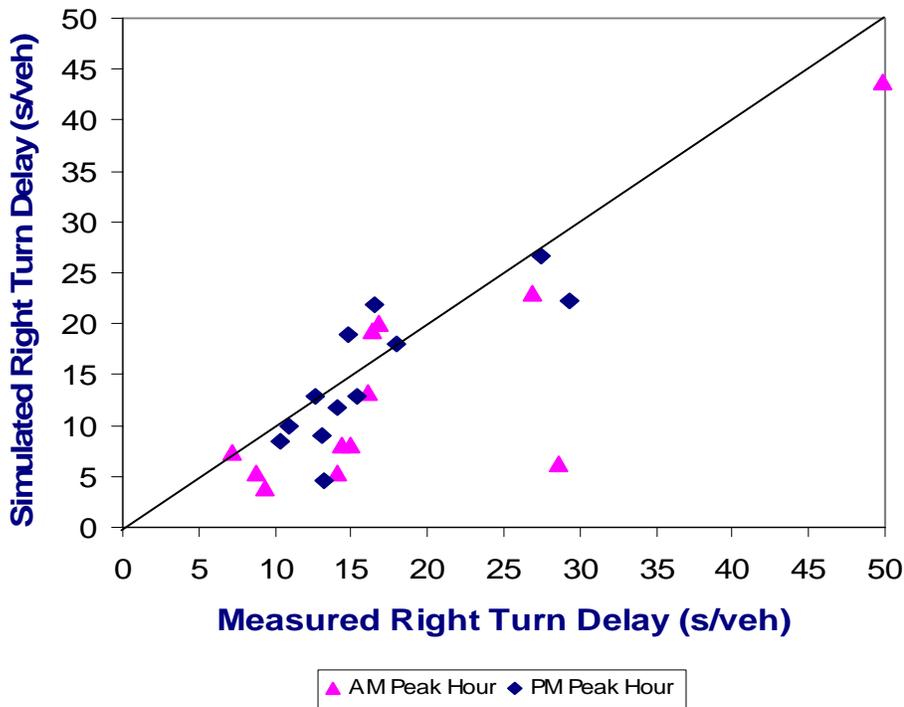
The final step in the CORSIM model calibration process was to observe the simulation operations. If the simulated phases did not reasonably match the field observations of green phase duration, then the coordination parameters were further adjusted until either reasonable agreement was obtained or the limits of the other controller settings (e.g., maximum green time) were reached.

## CALIBRATION RESULTS

The calibration process focused on three interchange parameters which were used to adjust the modeled interchange operations and ultimately served as the basis for comparing the model results with the actual field data/results. The three parameters are the off-ramp right turn delay, green phase durations, and percent of off-ramp right turn vehicles stopping. All six interchanges were analyzed for both the morning and evening peak hours.

### Delay

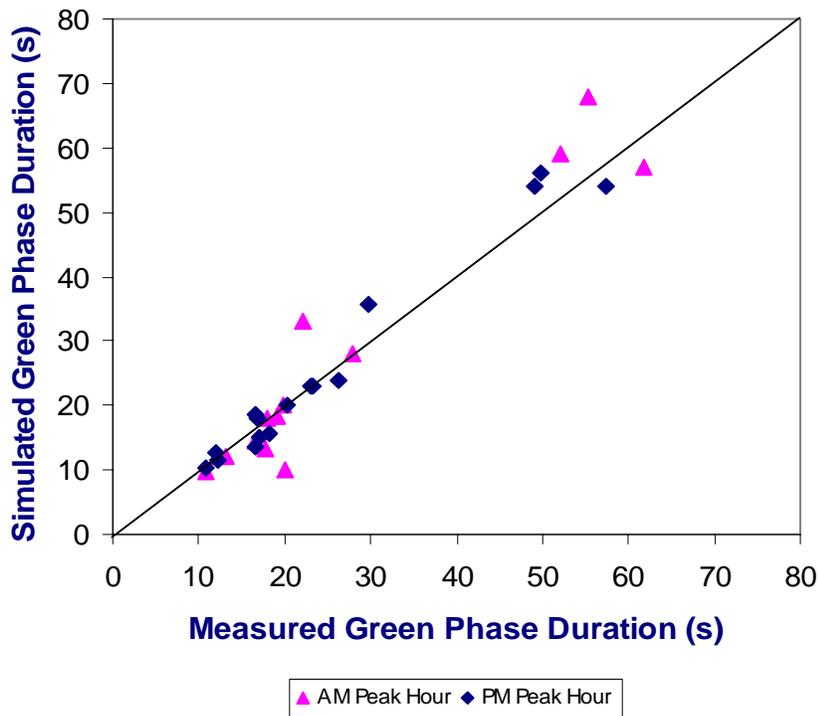
Figure 18 shows the relationship between the off-ramp right turn control delay obtained from the model simulation and the field observations. In general, the predicted off-ramp right turn delay from the model was somewhat less than the observed delay. In part, this is due to CORSIM allowing turns to be completed in shorter gaps than most drivers would typically use and because of the 610-foot spacing between the interchange signal and the yield controlled off-ramp right turn movements. Adjustments to the gap acceptance distribution and the follow-up time for the off-ramp right turn traffic had little effect on the overall tendency of the model to underestimate off-ramp right turn delay. In addition, increasing the follow-up time would randomly cause oversaturation in the off-ramp right turn lane(s) which would result in large variations in simulated delay after only a small change in the follow-up time value. Therefore, the default model values were retained.



**Figure 18. Comparison of Simulated and Field-Measured Delays for the Off-Ramp Right Turn Movement**

## Green Phase Duration

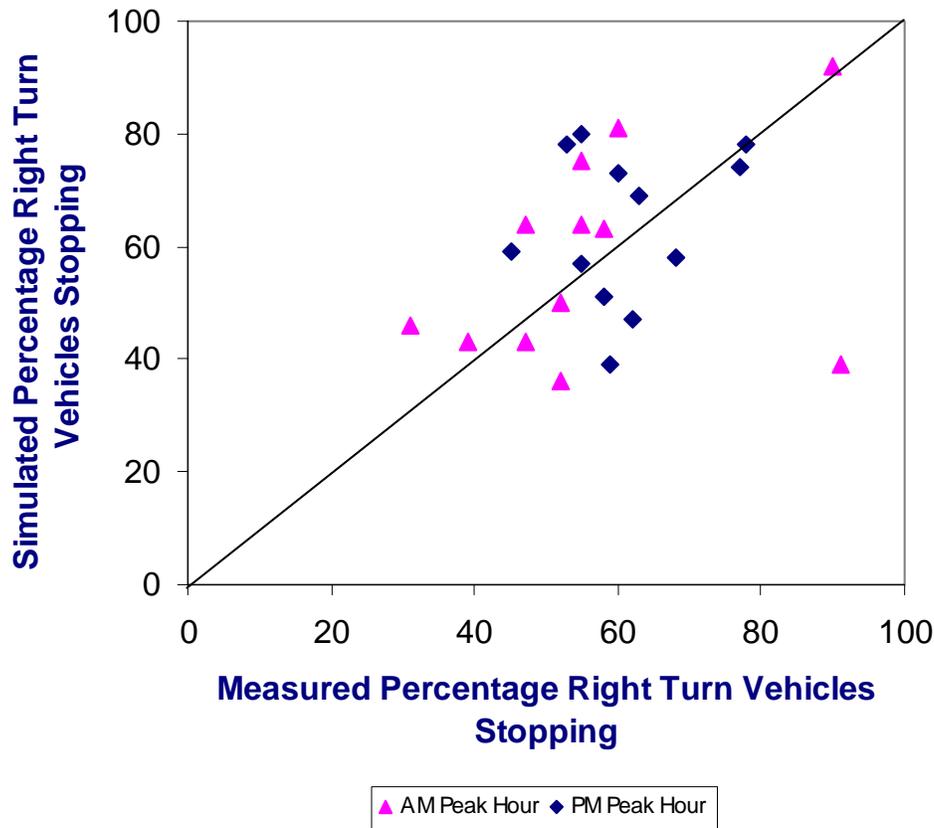
The green phase durations for the cross street through and left turn movements as well as the off-ramp left turn (and right turn when signalized) movements observed in the field compared very closely with the simulated values as shown in Figure 19. This agreement is a result of being able to directly manipulate these values as part of the data input process for the model. The green phase durations longer than 30 seconds shown in Figure 19 were all for the cross street through movements. Since these movements were associated with coordinated signal phases, they would acquire any extra green time that was not used by other movements during a particular signal cycle length. Thus, the variation in the green phase duration of the cross street through phases is higher than the other phases.



**Figure 19. Comparison of Simulated and Field-Measured Green Phase Durations for the Off-Ramp Right Turn Movement**

## Percentage of Off-Ramp Right Turn Vehicles Stopping

Figure 20 shows the relationship between the percentage of off-ramp right turn vehicles stopping in the simulations and in field observations. The relationship shown in the figure is not very strong for several reasons. First, the number of vehicles that have to stop when turning right depends on the current signal phase, traffic from other movements, and whether previously arriving off-ramp right turn vehicles have stopped and are in a queue. The simulation will never be able to match field-observed conditions in this regard. Second, the definition of a “stop” in CORSIM is very restrictive, requiring the simulated vehicle to come to a complete stop. A field-observed stop was based on the definition provided in the *Highway Capacity Manual* [12] which only considers a stop to be when a vehicle has come within a vehicle length of a stopped vehicle and intends to stop itself. Simulated vehicles that roll through a yield sign (or a right turn on red) may not be considered as fully stopped but might have been considered differently in the field. Third, CORSIM may be allowing vehicles to make right turns on red at times when actual drivers would not consider such maneuvers. An example is when CORSIM allows a vehicle to turn right on red when a suitable gap is found in the traffic stream on the approaching link even if this approaching traffic is the beginning of a queue discharge.



**Figure 20. Comparison of Simulated and Field-Measured Percentage of Vehicles Stopping for the Off-Ramp Right Turn Movement**

## **Analysis of Off-Ramp Right Turn Control Types**

The previous chapter presented some analysis and results pertaining to off-ramp right turn operations for the actual study sites. Although those results and determinations are based on actual field data, they are limited in scope to only six SPUI sites. In order to draw broader conclusions concerning the effects of different types of control on the off-ramp right turn movement, more samples and data are required. To facilitate this need a massive amount of field data would have to be collected and processed or the limited real data can be used to develop a working model of SPUIs where the off-ramp right turn control could be varied, as is done in this project. The calibration of the model parameters based on actual field data allows for deviation from the replicated field conditions to experimental/hypothetical situations. This then allowed for the testing of other forms of control while having some confidence that the results would be representative of actual traffic conditions under the same conditions. This section explains the process and results of conducting these analyses using the calibrated CORSIM model of a SPUI.

### **Off-Ramp Right Turn Control Types**

The first step in the analysis process is to determine what control types will be evaluated and contrasted. The two prominent off-ramp right turn traffic control types used in the Phoenix area are signal control and yield control. Therefore, the control types evaluated in the analysis would focus only on these two control types and disregard other options such as stop control or free flow/merge. Within the signal and yield control types, there are other factors that would affect the operation of the off-ramp right turn movement, such as number of right turn lanes, vehicle detection usage/presence, and signal phasing. There are four control types (two variations of signal and yield control) examined in the analysis, which equate to eight control scenarios when considering each control type. Each variation would have two versions for one and two off-ramp right turn lanes. Each control type is described below.

#### *Signal Controlled Off-Ramp Right Turn*

The off-ramp right turn movement can be controlled by signal indications much as any other intersection movement is controlled. The signal head(s) will indicate a green right turn arrow during the portion of the signal cycle when the off-ramp right turn movement is considered protected—in other words during the adjacent cross street left turn phase. At all other times, a red indication would be displayed to off-ramp right turn traffic requiring that the right turn traffic stop and check traffic conditions before turning right (unless otherwise posted, although postings of this nature were not present at the study sites).

This control type has two variations that were assessed in the model analyses. The variations concern the allotment of signal phasing to the off-ramp right turn traffic. One version only gives a green arrow indication to the off-ramp right turn movement during the adjacent cross street left turn phase. This is called an overlap phase and the signal control type employing this phasing is referred to as “Signal 1-phase” within this report. An

example of a study site using this control is the SR 51/Indian School Road interchange. The other variation of the off-ramp right turn signal control type is when there are two phases that can provide the green arrow indication for the off-ramp right turn movement. One of the phases is the overlap phase previously described, and the additional phase is when off-ramp traffic demand is high. This additional phase allows off-ramp left turns and right turns in one direction (or both directions in sequence) to be the only actively controlled movement in the interchange. In this situation, the off-ramp right turn movement receives additional green arrow time to accommodate the traffic demands. This control variation is referred to as “Signal 2-phase” in this report. An example of a study site using this control is the Loop 101/Bell Road interchange.

As mentioned previously, these two control variations within the general signal control type can be applied to off-ramp right turn movements with one or two lanes to bring the number of signal control type scenarios to four within the context of this project.

#### *Yield Controlled Off-Ramp Right Turn*

The yield control type for off-ramp right turn movements seems to be fairly basic with no room for variation. However, this control type was split into two versions incorporating vehicle presence detection or just the standard yield sign with no vehicle detection. The off-ramp right turn control that uses yield signs and vehicle detection works similarly to the Signal 1-phase control, but without the signal head indications for the off-ramp right turn vehicles. Although the off-ramp right turn movement is controlled by the yield signs, a detection loop placed in the right turn lane could be linked to the adjacent cross street left turn phase. If the off-ramp right turn demand exceeded the adjacent cross street left turn demand, then the cross street left turn phase could be prolonged (to a certain degree) to allow additional time where no traffic is conflicting with the off-ramp right turn movement. Essentially the off-ramp right turn traffic would be acting as pseudo cross street left turn traffic. In this report, this control type is called “Yield With Detect.” None of the study sites currently use this control variation. The other yield control version does not incorporate the vehicle detection loop and the off-ramp right turn movement relies on gaps in the cross street traffic in order to enter the roadway. They also can take advantage of the “sheltered” effect when the adjacent cross street left turn phase is active, but only so long as the cross street left turn demand is present. This control type variation is referred to as “Yield No Detect.” All study sites with unsignalized off-ramp right turns have this basic yield control.

The remaining four control type scenarios are represented by the two yield control type variations applied against off-ramp right turn configurations of one and two lanes.

#### **Creation of Model Base Files**

The results of the CORSIM calibration process were interchange models functioning similarly to their corresponding field sites. The field sites and their model equivalents represent almost all of the eight control scenarios previously described. However, variations in lane configurations and timing would make comparisons of two interchanges

complicated when attempting to discern the effects of the off-ramp right turn control type. In order to facilitate the comparison of two models where only the off-ramp right turn control type is varying, new base models had to be created representing the eight control type scenarios. These new base models were created so that they only differed with respect to the off-ramp right turn control type and number of off-ramp right turn lanes. Lane configurations for all movements aside from the off-ramp right turn were maintained based on the typical lane configurations present at actual SPUIs: three cross street through lanes in each direction, two cross street left turn lanes, one cross street right turn lane, and two off-ramp left turn lanes. Also, the particular off-ramp right turn control represented by the base model was applied to both off-ramp right turn movements at the interchange (i.e., no mixing of off-ramp right turn control types). The same general signal timing was used for each base model. It is important to realize that even though these newly created base models were alterations from the models developed to represent the study sites, the inherent parameters governing the simulation and results that were calibrated to actual field conditions remain intact and functional.

### **Determination of Volume Conditions to Analyze**

Once the calibrated base models were developed representing the eight control scenarios, volume distributions had to be developed to act as input for the model simulations. Various volume magnitude and movement distributions are necessary in order to approximate the effects of one off-ramp control type over a range of possible traffic conditions. By applying the same volume distributions to each of the eight base models, then conclusions can be drawn as to how one control type fares versus the others.

The best and readily available source for volume distribution input values was the turning movement data collected at the actual SPUI sites for the AM and PM peak hours. Since the volumes passing through the particular interchanges are somewhat governed by the interchange capacity—lane configuration—the volumes collected were reduced down to per lane equivalents. This would allow the volumes to be applied back to the generic interchange lane configuration employed in all of the base models. Therefore, in some instances the volume distribution inputs for the base models were not exactly equal to the actual traffic conditions observed in the field. For example, a turning movement count for an off-ramp right turn movement with only one lane would be multiplied by two to determine the equivalent volume to input into the base model scenarios where two-lane off-ramp right turn operations are being examined. Similar adjustments would be made for all movements at the interchange in any case where the actual lane configuration differed from the generic layout of the base models. Moreover, a uniform reduction in volume by 30% was applied to ensure that any and all volume scenarios would be applicable for use (i.e., not lead to oversaturated conditions) under any control type scenario. This data provided twelve volume distributions (2 peak hour conditions times 6 study sites) that were analyzed for each of the eight control scenarios.

## **Model Simulation Process**

The process used to simulate the various control/volume scenarios was fairly straightforward in order to promote subsequent comparisons of results. The volume distributions based on the six study sites that were adjusted to conform to the base model interchange layout, including whether the off-ramp movement was in one lane or two lanes, were input into each base model and five simulation runs were conducted. Each of the five runs was initialized with a different random number seed and subsequent trios of simulation runs for other volume/control scenarios were conducted using the same random number seeds. In this manner, an average of the simulations could be computed and said to represent the results of that particular scenario. This average could then be compared against another average knowing that the change in control type/volume distribution was the origin of any differences in results.

In an additional attempt to facilitate meaningful comparisons of control type scenarios, the typical signal timing used in all of the base models was not adjusted in accordance with the particular volume distribution being assessed. Most of the study sites had similar signal timing settings. Even so, some scenarios may be operating under a signal control where the timings are not optimal for the hypothetical conditions. Optimization of the signal timing for each scenario iteration was not conducted since there was no means of optimizing the timing without using some subjective engineering judgment which could bias the simulation outcomes beyond the effects related only to the control type. Since the optimization was not conducted for any of the iterations, all of the results are comparable, albeit skewed towards being inefficient.

## **Simulation Results**

Each simulation run produces an output file containing numerous measures of effectiveness (MOEs) for the links making up the network representation of the interchange, individual movements, lane-by-lane statistics, and overall network (interchange) performance, as well as others. After each control/volume scenario was input and processed in CORSIM, there were a grand total of 480 output files containing the simulation results. A software package tool was used to extract the pertinent data from the simulation output files and tabulate the results for further processing and assessment.

The most effective way to compare the results given the subtlety of the control types/scenarios and the range of interchange volumes assessed was to perform a paired comparison of each control type within the two lane configuration groups (i.e., one lane and two lane off-ramp right turn lanes). The control delay (in average seconds of delay per vehicle) for the entire interchange was selected as the measure of effectiveness since this would represent the comprehensive effect of one control type versus another. The comparison of the control type for each volume scenario (12 in all, AM and PM volumes for each interchange location) used the corresponding "Yield No Detection" control type as the basis for comparison. The proportional differences in interchange control delay between control types were statistically insignificant in every case. Table 17 presents the percent difference for each control scenario averaged for the 12 volume scenarios.

**Table 17. Comparison of Simulation Results for Off-Ramp Right Turn Control Type Scenarios**

Off-Ramp Right Turn Control Variation:			Signal 1-Phase		Signal 2-Phase		Yield With Detection		Yield Without Detection	
			I/C Control Delay (sec/veh)	% Difference from Yield Without Detection	I/C Control Delay (sec/veh)	% Difference from Yield Without Detection	I/C Control Delay (sec/veh)	% Difference from Yield Without Detection	I/C Control Delay (sec/veh)	
One Off-Ramp Right Turn Lane	Bell Rd I/C	AM	17.21	1.42%	19.84	16.91%	17.74	4.51%	16.97	
	Volumes	PM	21.31	2.92%	24.24	17.07%	21.17	2.23%	20.70	
	Cactus Rd I/C	AM	15.78	3.37%	18.36	20.26%	15.98	4.65%	15.27	
	Volumes	PM	16.14	6.39%	18.81	23.98%	16.05	5.79%	15.17	
	Glendale Ave I/C	AM	21.45	1.98%	24.00	14.08%	21.65	2.92%	21.04	
	Volumes	PM	17.30	4.55%	19.59	18.39%	16.64	0.54%	16.55	
	Greenway Rd I/C	AM	15.55	5.85%	17.92	21.96%	14.99	2.01%	14.69	
	Volumes	PM	20.16	21.90%	20.48	23.85%	17.87	8.06%	16.54	
	Indian School Rd I/C	AM	18.10	30.75%	17.46	26.10%	15.00	8.33%	13.84	
	Volumes	PM	18.01	17.77%	18.11	18.41%	15.73	2.81%	15.29	
	Rural Rd I/C	AM	14.97	8.43%	17.35	25.69%	14.56	5.47%	13.81	
	Volumes	PM	16.45	1.66%	22.93	41.70%	16.46	1.70%	16.18	
	Average Percent Difference			8.92%		22.37%		4.09%		
	Two Off-Ramp Right Turn Lanes	Bell Rd I/C	AM	19.85	26.61%	24.36	55.34%	17.58	12.12%	15.68
Volumes		PM	25.31	30.56%	30.53	57.49%	22.32	15.17%	19.38	
Cactus Rd I/C		AM	17.83	28.69%	21.72	56.77%	15.84	14.33%	13.85	
Volumes		PM	21.01	44.87%	47.09	224.73%	17.25	18.93%	14.50	
Glendale Ave I/C		AM	28.47	43.60%	32.85	65.66%	22.06	11.25%	19.83	
Volumes		PM	18.08	7.33%	22.81	35.39%	16.54	-1.82%	16.85	
Greenway Rd I/C		AM	20.40	44.62%	19.50	38.20%	14.93	5.83%	14.11	
Volumes		PM	32.74	106.77%	23.31	47.20%	20.85	31.69%	15.83	
Indian School Rd I/C		AM	18.56	44.75%	21.90	70.86%	14.85	15.86%	12.82	
Volumes		PM	16.73	22.14%	20.82	51.99%	15.34	11.98%	13.70	
Rural Rd I/C		AM	18.52	42.98%	21.16	63.36%	14.14	9.20%	12.95	
Volumes		PM	18.25	20.88%	22.30	47.72%	16.16	7.03%	15.09	
Average Percent Difference			38.65%		67.89%		12.63%			

The results shown in Table 17 indicate that in almost all volume scenarios, the “Yield Without Detection” control type (the basis for the comparisons) has the lowest overall interchange control delay. When averaged interchange control delays were compared, the other control type variations resulted in more delay. In the scenarios with one off-ramp right turn lane, the overall interchange delay for the “Yield With Detection” and “Signal 1-Phase” were not much greater (about 4 and 9 percent more, respectively). The differences in interchange delay were more prominent in the two-lane off-ramp right turn scenarios due to modeling constraints previously discussed, which caused the left hand lane of the two lane off-ramp right turn to experience more delay than necessary in the scenarios with signal control. Therefore, the magnitudes of the percent differences for the signal control types in this two-lane group of scenarios are exaggerated, yet they still reflect the same general relationship as the one-lane group of scenarios. Also, note that these percent differences apply for the normal ranges of interchange volumes and turning movements used in this project. Unusual situations may result in different results for each control type.



## **CHAPTER 4 CONCLUSIONS**

### **INTRODUCTION**

This study intends to draw conclusions regarding right turn control types for off-ramp right turn movements at single point urban interchanges (SPUIs) without frontage roads. The two characteristics of this movement that provided the basis for any determinations were safety and operation. Each of these components is critical to effectively controlling and processing the off-ramp right turn movement at a SPUI. The following information provides a summary of the results and conclusions developed from the preceding procedures and analysis.

### **SAFETY**

The safety evaluation of the six SPUI sites was based on two characteristics, the established three-year crash history for the particular interchange (relating to off-ramp right turn movements only) and the short-term observations of conflicts that occurred during the data collection phase of the project. Although the two sets of results correlate well, there is still room for speculation as to whether the off-ramp right turn control type is the impetus for the particular trends. Extensive safety evaluations of other factors present at the study interchanges were not conducted nor were they possible to conduct within the context of this project.

#### **Crash History Analysis**

The crash history investigations were facilitated by ADOT's Accident Location Identification Surveillance System (ALISS) database, which was queried for the most recent three-year period of crash information (August 1, 2000 through July 31, 2003) at the time of the request. The query consisted of any crashes occurring specifically in the right turn lane(s) on the off-ramp or at the cross road. Crashes reported as occurring on the cross road involving an off-ramp right turn vehicle were also included in the query request. The effective distance for the query was set at 300 feet from the off-ramp right turn/cross road junction point. The resulting number of crash records returned from the query was about 650 for the six interchanges (twelve off-ramps) for the three-year period.

The analysis yielded that the SPUI sites (off-ramps) with signal control tended to have a lower crash rate than the SPUI sites (off-ramps) with yield control over the three-year evaluation period. The mean crash rate for the five off-ramps with signal control was 1.16 crashes per million entering vehicles (entering vehicles were considered off-ramp right turn traffic, conflicting cross street through traffic, and opposing off-ramp left turn traffic). The mean crash rate for the off-ramp right turn movements with yield control was 2.24. Some of the crash rates at selected locations were very low (southbound off-ramp at the Greenway Road and Cactus Road interchanges and the northbound off-ramp at the Glendale Avenue interchange). After confirming that the crash statistics were accurately reported, a sensitivity analysis was conducted on the estimated daily off-ramp traffic

volumes used in the calculations. The analysis indicated that if the daily off-ramp volumes used in the calculation were overestimates (which would yield a lower crash rate), then exaggerated reductions in the volume estimates (yielding higher crash rates) would either have no effect on the overall three-year mean or would have a minimal effect (i.e., the three-year means may be at most 10% more than reported). Therefore, the relatively low mean crash rates were considered valid for inclusion in the assessment of crash rates based on the control type. A statistical t-test reveals that when all average crash rates within each off-ramp right turn control group are considered, there is no significant difference ( $t_{\text{calc}} = 1.510$ ,  $t_{.05, v=10} = 1.812$ ) between the two group averages.

When considering crash data alone, the results seem to indicate that for this limited sample of SPUI off-ramp right turn sites, there is an inclination that signal control tends to be safer, although not to a statistical significance. There are other factors that were not quantified in this study that could be contributing to the crash rates aside from the associated type of off-ramp right turn control. One of these potential substantial factors is the sight distance afforded to the driver of an off-ramp right turn vehicle. Off-ramp right turn drivers actually have too much sight distance when approaching the cross street and may be more focused on looking for gaps in the cross street traffic than on the vehicle(s) in front of them. This condition would lead to a good proportion of rear-end collisions occurring in the off-ramp right turn lanes near the cross street intersection. Crashes occurring due to this situation would be particularly evident at off-ramp right turn lanes controlled by a yield control since anticipating gaps in the cross street traffic would allow the off-ramp right turn vehicle to only slow down rather than stop at the yield line. Other factors that are not accounted for in the mean crash rate determinations include, but are not limited to, off-ramp/cross street intersection angle, potential site-specific hindrances, and the presence or absence of a freeway overpass/underpass.

### **Conflict Observations & Analysis**

As a supplement to the crash history investigations, conflict observations and analyses were conducted for the off-ramp right turn movements at the study sites. Although traffic crash records provide the most direct measure of safety for a roadway location, adequate data may not be available for analysis. Moreover, some crashes are not reported or records may be only available for a time period which may not represent current conditions at the study area. Therefore, conflict data specifically pertaining to the off-ramp right turn movements was collected for the AM and PM peak periods at the study sites.

For the purposes of this study, a conflict was considered to be a traffic event involving two or more road users (e.g., vehicles, pedestrians, bicyclists), in which one or more user performs an abnormal or unusual action causing another or others to execute an abrupt or evasive maneuver to avoid a collision. The most common avoidance maneuver related to the off-ramp right turn movement is either abrupt braking or swerving to avoid a collision.

The results from the conflict rate computations show the same trend as the crash history data. The mean conflict rate for the off-ramp locations with signal control was 0.350 conflicts per one thousand potentially conflicting vehicles. The mean conflict rate for the off-ramp

locations with yield control was more than double at 0.842. Again, the inherent interchange factors described above could also be contributing to the differences in conflict rates in addition to the off-ramp right turn control type. A statistical t-test revealed that this difference in the mean conflict rates was not significant ( $t_{\text{calc}} = 1.705$ ,  $t_{.05, v=10} = 1.812$ ).

## **Safety Conclusions**

Although there may be other contributing factors to the crash/conflict rates for off-ramp right turns controlled by signals and yield signs, it does appear that the difference in rates is cause enough to consider that signal control at SPUI off-ramp right turns may be safer. However, safety is only one of two main components that were evaluated to determine the effectiveness of different control types for the off-ramp right turns. The following section will provide the summarization for the operations analysis of the off-ramp right turns at the study sites.

## **OPERATIONS**

Controlling traffic is a delicate balance of weighing safety concerns against operational efficiencies. Each component has real costs associated with poor performance. The off-ramp right turn operations for the study sites in this project were evaluated in two ways: 1) calculated delays based on actual field-collected data, and 2) simulated operation and delay based on the CORSIM model.

### **Field Measurements & Calculations**

The data collected included interchange turning movement volumes and other aspects of operation specifically related to the off-ramp right turn movement. This collection process and detailed calculations were presented in chapter 2. The collected field data was used to calculate time-in-queue per off-ramp right turn vehicle and number of vehicles stopping per lane per signal cycle length. These values, coupled with information from the HCM [12], were used to calculate the average control delay per off-ramp right turn vehicle.

These calculation procedures were performed for each off-ramp right turn movement at the study sites regardless of the traffic control in place. Even though there was not a portion of the signal cycle length devoted to the off-ramp right turn movements where yield control was used, the cycle length value for the interchange was still considered in the control delay calculations. This assumption is based on the yield control operation being a derivative of gap acceptance in the cross road traffic stream for off-ramp right turn traffic. These gaps are created by the traffic pattern fluctuations and by the cycling of the overall interchange signal control. Since right turn on red is allowed and executed by motorists at all study sites with off-ramp right turn signalization, the differences in the traffic control types from the perspective of off-ramp right turn control delay calculations are subtle.

The calculated delay results are shown in Table 18 (p.70). Since the data component pertaining to number of vehicles that stopped one or more times was collected only for the peak period (i.e., the 1 ½ hour observation period), the peak hour value was pro-rated based on the proportion of time. Since the peak period and peak hour durations were relatively close, this assumption should not have a prominent effect on the peak hour delay calculations.

**Table 18. Calculated Peak Hour Control Delay for Off-Ramp Right Turn Movements at Study Sites**

ORRT Control Type	Off-Ramp Right Turn Signal Control						Off-Ramp Right Turn Yield Control																							
	Indian School Road			Bell Road			Cactus Road			Greenway Road			Rural Road			Glendale Ave														
	SB ORRT	NB ORRT	ORRT	Signal	SB ORRT	NB ORRT	ORRT	Signal	SB ORRT	NB ORRT	ORRT	Yield	SB ORRT	NB ORRT	ORRT	Yield	SB ORRT	NB ORRT	ORRT	Yield	SB ORRT	NB ORRT	ORRT	Yield	SB ORRT	NB ORRT	ORRT	Yield		
<b>Sum of Vehicles in Queue <sup>1</sup></b>																														
AM Peak Hour (actual)	474	186	874	421	421	945			303	61	716	258	457	119																
PM Peak Hour (actual)	119	109	1686	696	85				452	509	296	179	250	183																
<b>Vehicles That Stopped 1+ Times</b>																														
AM Peak Hour (pro-rated from peak period data)	325	149	449	282	304				195	54	275	188	353	149																
PM Peak Hour (pro-rated from peak period data)	94	105	796	496	77				252	262	317	175	269	195																
<b>Total ORRT Vehicles</b>																														
AM Peak Hour	544	273	965	511	336				486	215	140	478	359	319																
PM Peak Hour	229	170	1039	783	146				362	555	336	549	299	267																
<b>Control Delay/Vehicle Computations</b>																														
Number of ORRT Lanes	1	1	2	2	1				1	1	1	1	2	1																
Count Interval (s)	19	19	19	19	19				19	19	19	19	19	19																
<b>Calculations for Peak Hour</b>																														
Control Delay/Total Vehicles (s) AM	16.56	12.95	17.21	15.65	53.44				6.22	26.78	8.28	28.46	13.65	7.09																
Control Delay/Total Vehicles (s) PM	9.87	12.18	30.83	16.89	11.06				16.85	15.47	28.78	10.24	11.37	13.02																
<b>Note</b>																														
1 - Sum of queued vehicles sampled at 19-second intervals. Vehicle stopped more than 19 seconds would be counted more than once.																														

**KEY**  
**ORRT** - Off-Ramp Right Turn; **OR** - Off-Ramp; **LT** - Left Turn; **NB** - Northbound; **SB** - Southbound; **WB** - Westbound; **EB** - Eastbound

The averages for the control type groups suggest that the signal control type may cause more delay to the off-ramp right turn movement. However, when the atypical AM peak conditions for the northbound off-ramp right turn at Glendale Avenue are excluded, there is very little difference in control delay between the two control types (a difference of about 8 seconds for the AM conditions changes to a difference of less than one second). The exclusion of the Glendale data for the AM peak seems reasonable because eastbound Glendale Avenue is the only cross street direction amongst the study sites that has two through lanes—although it still accommodates volume levels commensurate with other sites. The similarity in average delays, further supported by the results for the PM conditions, is despite the inherent characteristic interchange differences which could bias the results.

### **Model/Simulation Analysis**

The interchange and off-ramp right turn movement data collected in the field was limited in scope and thus discounted any conclusions drawn directly from the field data. In order to supplement this data, the field-measured interchange/movement characteristics were used to develop a model of each study site. The simulation of the model was then calibrated to conform to the actual interchange operations, resulting in reasonable approximations of in-field interchange operations. This process allowed the rather limited sample of field data to form the basis for examining a variety of interchange/ traffic situations through the use of the model.

The calibrated model/simulation parameters resulting from the field data inputs were applied to new generic base models of SPUIs that were developed for the express purpose of testing different off-ramp right turn control types. By maintaining as many aspects of the interchange as possible between the base models, aside from the off-ramp right turn control type being evaluated, the results could be interpreted to be directly related to the off-ramp right turn control type being simulated. Base models were created to represent eight different control/interchange scenarios according to control type (signal or yield), the number of off-ramp right turn lanes (1 or 2), and the phasing/detection assumed (phasing for signal control and vehicle detection for yield control). The results of the model simulations were presented in chapter 3. They indicated that the overall interchange control delay was lowest for the simulations associated with the “Yield No Detection” off-ramp right turn control type. The off-ramp right turn control of “Yield With Detection” had the next lowest associated average interchange control delay while the “Signal Two Phase” control type had the highest associated interchange control delay.

### **Operations Conclusions**

Examination of the simulation results shows that yield control (either with or without vehicle detection) for the off-ramp right turn movement is associated with lower average interchange control delays. An assertion for why this trend is evident in the simulation results is that average interchange control delay increases when the off-ramp right turn movement is signaled because the interchange clearance interval/time has to be increased to account for the expanse of the interchange area to include the off-ramp right turn location. The increase in clearance time detracts from the efficiency of the interchange

since it represents time when no traffic movements should be initiating. Other movements at the interchange are also subject to proper clearance times, but because those conflicting movements are occurring closer to the interchange center, the times do not have to be excessive. The off-ramp right turn lanes at SPUIs are purposefully removed from central interchange area and thus when they are signalized they prompt an even more pronounced clearance interval associated with the interchange.

## OVERALL CONCLUSIONS

The efforts executed during this project had the intended goal of determining which control type would be best to use for off-ramp right turn movements at single-point urban interchanges without frontage roads. The data collection effort, both in the field and through the crash databases, resulted in very detailed and beneficial information that was used to its fullest. However, despite the efforts and underlying goal, the results from the safety and operations analyses appear to be contrary, making it necessary to compare the two characteristics using a common basis. Safety and operation can be measured in terms of cost, so the following describes the procedure for estimating the overall yearly costs associated with yield and signal control based on off-ramp right turn data.

### Crash Costs

Table 19 shows data and calculations pertaining to crash costs at five of the six study sites. (The Glendale Avenue interchange was not included in the assessment since its crash data pertained to off-ramps with different types of control and thus could not be integrated with the overall interchange operation/delay computations). The crash cost calculations for each interchange are represented by the number of crashes associated with the off-ramp right turn movement only. Thus, the total crash cost values are not representative of the total crash costs per interchange, but are valid for use in the comparison against interchange operational costs since the unknown crash cost component is assumed to be equal for all the interchanges.

**Table 19. Summarized Crash Data and Estimated Annual Costs at the Study Sites**

		3-Year Crash History				No Injury Crash Costs per Year	Injury Crash Costs per Year	Fatal Crash Costs per Year	Total Crash Costs per Year
		Total Crashes	No Injuries	Injuries*	Fatal				
<b>Interchange/Off-Ramp</b>									
Off-Ramp Right Turn Signal Control	Indian School/SR51 SB Off-Ramp	55	45	10	0	\$ 72,180	\$ 166,057	\$ -	\$ 238,237
	Indian School/SR51 NB Off-Ramp	29	21	8	0	\$ 33,684	\$ 132,845	\$ -	\$ 166,529
	Bell Road/L101(W) SB Off-Ramp	44	34	10	0	\$ 54,536	\$ 166,057	\$ -	\$ 220,593
	Bell Road/L101(W) NB Off-Ramp	51	35	16	0	\$ 56,140	\$ 265,691	\$ -	\$ 321,831
<i>Interchange Avg.</i>									\$ 473,595
Off-Ramp Right Turn Yield Control	Cactus Road/SR51 SB Off-Ramp	10	8	2	0	\$ 12,832	\$ 33,211	\$ -	\$ 46,043
	Cactus Road/SR51 NB Off-Ramp	100	70	30	0	\$ 112,280	\$ 498,170	\$ -	\$ 610,450
	Greenway Road/SR51 SB Off-Ramp	3	3	0	0	\$ 4,812	\$ -	\$ -	\$ 4,812
	Greenway Road/SR51 NB Off-Ramp	65	46	19	0	\$ 73,784	\$ 315,508	\$ -	\$ 389,292
	Rural Road/L202 WB Off-Ramp	99	75	24	0	\$ 120,300	\$ 398,536	\$ -	\$ 518,836
	Rural Road/L202 EB Off-Ramp	109	79	29	1	\$ 126,716	\$ 481,564	\$ 394,962	\$ 1,003,242
<i>* includes crashes involving possible injuries</i>									<i>Interchange Avg.</i> \$ 857,558

The estimated cost of a single crash depends on whether injuries/fatalities were involved. The National Highway Traffic Safety Administration has published *The Economic Impact of Motor Vehicle Crashes 2000* [15] which provides details on the average costs of crashes depending on injuries. The costs are composed of several factors: medical costs, property damage loss, lost productivity (market and household), and other related costs. The average cost for crashes involving property damage only was \$4,812 (in 2004 dollars). Crashes involving injuries of varying degrees have an average cost of \$49,817. Crashes with any fatalities, which are about 75 times less likely to occur as other injury crashes, have an average cost of \$1,184,885 associated with them. The crash costs presented in Table 19 have been averaged to obtain the yearly estimate since the number of crashes shown is for a three-year period. The average yearly cost of crashes for the study interchanges, grouped by off-ramp right turn control type, indicates that interchanges using yield control for the off-ramp right turn movement are about \$384,000 more costly than the interchanges using signal control.

### **Operations Costs**

Table 20 (p.74) presents the operations cost data for the same five study sites where crash data was considered in Table 19. Operational delay data were only available with respect to peak period/hour for this project. Since these delays are only evident for a limited period of the day (assumed to be four hours for the purpose of this exercise), the other portion of the day must be accounted for in order to estimate the daily user costs of delay and equivalent average yearly costs of delay. Based on a similar procedure employed in *Evaluation of Operational Efficiencies, Cost and Accident Experience of Four Phase Single Point Urban Interchanges* [16], average user control delays associated with the off-peak period (eight hours) are estimated to be two-thirds of the peak period average value. Also, the volume processed at an interchange during the eight-hour off-peak period is about 38% greater than the volume processed during the four-hour peak period. The sum of the two is assumed to be representative of the daily traffic total.

Typically, road user costs are based on the 1977 *Manual on User Benefit Analysis* published by AASHTO (updated as of August 2003) [17]. The manual provides user cost information for a number of aspects including “value of time” data (user delay costs), which accounts for a majority of the user costs in this project’s comparison of the control types for off-ramp right turn movements. The value of time is a function of the average hourly wage earned by the persons impacted by the delays (separated by passenger vehicles and trucks), the percentage of the hourly wage considered as the value of time (50% for passenger vehicles, 100% for trucks), and the average passenger occupancy (1.5 for passenger vehicles, 1.05 for trucks). The hourly wages associated with passenger vehicles is \$18.56 per hour and the hourly compensation associated with trucks is \$20.23 per hour. These values are then adjusted by the value of time factors (50% and 100%, respectively for passenger vehicles and trucks) and vehicle occupancy to arrive at value of time figures, in 2000 dollars, of \$13.92 and \$21.24 for passenger vehicles and trucks, respectively. In order to apply these delay cost figures to the calculated delay results from the project, the figures are converted to 2004 dollars using consumer price index (CPI) conversion factors.

**Table 20. Summarized Operations Data and Estimated Annual Costs at the Study Sites**

Interchange/Off-Ramp		Avg. Interchange Control Delay - Peak Hours (2 hr) [sec./veh]	Avg. Peak Period (4 hr) Volume	Avg. Peak Period (4 hr) Vehicle-Hours of Delay	Avg. Peak Period (4 hr) Delay Costs	Avg. Interchange Control Delay - Off-Peak Period (8 hr) [sec./veh]	Off-Peak Period (8 hr) Volume	Off Peak Period (8 hr) Vehicle-Hours of Delay	Off-Peak Period (8 hr) Delay Costs	Average User Delay Costs per Year
Off-Ramp Signal Control	Indian School/SR51 SB Off-Ramp	17.78	19000	93.86	\$ 1,432	11.86	26239	86.42	\$ 1,319	\$ 1,004,121
	Indian School/SR51 NB Off-Ramp									
	Bell Road/L101(W) SB Off-Ramp	27.44	27668	210.92	\$ 3,219	18.30	38210	194.19	\$ 2,963	\$ 2,256,438
	Bell Road/L101(W) NB Off-Ramp									
<i>Interchange Avg. \$ 1,630,279</i>										
Off-Ramp Right Turn Yield Control	Cactus Road/SR51 SB Off-Ramp	15.22	20136	85.13	\$ 1,299	10.15	27808	78.38	\$ 1,196	\$ 910,754
	Cactus Road/SR51 NB Off-Ramp									
	Greenway Road/SR51 SB Off-Ramp	15.62	21728	94.25	\$ 1,438	10.41	30006	86.78	\$ 1,324	\$ 1,008,317
	Greenway Road/SR51 NB Off-Ramp									
Rural Road/L202 WB Off-Ramp	14.51	21000	84.64	\$ 1,292	9.67	29001	77.92	\$ 1,189	\$ 905,422	
Rural Road/L202 EB Off-Ramp										
<i>Interchange Avg. \$ 941,498</i>										

The result is a factor of 1.08, which is applied to the user delay costs of \$13.92 and \$21.24 to obtain \$15.10 (passenger vehicles) and \$23.04 (trucks) in terms of 2004 dollars. A weighted average user delay cost of \$15.26 is calculated based on truck traffic representing 2% (as derived from the collected field data) of the traffic volume.

User cost of delay is selected as the point of comparison between the study interchanges due to its substantial contribution to the overall road user cost. User costs of idling, stopping, and operating speed are all assumed to be equal between the interchanges as well as minimal when compared to the user cost of delay.

The average interchange control delay values presented in Table 20 are based on the average result generated by the multiple models runs (as previously shown in Table 17) for the associated control scenario matching the actual study site configuration. The average yearly cost of delay for the study interchanges, grouped by off-ramp right turn control type, indicates that interchanges using signal control for the off-ramp right turn movement are about \$689,000 more costly.

For use in this comparison only, the total average yearly costs (crash costs + delay costs) for interchanges using signal control for the off-ramp right turn movement are estimated at \$2,100,000. Interchanges that have yield control for the off-ramp right turn movement have an average yearly cost estimate of \$1,800,000. Despite yield control sites appearing to have higher crash rates (although not statistically significant), their overall savings in user cost of delay offsets the increased costs of crashes. However, the difference in total costs does not appear to be substantial, at least not to a degree where the selection of a certain control type would be more convincing than the other. The notes presented from the observations conducted during this project could be used to further refine the operation and safety of the off-ramp right turn movement at SPUIs in lieu of dramatic changes in policies governing type of control since this research concludes that, for all intents and purposes, the signal and yield control types are essentially equal when considering the combined aspects of crashes, operations, and costs.

## **IMPLEMENTATION**

This research project and its associated analysis have determined that neither signal nor yield control has an overwhelming advantage over the other with respect to the combined safety, operations, and costs associated with off-ramp right turn movements at SPUIs without frontage roads. Therefore, suggested implementation of one control over the other is unwarranted. A more extensive research study with an expanded sample of interchanges may yield more detailed conclusions which might suggest changes to current traffic control protocols.



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## **Appendix A**

### **Raw Traffic Volume Data Collected** (Interchange Turning Movement Counts, Automatic Traffic Recorder Counts, & Queuing Data)

Start Date: 1/20/2004  
Start Time: 07:00 AM  
Site Code: 4001

Street Name	From North				From East				From South				From West				INTSEC	HOUR
	SR-51 (SB) OFF RAMP				INDIAN SCHOOL RD				SR-51 (NB) OFF RAMP				INDIAN SCHOOL RD					
Start Time	Left	Thru	Right	RTOR	Left	Thru	Right	RTOR	Left	Thru	Right	RTOR	Left	Thru	Right	RTOR	TOTAL	TOTAL
7:00 AM	4	0	8	2	0	12	2	0	16	0	2	2	0	16	4	0	64	
7:01 AM	0	0	14	5	1	17	4	0	0	0	4	2	5	7	2	0	54	
7:02 AM	4	0	7	2	9	6	4	0	7	0	1	1	7	8	6	0	59	
7:03 AM	4	0	12	3	0	11	4	0	12	0	7	2	0	10	3	0	63	
7:04 AM	0	0	5	0	3	24	4	0	1	0	6	2	3	10	0	0	56	
7:05 AM	6	0	8	1	2	0	5	0	25	0	7	3	6	8	7	0	74	
7:06 AM	3	0	7	2	1	22	5	0	9	0	5	0	0	8	3	0	63	
7:07 AM	0	0	9	2	3	22	5	0	0	0	1	1	7	15	4	0	66	
7:08 AM	6	0	11	4	4	0	3	0	8	0	4	6	9	12	5	0	62	
7:09 AM	6	0	4	1	0	18	2	0	10	0	5	2	0	4	10	0	59	
7:10 AM	6	0	8	3	3	26	4	0	0	0	0	0	7	13	2	0	69	
7:11 AM	2	0	8	1	6	0	3	0	21	0	4	4	4	2	9	0	58	
7:12 AM	5	0	7	5	2	20	0	0	10	0	3	2	4	2	4	0	68	
7:13 AM	1	0	9	5	0	11	5	0	0	0	5	4	14	6	6	0	57	
7:14 AM	6	0	12	5	1	10	3	0	9	0	0	0	0	3	4	0	53	
7:15 AM	4	0	13	5	0	11	1	0	12	0	5	5	0	3	4	0	53	
7:16 AM	5	0	12	2	9	18	4	0	0	0	3	1	5	10	0	0	66	
7:17 AM	1	0	7	2	7	0	5	0	15	0	2	2	6	2	4	0	49	
7:18 AM	4	0	7	2	0	17	4	0	8	0	2	1	0	17	5	0	64	
7:19 AM	1	0	8	3	9	28	6	0	0	0	4	2	4	14	2	0	76	
7:20 AM	5	0	13	0	10	0	5	0	15	0	2	0	4	2	3	0	59	
7:21 AM	4	0	12	2	0	16	3	0	12	0	4	4	0	6	2	0	59	
7:22 AM	1	0	12	3	5	23	4	0	0	0	5	3	4	13	3	0	70	
7:23 AM	1	0	7	2	6	6	4	0	13	0	5	0	6	8	4	0	60	
7:24 AM	3	0	9	1	0	21	1	0	13	0	8	8	0	8	4	0	67	
7:25 AM	1	0	6	3	7	22	8	0	5	0	2	1	10	13	4	0	78	
7:26 AM	2	0	4	2	3	16	5	0	4	0	7	5	2	14	8	0	65	
7:27 AM	7	0	7	5	0	9	7	0	11	0	5	5	1	2	4	0	53	
7:28 AM	0	0	7	2	7	28	4	0	0	0	7	3	5	13	3	0	74	
7:29 AM	7	0	7	3	6	0	4	0	20	0	0	0	8	2	6	0	60	
7:30 AM	4	0	3	2	0	22	2	0	13	0	2	1	0	16	6	0	68	
7:31 AM	0	0	5	2	12	28	5	0	1	0	2	0	8	22	3	0	86	
7:32 AM	3	0	11	2	9	0	5	0	11	0	4	4	6	0	5	0	54	
7:33 AM	7	0	5	1	0	30	4	0	8	0	7	1	0	20	3	0	84	
7:34 AM	6	0	6	1	5	31	4	0	0	0	2	1	7	27	3	0	91	
7:35 AM	0	0	11	5	8	0	2	0	17	0	4	2	9	1	5	0	57	
7:36 AM	7	0	10	7	0	22	3	0	6	0	7	6	0	16	5	0	76	
7:37 AM	4	0	5	0	0	41	10	0	0	0	3	3	0	26	7	0	96	
7:38 AM	1	0	6	4	11	0	2	0	14	0	7	7	9	0	13	0	63	
7:39 AM	10	0	5	2	0	21	5	0	16	0	8	4	0	24	2	0	91	
7:40 AM	0	0	9	3	5	27	7	0	0	0	5	2	9	30	8	0	100	
7:41 AM	1	0	8	2	0	5	6	0	10	0	7	7	3	26	8	0	74	
7:42 AM	7	0	10	1	0	28	3	0	16	0	4	1	0	0	5	0	73	
7:43 AM	7	0	8	3	11	24	9	0	0	0	3	0	11	18	4	0	95	
7:44 AM	5	0	10	4	6	0	4	0	14	0	10	7	2	1	7	0	61	
7:45 AM	9	0	8	0	0	34	4	0	12	0	4	4	0	18	3	0	92	
7:46 AM	4	0	9	6	11	25	5	0	0	0	6	2	5	15	7	0	87	
7:47 AM	4	0	9	2	13	0	2	0	18	0	12	7	4	0	7	0	69	
7:48 AM	6	0	9	4	4	28	9	0	8	0	1	1	6	18	3	0	92	
7:49 AM	5	0	11	6	7	48	4	0	0	0	1	1	0	32	7	0	113	
7:50 AM	4	0	8	3	6	0	9	0	20	0	3	3	9	0	8	0	67	
7:51 AM	3	0	8	1	0	29	0	0	8	0	5	3	0	21	7	0	81	
7:52 AM	0	0	11	3	3	35	7	0	0	0	9	6	5	17	4	0	91	
7:53 AM	8	0	9	0	5	3	6	0	13	0	6	4	4	13	5	0	72	
7:54 AM	5	0	9	3	0	28	4	0	10	0	2	2	5	6	5	0	74	
7:55 AM	0	0	9	4	2	35	4	0	0	0	4	0	1	16	6	0	77	
7:56 AM	10	0	9	0	9	0	4	0	18	0	10	5	7	0	6	0	73	
7:57 AM	5	0	8	2	1	27	1	0	20	0	4	4	0	12	4	0	82	
7:58 AM	0	0	13	5	8	24	12	0	0	0	4	0	3	19	5	0	86	
7:59 AM	9	0	13	5	5	0	3	0	17	0	5	2	4	0	8	0	64	4280
8:00 AM	4	0	8	5	2	29	7	0	9	0	1	1	4	22	2	0	84	4300
8:01 AM	6	0	12	8	0	24	5	0	0	0	3	3	0	33	6	0	89	4335
8:02 AM	1	0	13	4	8	0	4	0	18	0	5	5	10	1	4	0	64	4340
8:03 AM	7	0	8	4	0	19	4	0	8	0	2	2	0	18	4	0	70	4347
8:04 AM	0	0	11	5	1	28	5	0	9	0	0	1	7	19	6	0	91	4382
8:05 AM	8	0	8	6	6	17	1	0	0	0	1	1	3	21	7	0	72	4380
8:06 AM	12	0	5	3	0	11	4	0	17	0	8	4	0	1	1	0	59	4376
8:07 AM	4	0	12	4	12	22	3	0	0	0	1	0	4	15	1	0	74	4384
8:08 AM	7	0	7	6	5	5	4	0	12	0	7	2	6	15	3	0	71	4393
8:09 AM	5	0	10	5	5	10	2	0	3	0	10	6	0	3	5	0	53	4387
8:10 AM	2	0	9	6	3	21	4	0	0	0	2	3	0	17	10	0	68	4386
8:11 AM	4	0	10	6	11	0	3	0	17	0	6	3	13	1	11	0	76	4404
8:12 AM	6	0	8	3	0	25	5	0	18	0	4	4	0	22	7	0	95	4431
8:13 AM	1	0	8	0	8	23	8	0	0	0	6	1	2	21	6	0	83	4457
8:14 AM	9	0	9	3	5	0	7	0	8	0	3	1	11	15	5	0	72	4465
8:15 AM	11	0	9	0	0	19	2	0	15	0	7	3	0	6	6	0	75	4487
8:16 AM	0	0	11	3	8	25	5	0	0	0	4	2	6	16	3	0	78	4499
8:17 AM	9	0	12	5	7	0	7	0	12	0	2	2	3	0	2	0	54	4504
8:18 AM	12	0	9	0	0	24	3	0	12	0	7	7	0	18	4	0	89	4529
8:19 AM	4	0	16	6	5	23	2	0	0	0	1	0	9	15	2	0	77	4530
8:20 AM	0	0	15	4	6	0	3	0	9	0	5	1	3	6	3	0	50	4521
8:21 AM	5	0	11	2	0	25	8	0	8	0	3	1	0	7	4	0	71	4533
8:22 AM	3	0	7	2	1	31	7	0	0	0	1	1	6	15	1	0	72	4535
8:23 AM	4	0	16	4	9	0	2	0	13	0	10	6	5	5	5	0	69	4544
8:24 AM	5	0	5	2	0	26	6	0	18	0	3	2	0	12	3	0	76	4555
8:25 AM	6	0	12	4	3	16	9	0	0	0	0	0	3	19	4	0	72	4549
8:26 AM	1	0	10	5	3	5	4	0	11	0	2	1	5	12	2	0	55	4539
8:27 AM	10	0	7	2	0	10	5	0	5	0	5	5	3	5	4	0	54	4540
8:28 AM	0	0	6	2	4	31	6	0	3	0	6	4	3	27	4	0	90	4556
8:29 AM	9	0	5	2	7	4	2	0	14	0	2	1	6	6	8	0	63	4559
Approach Total	1192	0	804	272	2242	366	1476	400	0	1153	0	388	237	351	1082	428	0	6448
Grand Total	388	0	804	272	16.3%	65.8%	17.8%	0.0%	66.3%	0.0%	33.7%	20.6%	18.9%	58.1%	23.0%	0.0%		
Approach %	32.6%	0.0%	67.4%	22.8%	5.7%	22.9%	6.2%	0.0%	11.9%	0.0%	6.0%	3.7%	5.4%	16.8%	6.6%	0.0%		
Total %	8.0%	0.0%	12.5%</															

Start Date: 1/20/2004  
Start Time: 04:30 PM  
Site Code: 4001

Street Name	From North				From East				From South				From West				INTSEC	HOUR					
	SR-51 (SB) OFF RAMP	Left	Thru	Right	RTOR	INDIAN SCHOOL RD	Left	Thru	Right	RTOR	SR-51 (NB) OFF RAMP	Left	Thru	Right	RTOR	INDIAN SCHOOL RD			Left	Thru	Right	RTOR	
Start Time																						TOTAL	TOTAL
4:30 PM	5	0	3	2	14	26	0	0	0	0	5	2	13	25	7	0	0	0	0	0	0	98	
4:31 PM	13	0	5	3	3	11	12	0	0	6	0	4	3	5	27	5	0	0	0	0	0	91	
4:32 PM	3	0	3	1	0	15	6	0	0	2	0	5	2	0	16	10	0	0	0	0	0	60	
4:33 PM	10	0	4	3	9	25	7	0	0	0	0	2	1	11	27	9	0	0	0	0	0	104	
4:34 PM	4	0	6	4	5	1	10	0	0	8	0	5	3	6	2	5	0	0	0	0	0	52	
4:35 PM	0	0	5	4	3	26	11	0	0	0	0	3	2	4	24	11	0	0	0	0	0	87	
4:36 PM	14	0	3	1	0	34	5	0	0	0	0	5	4	0	32	6	0	0	0	0	0	99	
4:37 PM	5	0	5	3	9	0	9	0	0	9	0	5	3	19	1	8	0	0	0	0	0	70	
4:38 PM	0	0	0	0	2	25	6	0	0	3	0	3	3	8	20	12	0	0	0	0	0	79	
4:39 PM	12	0	6	1	3	26	8	0	0	0	0	5	3	1	27	12	0	0	0	0	0	100	
4:40 PM	5	0	5	4	2	0	10	0	0	11	0	3	0	10	0	6	0	0	0	0	0	52	
4:41 PM	0	0	1	1	5	24	6	0	0	5	0	4	3	11	23	7	0	0	0	0	0	86	
4:42 PM	6	0	7	1	7	30	1	0	0	0	0	1	1	6	49	7	0	0	0	0	0	114	
4:43 PM	4	0	4	3	4	1	1	0	0	10	0	0	0	8	19	4	0	0	0	0	0	55	
4:44 PM	0	0	1	1	0	19	4	0	0	2	0	3	3	4	15	13	0	0	0	0	0	61	
4:45 PM	0	0	1	1	0	29	2	0	0	0	0	6	5	16	34	6	0	0	0	0	0	110	
4:46 PM	13	0	3	2	2	1	6	0	0	7	0	3	1	10	0	4	0	0	0	0	0	49	
4:47 PM	0	0	0	0	6	21	10	0	0	6	0	0	0	1	26	15	0	0	0	0	0	85	
4:48 PM	0	0	2	0	0	40	3	0	0	0	0	3	3	12	52	6	0	0	0	0	0	118	
4:49 PM	6	0	3	1	7	14	7	0	0	4	0	3	2	10	18	5	0	0	0	0	0	77	
4:50 PM	8	0	7	5	1	19	5	0	0	5	0	4	2	6	8	13	0	0	0	0	0	76	
4:51 PM	0	0	4	2	5	28	3	0	0	0	0	2	2	3	30	6	0	0	0	0	0	81	
4:52 PM	9	0	5	4	4	16	3	0	0	8	0	1	1	11	24	10	0	0	0	0	0	91	
4:53 PM	13	0	4	4	0	26	4	0	0	3	0	6	2	0	8	5	0	0	0	0	0	69	
4:54 PM	1	0	5	3	8	15	2	0	0	0	0	1	1	9	22	11	0	0	0	0	0	85	
4:55 PM	11	0	2	2	4	14	4	0	0	4	0	4	4	6	25	11	0	0	0	0	0	59	
4:56 PM	1	0	1	1	0	18	2	0	0	4	0	8	5	1	8	16	0	0	0	0	0	105	
4:57 PM	0	0	0	0	14	3	2	0	0	0	0	2	2	14	33	9	0	0	0	0	0	74	
4:58 PM	11	0	8	2	7	0	13	0	0	7	0	1	1	11	10	6	0	0	0	0	0	79	
4:59 PM	8	0	4	4	0	23	7	0	0	6	0	2	2	4	19	6	0	0	0	0	0	70	
5:00 PM	0	0	7	5	5	13	2	0	0	0	0	3	3	9	26	5	0	0	0	0	0	70	
5:01 PM	8	0	4	0	1	0	4	0	0	11	0	1	1	11	22	8	0	0	0	0	0	82	
5:02 PM	9	0	1	1	0	37	5	0	0	8	0	3	1	0	12	7	0	0	0	0	0	105	
5:03 PM	0	0	5	0	12	27	3	0	0	4	0	5	0	13	27	9	0	0	0	0	0	61	
5:04 PM	8	0	5	2	5	0	10	0	0	5	0	0	1	12	9	7	0	0	0	0	0	77	
5:05 PM	9	0	2	1	0	23	6	0	0	11	0	0	0	0	14	12	0	0	0	0	0	86	
5:06 PM	0	0	6	2	10	31	4	0	0	0	0	1	0	4	28	2	0	0	0	0	0	85	
5:07 PM	3	0	5	2	4	9	7	0	0	3	0	2	1	14	27	11	0	0	0	0	0	81	
5:08 PM	4	0	1	1	0	25	6	0	0	7	0	2	1	11	11	14	0	0	0	0	0	96	
5:09 PM	0	0	8	3	6	28	2	0	0	4	0	3	3	7	28	10	0	0	0	0	0	71	
5:10 PM	9	0	6	1	7	0	5	0	0	2	0	3	3	15	16	8	0	0	0	0	0	79	
5:11 PM	9	0	6	2	0	28	5	0	0	0	0	1	0	7	14	9	0	0	0	0	0	87	
5:12 PM	0	0	2	2	11	21	6	0	0	7	0	4	2	3	27	6	0	0	0	0	0	57	
5:13 PM	14	0	3	0	4	0	5	0	0	7	0	4	0	0	24	13	0	0	0	0	0	93	
5:14 PM	8	0	3	1	4	24	7	0	0	7	0	3	0	0	0	3	0	0	0	0	0	99	
5:15 PM	0	0	4	2	1	35	1	0	0	0	0	3	2	8	41	6	0	0	0	0	0	72	
5:16 PM	7	0	2	1	7	1	6	0	0	3	0	7	1	13	16	10	0	0	0	0	0	105	
5:17 PM	8	0	3	0	2	31	5	0	0	2	0	1	1	9	31	13	0	0	0	0	0	77	
5:18 PM	0	0	4	4	1	23	2	0	0	0	0	1	1	0	41	5	0	0	0	0	0	75	
5:19 PM	12	0	6	4	8	0	1	0	0	7	0	1	1	15	18	7	0	0	0	0	0	90	
5:20 PM	5	0	7	3	3	15	5	0	0	3	0	2	2	12	21	5	0	0	0	0	0	78	
5:21 PM	0	0	6	5	1	34	4	0	0	0	0	2	2	1	38	4	0	0	0	0	0	67	
5:22 PM	14	0	5	2	12	3	7	0	0	6	0	1	1	8	6	5	0	0	0	0	0	77	
5:23 PM	0	0	3	1	0	25	1	0	0	5	0	3	0	0	28	12	0	0	0	0	0	87	
5:24 PM	11	0	4	2	7	22	1	0	0	1	0	1	0	9	24	7	0	0	0	0	0	67	
5:25 PM	16	0	1	1	5	0	5	0	0	4	0	3	1	14	11	8	0	0	0	0	0	83	
5:26 PM	8	0	3	3	0	27	2	0	0	2	0	0	0	7	25	9	0	0	0	0	0	103	
5:27 PM	0	0	4	2	10	30	1	0	0	4	0	2	1	3	43	6	0	0	0	0	0	95	
5:28 PM	13	0	3	3	8	0	4	0	0	3	0	3	2	21	31	9	0	0	0	0	0	80	4888 4:30 PM to 5:30 PM
5:29 PM	9	0	2	0	0	26	1	0	0	4	0	1	1	11	16	10	0	0	0	0	0	100	4890 4:31 PM to 5:31 PM
5:30 PM	0	0	7	4	6	29	2	0	0	0	0	3	1	0	47	6	0	0	0	0	0	76	4875 4:32 PM to 5:32 PM
5:31 PM	10	0	4	0	7	0	6	0	0	9	0	2	0	13	16	9	0	0	0	0	0	64	4879 4:33 PM to 5:33 PM
5:32 PM	9	0	2	2	0	21	3	0	0	6	0	3	0	0	13	7	0	0	0	0	0	113	4888 4:34 PM to 5:34 PM
5:33 PM	0	0	4	1	11	37	4	0	0	0	0	6	3	8	32	11	0	0	0	0	0	61	4897 4:35 PM to 5:35 PM
5:34 PM	8	0	2	1	9	0	4	0	0	5	0	6	2	15	7	7	0	0	0	0	0	94	4904 4:36 PM to 5:36 PM
5:35 PM	14	0	1	1	0	28	2	0	0	6	0	3	1	0	21	19	0	0	0	0	0	99	4900 4:37 PM to 5:37 PM
5:36 PM	0	0	3	1	5	25	2	0	0	0	0	7	4	15	31	7	0	0	0	0	0	61	4891 4:38 PM to 5:38 PM
5:37 PM	11	0	5	4	4	0	4	0	0	4	0	4	0	10	14	5	0	0	0	0	0	94	4906 4:39 PM to 5:39 PM
5:38 PM	11	0	4	2	0	34	4	0	0	3	0	2	2	9	19	8	0	0	0	0	0	99	4905 4:40 PM to 5:40 PM
5:39 PM	0	0	3	2	4	36	7	0	0	0	0	1	0	5	35	8	0	0	0	0	0	73	4926 4:41 PM to 5:41 PM
5:40 PM	9	0	2	0	3	0	10	0	0	9	0	3	0	13	18	6	0	0	0	0	0	90	4930 4:42 PM to 5:42 PM
5:41 PM	11	0	6	1	0	31	8	0	0	5	0	2	2	4	12	11	0	0	0	0	0	96	4912 4:43 PM to 5:43 PM
5:42 PM	0	0	3	1	5	40	3	0	0	0	0	6	4	7	24	8	0	0	0	0	0	57	4914 4:44 PM to 5:44 PM
5:43 PM	6	0	4	1	8	0	4	0	0	6	0	7	1	8	12	2	0	0	0	0	0	87	4940 4:45 PM to 5:45 PM
5:44 PM	7	0																					

Start Date: 2/3/2004  
Start Time: 07:00 AM  
Site Code: 4001

Street Name	From North SR-51 (SB) OFF RAMP				From East GLENDALE RD				From South SR-51 (NB) OFF RAMP				From West GLENDALE RD				INTSEC	TOTAL	TOTAL	
	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	RTOR	Trucks	Left	Thru	Right				Trucks
7:00 AM	0	0	11	0	0	4	5	0	1	0	2	0	0	11	23	7	0	64		
7:01 AM	30	0	12	0	7	13	2	0	11	0	2	2	0	0	8	5	0	90		
7:02 AM	1	0	10	0	0	10	4	0	3	0	6	1	0	3	14	2	0	53		
7:03 AM	28	0	8	0	0	9	16	1	0	5	2	2	0	0	25	4	0	98		
7:04 AM	3	0	6	0	0	3	7	0	10	0	8	1	3	10	8	2	0	57		
7:05 AM	19	0	4	0	0	2	6	6	0	0	5	5	0	0	31	4	0	77		
7:06 AM	15	0	5	0	0	0	12	3	0	7	0	9	2	0	17	4	0	77		
7:07 AM	17	0	10	0	0	5	0	9	0	0	5	5	0	0	32	9	1	87		
7:08 AM	20	0	5	0	0	0	17	5	0	9	0	5	3	0	15	2	0	86		
7:09 AM	11	0	7	0	0	9	0	2	0	0	0	0	0	0	59	6	0	94		
7:10 AM	26	0	6	0	0	0	20	3	0	9	0	9	5	0	3	0	0	85		
7:11 AM	3	0	5	0	0	10	0	6	1	0	0	3	3	0	0	52	6	1	85	
7:12 AM	25	0	5	0	0	0	11	5	0	21	0	7	2	0	9	1	7	77		
7:13 AM	0	0	5	0	0	8	1	3	0	0	0	7	0	0	53	3	0	77		
7:14 AM	34	0	9	0	0	0	23	7	0	13	0	3	0	3	7	5	0	110		
7:15 AM	0	0	6	0	0	3	0	8	0	0	0	10	4	0	0	49	11	0	87	
7:16 AM	31	0	4	0	0	1	17	5	1	16	0	4	4	0	3	9	6	0	96	
7:17 AM	0	0	11	0	0	0	3	10	1	0	0	11	3	0	3	40	6	0	84	
7:18 AM	35	0	5	0	0	7	20	2	3	10	0	5	5	0	0	11	9	0	104	
7:19 AM	0	0	4	0	0	0	2	9	2	0	0	12	5	0	8	34	7	1	76	
7:20 AM	35	0	7	0	0	11	27	9	3	16	0	3	3	0	0	22	6	0	136	
7:21 AM	0	0	8	0	0	0	5	2	0	0	0	13	4	0	3	29	10	0	70	
7:22 AM	34	0	5	0	0	6	21	10	2	19	0	3	4	0	0	21	5	0	124	
7:23 AM	0	0	4	0	0	0	3	8	0	0	0	11	2	0	10	23	10	0	69	
7:24 AM	37	0	4	0	0	3	16	3	0	15	0	1	1	1	0	30	8	0	117	
7:25 AM	0	0	6	0	0	0	6	8	1	3	0	12	5	1	13	17	7	0	72	
7:26 AM	38	0	9	0	0	8	18	0	0	9	0	1	1	0	12	13	10	0	125	
7:27 AM	1	0	5	0	0	0	8	2	1	0	0	10	2	1	0	41	7	0	61	
7:28 AM	27	0	8	0	0	6	16	5	1	8	0	3	3	0	0	41	7	0	121	
7:29 AM	6	0	4	0	0	0	11	2	1	10	0	8	1	0	11	7	8	0	67	
7:30 AM	16	0	3	0	0	5	5	4	0	1	0	2	2	0	0	42	12	0	92	
7:31 AM	15	0	4	0	0	0	24	3	1	5	0	4	3	0	8	10	1	0	74	
7:32 AM	17	0	9	0	0	9	0	14	0	0	0	1	1	0	0	57	10	1	117	
7:33 AM	21	0	8	0	0	0	31	12	0	16	0	5	1	1	10	3	15	0	121	
7:34 AM	8	0	5	0	0	4	0	5	1	0	0	1	1	0	0	60	9	0	92	
7:35 AM	25	0	3	0	0	0	37	8	0	10	0	7	2	2	3	0	7	0	100	
7:36 AM	6	0	7	0	0	9	0	1	0	0	0	1	1	1	0	59	12	0	95	
7:37 AM	28	0	5	0	0	0	25	6	0	10	0	7	3	0	11	2	3	0	97	
7:38 AM	0	0	3	0	0	6	1	8	0	0	0	4	0	0	8	7	12	0	140	
7:39 AM	33	0	8	0	0	0	40	7	0	19	0	9	4	0	1	46	5	1	73	
7:40 AM	0	0	7	0	0	3	1	1	0	0	0	9	4	2	0	6	8	8	0	126
7:41 AM	35	0	6	0	0	0	33	10	0	16	0	4	2	0	5	40	6	0	87	
7:42 AM	0	0	2	0	0	0	1	4	1	0	0	9	1	0	0	15	5	0	115	
7:43 AM	32	0	7	0	0	7	13	11	1	24	0	1	1	0	8	33	4	0	89	
7:44 AM	0	0	5	0	0	0	2	5	0	0	0	12	3	0	0	24	7	0	150	
7:45 AM	34	0	2	0	0	9	48	7	1	15	0	4	4	1	8	30	5	0	81	
7:46 AM	0	0	4	0	0	0	7	3	0	0	0	4	0	1	0	26	13	0	160	
7:47 AM	34	0	5	0	0	10	44	7	0	20	0	1	1	2	0	24	8	0	71	
7:48 AM	0	0	7	0	1	0	4	9	0	0	0	12	3	0	7	25	7	0	116	
7:49 AM	35	0	4	0	0	8	17	8	1	10	0	2	2	0	0	20	7	0	80	
7:50 AM	0	0	4	0	0	0	3	5	1	0	0	12	4	0	0	37	9	0	140	
7:51 AM	34	0	5	0	0	11	23	4	0	15	0	2	2	0	6	12	8	0	59	
7:52 AM	0	0	3	0	0	0	11	2	0	8	0	9	1	0	0	43	8	0	131	
7:53 AM	29	0	4	0	0	12	20	3	1	15	0	10	1	0	9	9	3	0	86	
7:54 AM	5	0	11	0	0	0	15	9	2	4	0	2	2	0	0	43	5	1	100	
7:55 AM	13	0	5	0	0	5	8	5	0	15	0	7	2	0	9	0	9	0	79	
7:56 AM	13	0	2	0	0	9	7	5	0	0	0	3	2	0	0	51	4	0	94	
7:57 AM	17	0	6	0	0	0	23	7	0	17	0	11	2	0	7	0	5	0	93	
7:58 AM	10	0	2	0	0	5	0	11	0	0	0	3	3	0	0	55	9	0	95	
8:00 AM	25	0	5	0	0	0	35	5	0	22	0	5	2	2	4	0	6	0	111	
8:01 AM	3	0	9	0	0	6	0	5	0	0	0	5	1	0	0	55	6	1	89	
8:02 AM	32	0	4	0	0	0	19	9	0	20	0	5	3	0	13	1	6	0	109	
8:03 AM	1	0	4	0	0	6	0	4	1	8	0	6	3	0	1	46	14	0	82	
8:04 AM	32	0	10	0	0	1	29	12	1	8	0	3	0	1	6	5	5	0	111	
8:05 AM	0	0	3	0	0	4	5	9	0	0	0	8	5	0	0	43	12	0	94	
8:06 AM	36	0	6	0	0	0	26	9	1	14	0	5	3	0	5	7	8	0	118	
8:07 AM	0	0	5	0	0	0	0	11	0	0	0	8	1	0	7	42	2	0	75	
8:08 AM	14	0	5	0	0	8	23	12	2	3	0	1	0	1	0	19	6	0	121	
8:09 AM	0	0	2	0	0	0	4	5	0	8	0	8	3	1	14	30	5	0	76	
8:10 AM	29	0	9	0	0	7	14	4	0	0	0	2	2	0	0	19	6	0	90	
8:11 AM	0	0	6	0	0	0	3	8	0	16	0	5	4	1	11	24	8	0	81	
8:12 AM	30	0	7	0	0	4	17	10	0	0	0	2	2	0	0	22	5	1	97	
8:13 AM	1	0	3	0	0	0	5	5	2	16	0	9	5	0	10	21	2	0	72	
8:14 AM	28	0	5	0	0	7	13	4	0	18	0	0	0	0	0	24	3	0	102	
8:15 AM	0	0	2	0	0	0	5	8	1	4	0	10	4	0	18	18	11	0	76	
8:16 AM	34	0	4	0	0	8	17	6	1	13	0	2	2	0	0	31	4	0	119	
8:17 AM	0	0	4	0	0	0	8	10	1	8	0	11	3	1	15	12	4	0	72	
8:18 AM	24	0	8	0	0	15	12	11	3	7	0	2	2	0	0	37	8	0	124	
8:19 AM	2	0	7	0	0	0	7	4	0	18	0	11	1	0	19	9	4	0	81	
8:20 AM	20	0	6	0	0	9	2	5	0	0	0	1	1	1	0	38	6	0	87	
8:21 AM	9	0	4	0	0	0	21	4	0	15	0	9	3	1	17	5	7	0	91	
8:22 AM	13	0	6	0	0	4	0	6	0	0	0	2	2	0	0	40	5	0	76	
8:23 AM	14	0	6	0	0	0	31	11	2	15	0	4	1	0	14	8	6	0	109	
8:24 AM	8	0	7	0	0	5	0	12	0	0	0	0	0	1	0	47	2	2	81	
8:25 AM	22	0	2	0	0	0	31	4	2	8	0	8	2	1	8	0	3	0	86	
8:26 AM	5	0	7	0	0	7	0	4	1	0	0	1	1	0	0	47	11	0	82	
8:27 AM	24	0	3	0	0	0	35	7	1	20	0	5	2	1	9	0	4	0	107	
8:28 AM	0	0	11	0	0	8	3	6	0	0	0	2	2	0	0	50	7	2	87	
8:29 AM	31	0	4	0	0	0	14	5	0	11	0	6	3	0	8	4	3	0	86	
Approach Total	1941	0	513	0	1	2010	1147	557	46	1153	0									

Start Date: 2/3/2004  
Start Time: 04:30 PM  
Site Code: 4001

Street Name	From North SR-51 (SB) OFF RAMP					From East GLENDALE RD					From South SR51 (NB) OFF RAMP					From West GLENDALE RD					INTEC TOTAL	HOUR TOTAL
	Left	Thru	Right	Yield	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	RTOR	Trucks	Left	Thru	Right	Trucks				
4:30 PM	6	0	0	0	0	5	29	14	0	6	0	5	5	0	6	9	11	0	83			
4:31 PM	9	0	7	0	0	5	12	14	2	8	0	2	2	0	0	15	6	0	78			
4:32 PM	0	0	3	0	0	0	31	22	0	0	0	1	1	0	11	25	9	0	102			
4:33 PM	7	0	5	0	0	8	34	20	0	14	0	4	4	0	0	2	8	0	102			
4:34 PM	10	0	3	0	0	0	5	21	0	12	0	2	1	0	12	17	6	0	88			
4:35 PM	2	0	4	0	0	7	36	16	1	0	0	2	1	0	16	23	10	0	116			
4:36 PM	3	0	5	0	0	5	35	17	0	11	0	4	2	0	0	1	6	0	85			
4:37 PM	13	0	0	0	0	0	9	17	0	6	0	3	2	0	11	16	10	0	118			
4:38 PM	2	0	7	0	0	11	34	13	1	2	0	3	2	0	9	2	6	0	111			
4:39 PM	13	0	7	0	0	5	31	23	0	17	0	0	0	0	1	20	9	0	101			
4:40 PM	13	0	5	0	0	4	8	24	0	10	0	3	2	0	13	16	9	0	104			
4:41 PM	0	0	5	0	0	0	35	21	0	1	0	4	3	0	17	22	8	0	114			
4:42 PM	5	0	6	0	0	9	24	11	0	9	0	3	3	0	0	2	4	0	69			
4:43 PM	13	0	4	0	0	0	7	25	0	10	0	4	4	0	11	25	9	0	112			
4:44 PM	1	0	3	0	0	0	46	15	0	0	0	2	2	0	15	9	5	0	113			
4:45 PM	4	0	0	0	0	16	32	18	0	11	0	3	2	0	0	16	10	0	122			
4:46 PM	7	0	4	0	0	3	15	9	0	7	0	0	0	0	0	16	10	0	71			
4:47 PM	0	0	5	0	0	0	51	16	0	0	0	3	2	0	15	21	11	0	122			
4:48 PM	13	0	5	0	0	6	47	16	0	17	0	1	0	0	10	0	10	1	125			
4:49 PM	15	0	6	0	0	5	9	23	2	11	0	0	0	0	1	15	12	0	97			
4:50 PM	0	0	2	0	0	1	44	22	0	0	0	3	1	0	14	19	7	0	112			
4:51 PM	16	0	9	1	1	16	24	18	1	6	0	2	2	0	11	3	2	0	107			
4:52 PM	13	0	9	0	0	0	10	25	0	9	0	0	0	0	1	20	4	0	102			
4:53 PM	2	0	4	0	0	0	36	21	0	0	0	0	0	0	15	27	13	0	116			
4:54 PM	17	0	1	0	0	5	30	18	0	6	0	3	3	0	17	1	6	0	104			
4:55 PM	12	0	5	1	1	6	7	18	0	4	0	2	2	0	0	19	10	0	83			
4:56 PM	2	0	4	0	0	0	38	17	0	0	0	2	0	0	14	22	8	0	107			
4:57 PM	1	0	5	0	0	8	28	18	1	8	0	3	2	0	17	9	5	0	102			
4:58 PM	9	0	6	0	0	10	20	20	0	6	0	3	3	0	0	17	8	0	99			
4:59 PM	3	0	4	0	0	0	38	14	0	0	0	1	1	0	17	23	4	0	104			
5:00 PM	0	0	5	0	0	3	38	10	0	10	0	5	4	0	14	4	9	0	98			
5:01 PM	13	0	5	0	0	7	5	17	0	6	0	3	3	0	0	10	4	0	70			
5:02 PM	0	0	6	0	0	0	43	17	0	0	0	2	0	0	20	23	15	0	126			
5:03 PM	5	0	6	0	0	11	22	15	0	11	0	0	0	0	20	1	9	0	100			
5:04 PM	5	0	3	0	0	8	20	8	0	7	0	5	4	0	0	20	8	0	84			
5:05 PM	3	0	5	0	0	0	37	20	1	0	0	2	3	0	18	25	12	0	124			
5:06 PM	15	0	8	0	0	4	46	19	1	17	0	4	2	1	16	3	8	0	138			
5:07 PM	14	0	8	0	0	9	19	19	0	6	0	1	1	0	0	16	11	0	103			
5:08 PM	4	0	3	0	0	0	31	15	0	0	0	2	2	0	13	23	10	0	101			
5:09 PM	12	0	3	0	0	4	32	17	1	7	0	1	0	1	17	1	7	0	101			
5:10 PM	11	0	4	0	0	10	8	24	0	5	0	2	2	0	0	22	9	0	95			
5:11 PM	6	0	3	0	0	1	43	13	0	0	0	1	0	0	14	19	8	0	108			
5:12 PM	5	0	7	0	0	18	42	15	0	5	0	5	5	0	1	1	13	0	112			
5:13 PM	8	0	3	0	0	0	5	18	1	7	0	1	1	0	4	18	7	0	71			
5:14 PM	3	0	5	0	0	8	31	21	0	0	0	2	1	0	18	25	7	0	120			
5:15 PM	2	0	4	0	0	2	39	13	0	7	0	3	1	0	21	16	4	0	111			
5:16 PM	9	0	2	0	0	4	10	14	0	9	0	7	4	0	0	23	6	0	84			
5:17 PM	5	0	1	0	0	0	32	14	1	0	0	2	2	0	15	10	8	0	117			
5:18 PM	6	0	0	0	0	6	46	17	1	7	0	2	2	0	0	18	11	0	76			
5:19 PM	4	0	6	0	0	0	5	18	0	5	0	5	5	0	0	18	11	0	76			
5:20 PM	4	0	5	0	0	0	40	14	0	0	0	2	2	0	12	19	6	0	102			
5:21 PM	15	0	3	0	0	5	42	19	0	11	0	1	0	0	20	1	5	0	122			
5:22 PM	13	0	10	1	1	7	8	19	0	9	0	4	3	0	2	25	11	0	108			
5:23 PM	5	0	5	0	0	0	32	12	0	0	0	2	1	0	15	27	7	1	105			
5:24 PM	3	0	2	0	0	1	34	15	0	4	0	3	2	0	17	10	14	0	103			
5:25 PM	14	0	5	0	0	5	4	18	0	12	0	3	3	0	0	11	1	0	73			
5:26 PM	5	0	1	0	0	0	45	11	0	0	0	0	0	0	14	25	8	0	109			
5:27 PM	5	0	5	0	0	8	32	12	0	9	0	1	1	0	14	6	10	0	102			
5:28 PM	7	0	4	1	1	9	5	15	0	6	0	2	2	0	0	24	7	0	79			
5:29 PM	3	0	6	0	0	0	40	11	0	0	0	2	2	0	15	24	10	0	111			
5:30 PM	10	0	1	0	0	4	46	17	0	7	0	4	3	0	18	0	6	0	113			
5:31 PM	7	0	7	0	0	4	7	11	0	3	0	1	0	0	1	18	3	0	62			
5:32 PM	4	0	5	0	0	4	40	17	0	0	0	1	3	0	14	21	5	0	106			
5:33 PM	3	0	4	0	0	5	5	14	1	9	0	2	2	0	18	10	6	0	113			
5:34 PM	19	0	9	0	0	0	42	14	2	0	0	2	2	0	0	19	4	0	85			
5:35 PM	6	0	7	0	0	6	24	16	0	0	0	1	1	0	15	13	10	0	94			
5:36 PM	5	0	4	1	1	6	24	16	0	0	0	1	1	0	0	6	5	0	69			
5:37 PM	14	0	7	0	0	2	8	19	1	6	0	2	1	0	15	23	12	0	111			
5:38 PM	6	0	2	0	0	0	39	13	0	0	0	1	1	0	14	7	6	0	106			
5:39 PM	2	0	6	0	0	7	33	20	1	6	0	5	3	0	11	24	5	0	117			
5:40 PM	7	0	9	0	0	6	16	15	0	4	0	1	1	0	0	17	10	0	85			
5:41 PM	7	0	5	0	0	0	44	19	0	0	0	2	2	0	12	10	6	0	120			
5:42 PM	4	0	4	0	0	4	49	21	0	7	0	2	2	0	0	17	2	0	83			
5:43 PM	9	0	7	0	0	4	7	27	0	8	0	3	3	0	13	30	4	0	118			
5:44 PM	4	0	2	0	0	1	46	18	0	0	0	0	0	0	11	14	10	0	114			
5:45 PM	2	0	3	0	0	5	44	19	0	5	0	1	1	0	0	13	4	0	69			
5:46 PM	11	0	2	0	0	4	9	22	0	3	0	1	1	0	0	13	4	0	69			
5:47 PM	7	0	3	0	0	0	41	19	0	0	0	3	2	0	17	22	11	0	123			
5:48 PM	3	0	5	0	0	8	34	17	0	4	0	3	3	0	16	16	3	0	109			
5:49 PM	11	0	13	0	0	3	13	19	0	6	0	2	2	0	0	10	8	0	85			
5:50 PM	7	0	4	0	0	0	37	19	2	0	0	2	2	0	15	17	10	1	111			
5:51 PM	5	0	7	0	0	1	41	18	1	8	0	5	1	0	9	1	7	0	102			
5:52 PM	8	0	9	0	0	5	5	19	0	5	0	1	1	0	0	26	6	0	94			
5:53 PM	6	0	3	0	0	0	46	23	1	0	0	0	0	0	13	25	4	0	120			
5:54 PM	2	0	5	0	0	2	41	22	1	3	0	0	0	0	13	15	9	0	112			
5:55 PM	9	0	8	0	0	2	3	19	1	9	0	6	5	0	0	10	8	0	72			
5:56 PM	9	0	7	0	0	0	39	25	0	0	0	1	0	0	6	22	5	0	114			
5:57 PM	3	0	8	0	0	1	44	20	0	0	0	1	1	0	13	23	8	0	126			
5:58 PM	6	0	5	1																		



Start Date: 1/27/2004  
Start Time: 04:30 PM  
Site Code: 4001

Street Name	From North SR-51 (SB) OFF RAMP				From East GREENWAY RD				From South SR-51 (NB) OFF RAMP				From West GREENWAY RD				INTSEC	TOTAL	TOTAL
	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks			
Start Time																			
4:30 PM	5	0	6	1	18	8	1	0	4	0	15	0	7	26	10	0	100		
4:31 PM	0	0	8	0	7	54	2	0	16	0	4	0	2	4	14	0	111		
4:32 PM	18	0	7	0	14	2	2	0	0	0	4	0	1	26	6	0	80		
4:33 PM	0	0	5	0	0	53	8	0	20	0	11	0	0	0	9	0	106		
4:34 PM	7	0	4	1	13	2	0	0	1	0	11	0	9	28	6	0	81		
4:35 PM	0	0	3	0	2	47	10	0	21	0	12	0	3	1	9	0	108		
4:36 PM	11	0	10	0	21	7	4	0	0	0	11	0	0	26	5	0	95		
4:37 PM	0	0	3	0	0	56	6	0	19	0	10	0	6	1	9	0	110		
4:38 PM	17	0	7	0	16	1	1	0	0	0	7	0	3	38	4	0	94		
4:39 PM	0	0	6	0	0	49	2	0	19	0	12	0	4	1	3	0	96		
4:40 PM	8	0	5	0	22	2	1	0	0	0	16	0	0	39	11	0	104		
4:41 PM	0	0	4	0	0	51	2	0	14	0	11	0	7	2	8	0	99		
4:42 PM	16	0	7	0	22	2	2	0	0	0	9	0	2	46	5	0	111		
4:43 PM	0	0	7	0	0	65	6	0	15	0	9	0	0	1	4	0	107		
4:44 PM	8	0	6	0	18	5	1	0	2	0	13	0	6	27	1	0	87		
4:45 PM	0	0	0	0	0	58	8	0	21	0	8	0	0	4	5	0	104		
4:46 PM	0	0	9	0	18	0	1	0	6	0	12	0	6	35	8	0	95		
4:47 PM	15	0	4	0	8	62	6	0	19	0	6	0	0	4	5	0	129		
4:48 PM	0	0	6	0	13	4	2	0	3	0	6	0	8	29	5	0	76		
4:49 PM	13	0	5	0	2	49	4	0	20	0	14	0	0	0	5	0	112		
4:50 PM	1	0	2	0	11	4	0	0	4	0	7	0	5	45	2	0	81		
4:51 PM	14	0	8	0	0	41	8	0	11	0	8	0	6	1	8	0	105		
4:52 PM	0	0	3	0	15	4	0	0	0	0	10	0	0	30	5	0	67		
4:53 PM	7	0	6	0	0	34	2	0	15	0	11	0	0	3	11	0	89		
4:54 PM	0	0	5	0	18	24	1	0	0	0	8	0	0	26	3	0	85		
4:55 PM	18	0	4	0	0	33	2	0	19	0	10	0	0	4	7	0	97		
4:56 PM	0	0	3	0	19	2	4	0	1	0	9	0	10	25	5	0	78		
4:57 PM	12	0	7	1	1	60	1	0	19	0	8	0	4	5	4	0	121		
4:58 PM	0	0	5	0	13	0	2	0	0	0	2	0	2	31	5	0	60		
4:59 PM	11	0	6	0	0	47	9	0	23	0	6	0	0	1	6	0	109		
5:00 PM	0	0	5	0	10	1	1	0	2	0	7	0	4	40	7	0	77		
5:01 PM	13	0	7	2	0	66	3	0	22	0	6	0	0	4	4	0	127		
5:02 PM	0	0	5	0	13	0	0	0	4	0	6	0	4	20	6	0	58		
5:03 PM	0	0	6	0	0	47	8	0	10	0	8	0	5	2	6	0	107		
5:04 PM	15	0	6	0	15	0	3	0	0	0	6	0	0	38	10	0	72		
5:05 PM	19	0	8	0	0	59	10	0	21	0	6	0	3	1	3	0	130		
5:06 PM	0	0	4	0	22	2	1	0	0	0	8	0	3	31	7	0	78		
5:07 PM	10	0	8	0	5	47	3	0	15	0	8	0	7	3	8	0	114		
5:08 PM	0	0	2	0	12	1	3	0	0	0	14	0	0	26	3	0	81		
5:09 PM	13	0	8	0	0	51	4	0	18	0	7	0	0	2	11	0	114		
5:10 PM	0	0	7	0	13	0	2	0	0	0	11	0	6	33	0	0	72		
5:11 PM	15	0	9	0	0	54	6	0	19	0	6	0	5	2	6	0	122		
5:12 PM	0	0	5	0	25	3	1	0	0	0	12	0	3	39	8	0	96		
5:13 PM	19	0	8	0	0	70	7	0	21	0	4	0	0	1	10	0	140		
5:14 PM	0	0	4	0	9	0	0	0	3	0	13	0	5	31	10	0	75		
5:15 PM	11	0	4	0	0	60	12	0	15	0	8	0	2	1	5	0	118		
5:16 PM	0	0	9	0	17	5	0	0	0	0	9	0	5	41	8	0	94		
5:17 PM	8	0	3	0	0	69	8	0	21	0	10	0	0	2	5	0	126		
5:18 PM	0	0	11	0	14	2	0	0	2	0	3	0	4	18	3	0	57		
5:19 PM	13	0	9	0	12	70	9	0	11	0	8	0	2	3	5	0	142		
5:20 PM	0	0	8	0	6	13	3	0	0	0	9	0	8	27	5	0	71		
5:21 PM	13	0	5	1	0	47	8	0	19	0	7	0	5	1	9	0	114		
5:22 PM	0	0	7	0	18	1	1	0	0	0	6	0	2	50	4	0	89		
5:23 PM	14	0	10	0	0	50	7	0	20	0	8	0	2	2	8	0	121		
5:24 PM	0	0	8	0	23	4	1	0	0	0	12	0	5	36	4	0	93		
5:25 PM	13	0	6	0	3	62	10	0	21	0	8	0	0	3	7	1	133		
5:26 PM	0	0	2	0	12	4	2	0	0	0	9	0	4	26	8	0	67		
5:27 PM	11	0	8	0	0	34	8	0	23	0	12	0	1	4	13	0	114		
5:28 PM	0	0	7	0	19	1	3	0	1	0	15	0	4	22	3	0	75		
5:29 PM	4	0	5	0	1	55	3	0	18	0	7	0	0	4	11	0	108	5862	
5:30 PM	0	0	7	0	14	33	3	0	0	0	8	0	0	28	3	0	96	5858	
5:31 PM	16	0	6	0	0	41	10	0	10	0	13	0	5	0	6	1	107	5854	
5:32 PM	0	0	2	0	18	9	0	0	0	0	4	0	0	34	6	0	73	5847	
5:33 PM	13	0	7	0	0	62	7	0	18	0	8	0	1	2	2	0	120	5861	
5:34 PM	0	0	3	0	16	0	1	0	1	0	8	0	6	46	7	0	88	5868	
5:35 PM	13	0	4	0	0	60	7	0	21	0	7	0	0	2	5	0	119	5879	
5:36 PM	0	0	1	0	21	1	2	0	0	0	10	0	6	26	8	0	75	5859	
5:37 PM	0	0	7	0	0	64	7	0	12	0	7	0	7	2	5	0	126	5875	
5:38 PM	0	0	2	0	15	7	0	0	0	0	9	0	0	21	6	0	60	5841	
5:39 PM	11	0	5	0	0	56	10	0	22	0	12	0	4	0	6	0	126	5871	
5:40 PM	0	0	1	0	14	6	3	0	0	0	17	0	1	37	7	0	86	5853	
5:41 PM	11	0	7	0	0	46	4	0	16	0	15	0	8	3	8	0	118	5872	
5:42 PM	0	0	3	0	10	3	2	0	0	0	7	0	10	29	1	0	65	5826	
5:43 PM	12	0	7	0	0	53	7	0	18	0	11	0	7	2	8	0	125	5844	
5:44 PM	0	0	7	0	13	2	3	0	0	0	9	0	1	35	11	0	81	5838	
5:45 PM	17	0	5	1	0	67	9	0	22	0	11	0	0	1	14	0	146	5880	
5:46 PM	0	0	4	0	22	0	2	0	1	0	15	0	4	40	4	0	92	5877	
5:47 PM	11	0	8	0	0	40	13	0	17	0	7	0	3	2	4	0	105	5853	
5:48 PM	0	0	1	0	19	3	3	0	0	0	11	0	2	33	7	0	79	5856	
5:49 PM	17	0	5	0	0	70	6	0	25	0	9	0	0	3	5	0	140	5884	
5:50 PM	0	0	3	0	13	1	0	0	0	0	16	0	9	21	11	0	74	5877	
5:51 PM	9	0	5	0	0	49	11	0	16	0	14	0	4	4	12	0	124	5886	
5:52 PM	0	0	5	0	20	1	2	0	0	0	8	0	2	28	4	0	70	5899	
5:53 PM	11	0	4	0	0	45	3	0	31	0	10	0	0	2	5	0	111	5921	
5:54 PM	0	0	9	0	17	2	0	0	3	0	12	0	6	25	8	0	82	5918	
5:55 PM	15	0	9	0	4	51	7	0	15	0	13	0	0	2	4	0	120	5941	
5:56 PM	0	0	7	0	6	13	4	0	0	0	10	0	0	25	4	0	69	5932	
5:57 PM	13	0	7	0	0	31	16	0	14	0	13	0	8	3	6	0	111	5922	
5:58 PM	0	0	5	0	11	2	0	0	0	0	9	0	2	28	9	0	66	5928	
5:59 PM	15	0	3	0	0	33	9	0	14	0	5	0	0	1	6	0	86	5905	
Approach Total	1069				3686				1695				2352						
Grand Total	571	0	498	8	753	2557	376	0	854	0	841	0	268	1507	577	2	8802		
Approach %	53.4%	0.0%	46.6%	0.7%	20.4%	69.4%	10.2%	0.0%	50.4%	0.0%	4								

Start Date: 1/21/2004  
Start Time: 07:00 AM  
Site Code: 4001

Street Name	From North SR-51 (SB) OFF RAMP			From East CACTUS RD			From South SR-51 (NB) OFF RAMP			From West CACTUS RD			INTEC	TOTAL	TOTAL	HOUR	
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right					
7:00 AM	0	0	1	0	0	14	2	0	5	0	0	0	0	25	6	0	53
7:01 AM	12	0	3	0	15	2	0	0	7	0	5	0	11	2	2	0	59
7:02 AM	0	0	7	0	5	23	4	0	5	0	0	0	0	22	9	0	75
7:03 AM	3	0	4	0	13	1	2	0	6	0	4	0	9	4	2	0	48
7:04 AM	0	0	3	0	0	11	3	0	1	0	0	0	0	23	9	0	50
7:05 AM	5	0	3	0	15	2	4	0	3	0	3	0	7	3	7	0	52
7:06 AM	0	0	3	0	0	15	4	0	4	0	0	0	0	29	8	0	63
7:07 AM	10	0	6	0	18	3	3	0	4	0	7	0	7	0	4	0	62
7:08 AM	0	0	8	0	0	12	4	0	3	0	0	0	0	29	6	0	62
7:09 AM	7	0	7	0	16	1	2	0	8	0	5	0	6	8	8	0	66
7:10 AM	0	0	1	0	2	13	5	0	3	0	0	0	0	26	3	0	53
7:11 AM	6	0	1	0	16	2	1	0	4	0	8	0	13	6	5	0	62
7:12 AM	0	0	2	0	3	17	1	0	2	0	0	0	0	30	6	0	61
7:13 AM	8	0	2	0	17	1	1	0	3	0	6	0	7	9	8	0	62
7:14 AM	0	0	6	0	1	16	2	0	2	0	0	0	0	26	5	0	60
7:15 AM	8	0	6	0	16	3	3	0	7	0	5	0	9	4	9	0	70
7:16 AM	0	0	2	0	0	19	1	0	2	0	0	0	0	29	10	0	63
7:17 AM	7	0	8	0	6	1	4	0	9	0	7	0	17	1	6	0	66
7:18 AM	0	0	6	0	0	17	6	0	8	0	0	0	0	34	4	0	75
7:19 AM	5	0	8	0	23	0	1	0	5	0	4	0	10	3	8	0	67
7:20 AM	0	0	6	0	1	29	4	0	5	0	0	0	0	10	3	0	58
7:21 AM	5	0	9	0	13	1	1	0	5	0	4	0	11	2	10	0	61
7:22 AM	0	0	5	0	0	28	2	0	5	0	0	0	0	27	3	0	70
7:23 AM	9	0	14	0	10	0	5	0	8	0	7	0	6	8	14	0	81
7:24 AM	0	0	6	0	0	20	5	0	2	0	0	0	0	26	7	0	66
7:25 AM	13	0	4	0	19	0	1	0	6	0	5	0	9	4	9	0	70
7:26 AM	0	0	7	0	5	21	2	0	7	0	0	0	0	41	11	0	94
7:27 AM	7	0	5	0	8	2	5	0	7	0	5	0	8	9	8	0	64
7:28 AM	0	0	8	0	0	21	4	0	0	0	0	0	0	25	3	0	67
7:29 AM	9	0	11	0	16	1	4	0	8	0	0	0	8	1	5	0	68
7:30 AM	11	0	7	0	6	20	0	0	9	0	0	0	0	33	3	0	78
7:31 AM	0	0	7	0	0	12	3	0	8	0	3	0	6	7	4	0	60
7:32 AM	0	0	7	0	16	27	0	0	2	0	0	0	0	34	6	0	92
7:33 AM	18	0	9	0	11	2	2	0	4	0	4	0	7	5	2	0	84
7:34 AM	0	0	9	0	2	19	5	0	3	0	0	0	0	40	2	0	60
7:35 AM	9	0	7	0	9	3	1	0	11	0	5	0	11	1	3	0	71
7:36 AM	0	0	8	0	0	22	0	0	4	0	0	0	0	29	8	0	60
7:37 AM	18	0	6	0	12	2	2	0	5	0	7	0	14	7	4	0	77
7:38 AM	0	0	5	0	4	21	2	0	2	0	0	0	1	29	4	0	68
7:39 AM	13	0	19	0	8	1	6	0	5	0	9	0	12	8	4	0	85
7:40 AM	0	0	12	0	0	21	4	0	1	0	0	0	0	42	2	0	82
7:41 AM	14	0	8	0	6	0	1	0	8	0	6	0	7	4	9	0	63
7:42 AM	0	0	10	0	10	16	5	0	3	0	0	0	0	34	6	0	84
7:43 AM	8	0	9	0	8	0	5	0	5	0	21	0	10	9	4	0	80
7:44 AM	0	0	16	0	2	19	5	0	5	0	0	0	0	33	9	0	89
7:45 AM	12	0	13	0	15	0	2	0	15	0	0	0	0	6	5	0	86
7:46 AM	0	0	7	0	1	21	3	0	4	0	0	0	0	32	4	0	72
7:47 AM	20	0	13	0	10	1	2	0	8	0	6	0	12	7	5	0	84
7:48 AM	0	0	5	0	9	23	6	0	8	0	0	0	0	42	4	0	97
7:49 AM	20	0	6	0	0	1	5	0	5	0	8	0	15	5	6	0	71
7:50 AM	0	0	3	0	8	31	3	0	13	0	0	0	0	30	5	0	93
7:51 AM	16	0	10	0	8	0	4	0	6	0	5	0	12	4	5	0	72
7:52 AM	0	0	5	0	0	26	3	0	6	0	0	0	0	38	8	0	86
7:53 AM	20	0	10	0	0	3	4	0	9	0	7	0	9	7	5	0	74
7:54 AM	0	0	12	0	14	17	4	0	4	0	0	0	0	38	6	0	95
7:55 AM	25	0	16	0	4	2	1	0	6	0	10	0	10	5	4	0	83
7:56 AM	0	0	9	0	2	17	2	0	11	0	1	0	0	42	3	0	87
7:57 AM	23	0	14	0	7	1	4	0	6	0	11	0	10	1	3	0	80
7:58 AM	0	0	3	0	2	30	2	0	5	0	0	0	0	43	3	0	88
7:59 AM	24	0	12	0	1	0	4	0	4	0	5	0	5	4	7	0	86
8:00 AM	0	0	9	0	11	18	1	0	2	0	0	0	1	44	2	0	86
8:01 AM	14	0	0	0	8	1	3	0	7	0	4	0	7	8	3	0	55
8:02 AM	0	0	4	0	3	23	8	0	3	0	0	0	0	30	6	0	77
8:03 AM	10	0	7	0	6	1	2	0	7	0	6	0	18	4	3	0	65
8:04 AM	0	0	8	0	0	22	5	0	3	0	0	0	0	36	5	0	79
8:05 AM	20	0	6	0	7	0	1	0	4	0	4	0	10	3	6	0	81
8:06 AM	0	0	5	0	2	22	2	0	9	0	0	0	0	34	9	0	63
8:07 AM	7	0	6	0	13	1	3	0	5	0	6	0	12	6	3	0	62
8:08 AM	0	0	6	0	0	28	5	0	5	0	0	0	0	26	5	0	75
8:09 AM	16	0	5	0	0	0	0	0	6	0	10	0	11	3	4	0	55
8:10 AM	0	0	8	0	15	31	2	0	4	0	0	0	1	37	5	0	103
8:11 AM	8	0	1	0	13	5	4	0	8	0	5	0	9	8	5	0	66
8:12 AM	0	0	4	0	0	7	4	0	2	0	0	0	0	49	11	0	77
8:13 AM	13	0	3	0	5	0	4	0	5	0	8	0	9	3	5	0	55
8:14 AM	0	0	3	0	0	16	5	0	8	0	0	0	0	25	7	0	64
8:15 AM	13	0	9	0	3	17	1	0	3	0	9	0	7	3	6	0	71
8:16 AM	0	0	7	0	8	15	0	0	3	0	0	0	0	30	2	0	65
8:17 AM	15	0	8	0	0	3	2	0	7	0	10	0	7	2	3	0	57
8:18 AM	0	0	14	0	16	10	3	0	7	0	0	0	1	30	9	0	90
8:19 AM	20	0	11	0	1	0	11	0	7	0	0	0	11	7	1	0	74
8:20 AM	0	0	9	0	8	14	6	0	9	0	0	0	0	29	5	0	80
8:21 AM	9	0	9	0	13	2	0	0	6	0	10	0	5	2	5	0	61
8:22 AM	0	0	3	0	2	25	5	0	5	0	0	0	0	24	9	0	73
8:23 AM	14	0	8	0	13	0	0	0	10	0	4	0	14	4	6	0	73
8:24 AM	0	0	10	0	4	18	2	0	6	0	0	0	0	29	3	0	72
8:25 AM	19	0	14	0	14	2	1	0	7	0	8	0	8	2	4	0	79
8:26 AM	0	0	7	0	3	9	5	0	5	0	0	0	0	26	6	0	61
8:27 AM	27	0	6	0	2	0	1	0	3	0	9	0	10	4	6	0	68
8:28 AM	0	0	9	0	9	25	4	0	8	0	0	0	0	29	10	0	94
8:29 AM	14	0	8	0	10	1	1	0	11	0	7	0	8	2	6	0	68
Approach Total	1235				1839				810				2565				
Grand Total	586	0	640	0	602	970	267	0	509	0	301	0	440	1624	501	0	6449
Approach %	47.4%	0.0%	52.6%	0.0%	32.7%	52.7%	14.5%	0.0%	62.8%	0.0%	37.2%	0.0%	17.2%	63.3%	19.5%	0.0%	
Total %	9.1%	0.0%	10.1%	0.0%	9.3%	15.0%	4.1%	0.0%	7.9%	0.0%	4.7%	0.0%	6.8%	25.2%	7.8%	0.0%	
Begin Peak Hour:	7:26																
Peak Hour Vol:	447	0	486	0	369	684	185	0	360	0	215	0	300	1173	302	0	4521
																	4521 7:26 AM to 8:26 AM

Start Date: 12/1/2004  
Start Time: 04:30 PM  
Site Code: 4001

Street Name	From North SR-51 (SB) OFF RAMP			From East CACTUS RD			From South SR-51 (NB) OFF RAMP			From West CACTUS RD			INTSEC	TOTAL	TOTAL			
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right						
4:30 PM	9	0	4	0	18	36	6	0	10	0	7	0	0	18	6	0	114	
4:31 PM	1	0	3	0	6	20	12	0	0	0	10	0	0	6	36	6	0	100
4:32 PM	9	0	6	0	0	9	6	0	12	0	11	0	0	0	0	4	0	57
4:33 PM	0	0	7	0	2	21	2	0	5	0	9	0	7	42	10	0	105	
4:34 PM	10	0	3	0	0	28	8	0	9	0	13	0	0	0	5	0	76	
4:35 PM	0	0	4	0	7	16	4	0	0	0	8	0	11	28	3	0	81	
4:36 PM	9	0	9	0	0	19	5	0	15	0	1	0	0	0	8	0	66	
4:37 PM	4	0	2	0	16	24	3	0	6	0	7	0	12	20	3	0	97	
4:38 PM	11	0	5	0	0	14	6	0	7	0	8	0	0	0	1	0	52	
4:39 PM	2	0	7	0	8	26	1	0	1	0	10	0	14	30	1	0	100	
4:40 PM	6	0	5	0	0	25	3	0	11	0	9	0	0	0	4	0	63	
4:41 PM	0	0	3	0	8	21	3	0	0	0	5	0	13	40	3	0	96	
4:42 PM	12	0	4	0	0	16	6	0	12	0	7	0	0	0	3	0	60	
4:43 PM	1	0	2	0	6	26	7	0	2	0	6	0	9	28	3	0	90	
4:44 PM	14	0	7	0	0	6	2	0	9	0	9	0	0	1	7	0	55	
4:45 PM	0	0	4	0	8	19	4	0	0	0	6	0	8	52	3	0	104	
4:46 PM	11	0	6	0	0	10	13	0	12	0	14	0	0	4	2	0	72	
4:47 PM	0	0	4	0	18	27	3	0	3	0	9	0	20	29	3	0	116	
4:48 PM	9	0	6	0	0	22	7	0	14	0	8	0	0	1	4	0	71	
4:49 PM	10	0	7	0	15	20	2	0	1	0	15	0	12	31	6	0	119	
4:50 PM	13	0	3	0	0	11	2	0	14	0	8	0	0	1	2	0	54	
4:51 PM	7	0	1	0	12	25	3	0	1	0	4	0	9	35	4	0	101	
4:52 PM	12	0	6	0	0	22	4	0	18	0	5	0	0	1	3	0	71	
4:53 PM	2	0	8	0	8	16	5	0	5	0	13	0	10	46	2	0	115	
4:54 PM	12	0	5	0	0	23	8	0	11	0	7	0	0	2	2	0	70	
4:55 PM	1	0	4	0	11	21	7	0	1	0	5	0	16	30	3	0	99	
4:56 PM	0	0	5	0	0	7	4	0	14	0	9	0	0	0	3	0	42	
4:57 PM	23	0	2	0	9	0	2	0	3	0	7	0	7	0	1	0	54	
4:58 PM	12	0	5	0	2	38	8	0	7	0	6	0	1	34	8	0	121	
4:59 PM	0	0	4	0	11	32	10	0	0	0	12	0	6	36	4	0	115	
5:00 PM	7	0	10	0	0	8	3	0	15	0	14	0	0	2	3	0	62	
5:01 PM	4	0	3	0	23	24	5	0	8	0	11	0	13	29	5	0	125	
5:02 PM	14	0	7	0	0	12	6	0	15	0	6	0	0	0	5	0	65	
5:03 PM	3	0	3	0	17	22	1	0	2	0	7	0	10	30	4	0	99	
5:04 PM	9	0	7	0	0	19	7	0	17	0	9	0	0	0	9	0	77	
5:05 PM	3	0	5	0	12	9	0	0	2	0	6	0	7	14	6	0	104	
5:06 PM	8	0	5	0	0	29	14	0	13	0	6	0	0	2	3	0	80	
5:07 PM	0	0	11	0	14	31	8	0	0	0	7	0	9	27	4	0	111	
5:08 PM	12	0	4	0	0	8	3	0	16	0	13	0	0	1	3	0	60	
5:09 PM	3	0	8	0	15	16	6	0	1	0	15	0	10	31	4	0	109	
5:10 PM	10	0	2	0	0	26	13	0	13	0	9	0	0	0	2	0	75	
5:11 PM	0	0	7	0	14	32	5	0	2	0	7	0	5	39	4	0	115	
5:12 PM	12	0	6	0	0	20	11	0	13	0	8	0	0	5	12	0	87	
5:13 PM	2	0	6	0	26	24	9	0	0	0	11	0	15	29	6	0	126	
5:14 PM	9	0	8	0	0	27	9	0	9	0	9	0	0	8	6	0	85	
5:15 PM	0	0	13	0	11	25	4	0	0	0	8	0	6	48	2	0	117	
5:16 PM	15	0	7	0	0	21	7	0	16	0	15	0	0	0	3	0	84	
5:17 PM	1	0	9	0	8	19	6	0	1	0	10	0	9	46	8	0	117	
5:18 PM	11	0	8	0	0	21	11	0	9	0	11	0	0	2	4	0	77	
5:19 PM	0	0	13	0	19	25	5	0	0	0	7	0	11	22	0	0	102	
5:20 PM	15	0	7	0	0	24	4	0	11	0	6	0	0	1	8	0	76	
5:21 PM	5	0	9	0	15	24	7	0	0	0	13	0	10	33	5	0	121	
5:22 PM	7	0	6	0	0	29	8	0	9	0	11	0	4	2	4	0	89	
5:23 PM	0	0	10	0	16	32	5	0	0	0	9	0	3	43	7	0	125	
5:24 PM	13	0	4	0	0	17	8	0	15	0	7	0	0	0	5	0	89	
5:25 PM	0	0	5	0	19	27	12	0	0	0	7	0	13	31	6	0	120	
5:26 PM	5	0	6	0	0	6	3	0	16	0	8	0	0	0	5	0	49	
5:27 PM	2	0	7	0	14	23	5	0	1	0	7	0	3	46	5	0	113	
5:28 PM	9	0	3	0	0	20	6	0	11	0	6	0	3	4	6	0	68	
5:29 PM	0	0	3	0	8	10	1	0	0	0	6	0	9	35	8	0	80	
5:30 PM	13	0	4	0	0	28	15	0	11	0	4	0	0	10	6	0	91	
5:31 PM	0	0	7	0	16	38	4	0	0	0	14	0	9	30	3	0	121	
5:32 PM	11	0	7	0	0	17	8	0	14	0	12	0	0	1	5	0	75	
5:33 PM	0	0	8	0	5	22	6	0	1	0	10	0	7	29	7	0	95	
5:34 PM	13	0	8	0	0	18	7	0	14	0	9	0	0	0	3	0	72	
5:35 PM	2	0	9	0	18	27	6	0	0	0	11	0	9	34	6	0	122	
5:36 PM	6	0	4	0	0	24	8	0	16	0	11	0	2	1	9	0	81	
5:37 PM	0	0	3	0	14	38	6	0	0	0	11	0	11	28	8	0	119	
5:38 PM	14	0	7	0	0	15	1	0	13	0	6	0	0	0	8	0	64	
5:39 PM	5	0	3	0	20	23	1	0	0	0	8	0	9	30	4	0	103	
5:40 PM	16	0	2	0	0	24	7	0	11	0	9	0	0	3	2	0	74	
5:41 PM	4	0	6	0	15	19	5	0	0	0	7	0	14	48	4	0	120	
5:42 PM	12	0	4	0	0	16	11	0	15	0	9	0	0	8	4	0	79	
5:43 PM	1	0	6	0	16	31	9	0	0	0	8	0	9	25	4	0	109	
5:44 PM	14	0	8	0	0	11	7	0	15	0	8	0	0	1	3	0	67	
5:45 PM	0	0	7	0	14	21	7	0	0	0	3	0	11	31	6	0	100	
5:46 PM	14	0	4	0	0	20	6	0	14	0	13	0	1	0	3	0	75	
5:47 PM	0	0	5	0	19	25	5	0	0	0	8	0	12	44	7	0	125	
5:48 PM	7	0	4	0	0	6	3	0	12	0	10	0	6	1	9	0	58	
5:49 PM	0	0	5	0	19	23	3	0	0	0	7	0	5	25	1	0	88	
5:50 PM	19	0	6	0	0	4	0	0	11	0	12	0	0	2	3	0	57	
5:51 PM	2	0	5	0	6	21	8	0	1	0	11	0	8	35	5	0	102	
5:52 PM	11	0	6	0	0	13	8	0	12	0	8	0	5	2	4	0	69	
5:53 PM	1	0	2	0	10	22	8	0	0	0	6	0	7	46	4	0	106	
5:54 PM	7	0	4	0	0	14	11	0	11	0	14	0	5	1	7	0	74	
5:55 PM	0	0	3	0	18	26	9	0	0	0	9	0	5	46	5	0	121	
5:56 PM	16	0	11	0	0	7	2	0	8	0	12	0	0	1	5	0	62	
5:57 PM	1	0	3	0	15	26	5	0	0	0	8	0	10	30	4	0	102	
5:58 PM	16	0	5	0	0	16	12	0	17	0	4	0	4	1	6	0	81	
5:59 PM	0	0	3	0	6	25	10	0	0	0	16	0	6	31	9	0	106	
Approach Total	1086				3003				1419				2526					
Grand Total	584	0	502	0	607	1850	546	0	614	0	805	0	456	1647	423	0	8034	
Approach %	53.8%	0.0%	46.2%	0.0%	20.2%	61.6%	18.2%	0.0%	43.3%	0.0%	56.7%	0.0%	18.1%	65.2%	16.7%	0.0%		
Total %	7.3%	0.0%	6.2%	0.0%	7.6%	23.0%	6.8%	0.0%	7.8%	0.0%	10.0%	0.0%	5.7%	20.5%	5.3%	0.0%		
Begin Peak Hour:	16:58																	
Peak Hour Vol:	380	0	362	0	449	1279	386	0	401	0	555	0	292	1140	303	0	5547	

Start Date: 1/28/2004  
Start Time: 07:00 AM  
Site Code: 4001

Street Name	From North					From East					From South					From West					INTEC	TOTAL	TOTAL	HOUR
	W. LOOP 101 (SB) OFF RAMP	Left	Thru	Right	Trucks	BELL RD - From East	Left	Thru	Right	Trucks	W. LOOP 101 (NB) OFF RAMP - From South	Left	Thru	Right	Trucks	BELL RD - From West	Left	Thru	Right	Trucks				
7:00 AM	4	0	10	8	0	14	5	1	0	0	0	9	5	0	21	11	15	0	90					
7:01 AM	0	0	17	8	0	5	12	0	0	18	0	6	5	0	1	23	7	3	89					
7:02 AM	7	0	11	7	1	5	10	1	1	0	0	4	4	1	28	11	9	1	88					
7:03 AM	0	0	17	7	0	1	11	0	0	15	0	6	4	0	2	12	7	1	71					
7:04 AM	4	0	12	10	0	4	9	0	1	0	0	5	4	0	17	21	9	3	81					
7:05 AM	0	0	16	5	0	5	9	2	1	16	0	9	4	0	12	18	7	0	94					
7:06 AM	6	0	14	10	0	11	2	0	0	0	0	3	1	1	27	34	8	1	105					
7:07 AM	0	0	12	5	0	4	11	2	0	12	0	10	3	2	15	21	13	1	100					
7:08 AM	1	0	8	7	0	6	19	1	2	0	0	6	6	0	22	9	13	1	85					
7:09 AM	0	0	18	10	1	0	17	3	0	0	0	3	2	6	6	20	10	0	77					
7:10 AM	4	0	10	5	0	12	2	2	0	0	0	11	9	0	27	17	14	0	89					
7:11 AM	0	0	13	9	1	11	5	2	0	16	0	6	3	1	3	19	7	2	89					
7:12 AM	3	0	13	3	2	3	11	3	0	13	0	11	9	4	9	16	5	1	94					
7:13 AM	0	0	13	7	2	14	9	1	0	0	0	8	6	0	27	16	8	0	87					
7:14 AM	2	0	12	7	2	0	20	1	0	6	0	17	2	4	8	26	13	0	104					
7:15 AM	4	0	8	8	2	13	3	2	0	0	0	8	7	0	27	6	9	0	80					
7:16 AM	0	0	18	9	0	2	18	2	1	19	0	8	8	2	9	18	13	1	107					
7:17 AM	3	0	14	10	0	11	4	0	0	0	0	5	4	0	20	21	18	0	96					
7:18 AM	0	0	21	9	0	0	19	2	1	18	0	4	3	1	1	23	7	1	95					
7:19 AM	3	0	6	4	3	12	3	3	0	0	0	6	5	1	28	14	14	0	89					
7:20 AM	0	0	16	8	1	3	19	1	1	12	0	11	5	0	9	26	4	2	101					
7:21 AM	2	0	9	6	1	11	4	0	0	1	0	10	6	0	27	11	12	0	86					
7:22 AM	0	0	22	9	0	1	16	1	0	25	0	9	2	9	5	15	1	1	95					
7:23 AM	3	0	15	12	1	6	3	0	0	0	0	5	5	0	25	28	10	0	95					
7:24 AM	0	0	20	5	0	0	16	1	0	19	0	9	9	1	5	23	9	2	102					
7:25 AM	8	0	11	7	0	9	18	0	0	0	0	7	3	0	27	20	4	0	104					
7:26 AM	0	0	14	4	2	0	18	1	0	16	0	11	3	2	11	21	6	2	98					
7:27 AM	2	0	6	5	0	11	3	2	1	0	0	8	6	0	25	24	12	0	93					
7:28 AM	0	0	28	13	4	1	11	5	2	20	0	12	2	9	21	7	3	114						
7:29 AM	6	0	5	3	1	12	2	1	2	0	0	2	2	1	24	9	10	1	71					
7:30 AM	0	0	16	6	3	4	19	1	0	24	0	11	8	3	5	21	13	0	114					
7:31 AM	0	0	4	4	0	11	14	1	0	0	0	6	3	0	32	20	13	0	101					
7:32 AM	0	0	26	14	0	0	24	2	0	14	0	12	5	2	7	19	4	0	108					
7:33 AM	1	0	6	6	0	14	1	0	1	0	0	5	5	1	30	11	15	0	83					
7:34 AM	0	0	22	9	0	5	15	0	1	23	0	12	12	1	28	17	9	0	105					
7:35 AM	8	0	7	8	1	13	3	1	0	0	0	7	5	0	7	21	6	1	98					
7:36 AM	0	0	25	6	1	0	13	3	1	23	0	8	8	1	9	22	13	3	116					
7:37 AM	0	0	12	12	0	9	21	1	0	0	0	5	5	0	28	7	9	0	96					
7:38 AM	0	0	26	9	2	0	20	0	1	10	0	6	5	5	10	25	10	0	107					
7:39 AM	0	0	10	7	1	17	5	1	0	0	0	8	7	0	26	8	8	0	83					
7:40 AM	0	0	30	10	3	2	12	2	1	26	0	14	8	1	1	23	5	2	115					
7:41 AM	4	0	18	11	2	12	5	1	1	0	0	7	7	0	31	11	9	0	98					
7:42 AM	0	0	22	0	1	0	22	4	0	18	0	7	4	2	0	25	7	1	103					
7:43 AM	8	0	17	14	1	6	9	2	1	0	0	8	8	0	27	22	14	2	113					
7:44 AM	0	0	27	10	5	0	21	1	0	17	0	8	2	2	2	24	9	0	109					
7:45 AM	4	0	9	6	0	14	5	1	0	0	0	6	5	0	25	16	10	2	90					
7:46 AM	0	0	26	2	0	4	18	2	0	16	0	11	9	2	7	27	8	0	119					
7:47 AM	4	0	13	10	0	13	5	1	0	0	0	16	9	1	4	25	5	1	97					
7:48 AM	0	0	25	6	2	0	16	1	0	24	0	16	9	1	4	25	5	0	116					
7:49 AM	5	0	17	9	1	8	13	4	1	0	0	12	9	0	28	16	7	1	110					
7:50 AM	0	0	37	9	2	0	19	1	1	26	0	18	10	1	3	25	3	0	132					
7:51 AM	4	0	18	11	1	11	10	0	0	0	0	9	7	1	27	22	11	0	112					
7:52 AM	0	0	20	9	1	0	15	2	0	26	0	16	10	0	3	17	11	0	110					
7:53 AM	5	0	13	10	0	12	3	1	0	0	0	11	6	0	29	13	10	3	97					
7:54 AM	0	0	21	12	1	1	22	0	1	32	0	10	8	1	0	21	12	0	119					
7:55 AM	11	0	10	8	0	6	2	1	1	0	0	8	4	1	30	7	5	4	80					
7:56 AM	0	0	21	10	0	0	18	1	0	24	0	15	9	0	4	25	7	2	115					
7:57 AM	8	0	7	7	0	10	2	0	1	0	0	8	5	0	35	15	9	2	94					
7:58 AM	0	0	35	10	1	1	9	0	0	22	0	5	4	1	4	17	6	1	101	5913	7:00 AM to 8:00 AM			
7:59 AM	6	0	11	10	1	5	4	2	2	0	0	7	1	1	31	15	7	0	88	5911	7:01 AM to 8:01 AM			
8:00 AM	0	0	27	4	2	0	22	0	0	26	0	7	4	4	0	15	6	2	103	5925	7:02 AM to 8:02 AM			
8:01 AM	4	0	8	7	0	10	14	2	0	10	0	4	4	0	23	12	9	0	96	5935	7:03 AM to 8:03 AM			
8:02 AM	0	0	13	4	0	0	11	0	0	15	0	3	5	1	0	25	7	4	79	5943	7:04 AM to 8:04 AM			
8:03 AM	0	0	7	4	0	7	0	1	0	0	0	3	2	1	34	13	9	0	81	5943	7:05 AM to 8:05 AM			
8:04 AM	0	0	7	4	1	0	14	1	0	13	0	8	6	0	5	27	8	2	83	5932	7:06 AM to 8:06 AM			
8:05 AM	2	0	9	8	2	12	4	0	1	0	0	8	5	0	37	11	9	0	92	5919	7:07 AM to 8:07 AM			
8:06 AM	0	0	22	9	1	0	21	3	0	14	0	13	8	1	5	26	12	0	116	5935	7:08 AM to 8:08 AM			
8:07 AM	5	0	9	9	0	10	0	0	0	0	0	10	8	1	33	8	11	0	86	5936	7:09 AM to 8:09 AM			
8:08 AM	0	0	24	15	1	0	9	2	0	14	0	2	1	0	1	16	5	1	73	5932	7:10 AM to 8:10 AM			
8:09 AM	4	0	12	10	0	11	0	0	1	0	0	3	1	0	26	10	7	1	73	5906	7:11 AM to 8:11 AM			
8:10 AM	0	0	12	9	0	0	7	0	0	9	0	9	6	1	0	15	3	0	48	5965	7:12 AM to 8:12 AM			
8:11 AM	2	0	14	12	0	9	2	1	1	0	0	9	6	1	30	9	5	0	81	5952	7:13 AM to 8:13 AM			
8:12 AM	0	0	18	12	1	0	16	2	2	12	0	13	5	2	5	8	4	2	78	5946	7:14 AM to 8:14 AM			
8:13 AM	4	0	7	7	0	4	0	0	0	0	0	5	1	0	34	12	5	0	71	5920	7:15 AM to 8:15 AM			
8:14 AM	7	0	9	9	1	0	24	2	1	20	0	9	9	2	3	24	6	3	111	5827	7:16 AM to 8:16 AM			
8:15 AM	0	0	23	8	1	0	5	0	0	0	0	5	4	0	28	9	8	0	78	5825	7:17 AM to 8:17 AM			
8:16 AM	0	0	19	5	1	0	15	0	1	18	0	9	8	1	0	13	2	0	76	5794	7:18 AM to 8:18 AM			
8:17 AM	4	0	6	6	0	9	5	3	1	0	0	6	4	0	31	9	14	1	87	5785	7:19 AM to 8:19 AM			
8:18 AM	0	0	20	9	0	0	19	1	0	16	0	6	5	0	2	21	2	0	87	5777	7:20 AM to 8:20 AM			
8:19 AM	6	0	6	6	1	6	1	1	2	0	0	11	9	0	13	9	9	0	62	5750	7:21 AM to 8:21 AM			
8:20 AM	0	0	23	11																				

Start Date: 1/28/2004  
Start Time: 04:30 PM  
Site Code: 4001

Street Name	From North				From East				From South				From West				INTSEC	TOTAL	TOTAL	
	W. LOOP 101 (SB) OFF RAMP	Left	Thru	Right	BELL RD - From East	Left	Thru	Right	W. LOOP 101 (NB) OFF RAMP - From South	Left	Thru	Right	BELL RD - From West	Left	Thru	Right				Trucks
4:30 PM	4	0	30	4	0	18	1	5	0	16	0	21	15	0	37	0	11	2	143	
4:31 PM	1	0	5	5	0	0	56	6	0	2	0	15	15	0	0	33	4	0	122	
4:32 PM	8	0	22	4	0	15	0	7	1	20	0	12	5	0	26	0	10	0	120	
4:33 PM	1	0	5	5	0	0	62	4	1	1	0	11	11	0	0	71	13	0	168	
4:34 PM	4	0	17	4	0	23	0	6	0	17	0	10	0	0	28	0	12	11	117	
4:35 PM	0	0	7	7	0	0	56	4	0	3	0	13	13	0	0	32	7	0	122	
4:36 PM	8	0	26	4	0	18	0	7	1	19	0	5	2	0	30	0	8	2	121	
4:37 PM	1	0	3	3	0	0	47	3	0	8	0	7	7	0	0	51	9	1	127	
4:38 PM	9	0	30	5	1	19	0	7	0	18	0	18	3	1	29	0	9	0	139	
4:39 PM	3	0	7	7	0	0	59	6	0	23	0	15	10	0	0	31	7	0	151	
4:40 PM	10	0	26	4	0	24	0	5	0	0	0	13	2	0	29	0	10	1	117	
4:41 PM	0	0	12	12	0	2	43	6	0	22	0	12	6	0	0	45	8	0	150	
4:42 PM	8	0	24	1	0	17	0	9	1	0	0	14	3	0	31	0	8	1	111	
4:43 PM	5	0	5	5	0	0	57	4	0	19	0	18	12	0	0	50	13	0	171	
4:44 PM	6	0	25	3	0	22	0	2	0	0	0	11	7	0	28	0	7	0	101	
4:45 PM	0	0	7	7	0	0	52	5	0	15	0	10	6	0	0	43	11	1	143	
4:46 PM	8	0	23	3	1	19	0	5	0	0	0	10	2	0	27	0	14	0	106	
4:47 PM	4	0	9	9	1	2	50	6	0	24	0	20	11	0	0	48	10	0	173	
4:48 PM	9	0	38	7	0	12	0	0	0	0	0	12	3	0	32	0	15	1	123	
4:49 PM	3	0	9	10	0	0	55	4	1	13	0	8	5	0	0	49	9	0	150	
4:50 PM	5	0	30	5	0	15	0	6	1	0	0	17	0	0	32	0	6	0	111	
4:51 PM	2	0	4	4	0	2	47	6	0	22	0	16	11	0	0	53	7	1	159	
4:52 PM	6	0	24	5	0	15	5	5	0	0	0	15	5	0	31	7	6	1	114	
4:53 PM	0	0	3	3	0	0	50	7	0	24	0	12	9	0	0	23	7	1	126	
4:54 PM	8	0	28	3	1	17	0	7	1	0	0	16	1	1	30	0	6	2	112	
4:55 PM	1	0	9	9	0	2	46	4	0	16	0	19	12	0	0	54	10	0	161	
4:56 PM	9	0	27	3	0	17	0	1	0	0	0	16	3	0	32	0	5	0	107	
4:57 PM	2	0	9	9	0	0	38	6	0	22	0	15	8	0	0	40	7	0	139	
4:58 PM	8	0	35	7	0	21	0	11	0	0	0	12	2	0	33	0	13	0	133	
4:59 PM	1	0	8	8	0	0	51	3	1	20	0	17	12	0	0	51	10	1	161	
5:00 PM	13	0	23	4	0	20	0	6	0	0	0	17	10	0	0	27	0	9	1	115
5:01 PM	1	0	4	4	0	3	48	8	2	20	0	17	10	0	0	37	6	1	138	
5:02 PM	12	0	33	8	2	16	0	3	0	0	0	6	2	0	22	0	6	0	100	
5:03 PM	0	0	9	9	0	4	52	3	0	22	0	14	14	0	2	33	11	0	150	
5:04 PM	11	0	32	3	0	18	0	7	0	0	0	9	2	0	28	0	14	0	119	
5:05 PM	0	0	8	8	0	0	47	8	0	21	0	10	8	0	1	43	5	0	143	
5:06 PM	4	0	21	5	0	14	0	4	0	0	0	12	5	0	28	0	13	1	96	
5:07 PM	1	0	5	5	1	2	46	4	0	23	0	15	10	0	0	39	13	0	148	
5:08 PM	5	0	38	9	0	25	0	8	0	0	0	13	4	0	27	0	14	0	130	
5:09 PM	0	0	13	13	0	0	39	1	1	19	0	10	6	0	0	41	7	1	130	
5:10 PM	3	0	31	5	0	23	0	3	0	0	0	13	4	0	28	0	13	0	114	
5:11 PM	0	0	7	7	0	4	49	9	1	13	0	10	9	0	2	46	11	0	151	
5:12 PM	9	0	35	7	0	22	0	7	1	0	0	14	0	0	30	0	8	0	125	
5:13 PM	2	0	5	5	0	4	39	3	0	19	0	13	9	0	0	48	9	0	142	
5:14 PM	9	0	31	4	0	18	0	8	1	0	0	14	2	0	31	0	12	2	123	
5:15 PM	1	0	9	9	0	5	42	4	0	0	0	0	0	0	2	47	10	0	120	
5:16 PM	9	0	17	2	0	12	0	3	0	10	0	17	9	0	26	0	13	2	107	
5:17 PM	0	0	7	7	0	5	56	4	0	0	0	13	2	0	2	49	12	0	148	
5:18 PM	13	0	28	1	0	14	0	9	0	19	0	15	10	0	26	0	8	0	132	
5:19 PM	2	0	8	8	0	0	39	0	0	0	0	8	0	3	2	44	8	0	111	
5:20 PM	9	0	22	8	0	26	0	6	0	26	0	18	12	0	27	0	10	0	144	
5:21 PM	0	0	9	9	0	3	42	3	0	0	0	15	3	0	2	36	17	0	127	
5:22 PM	3	0	32	5	0	16	4	0	0	27	0	15	12	0	28	0	5	0	130	
5:23 PM	0	0	9	9	0	8	55	5	0	0	0	20	4	0	2	45	5	0	149	
5:24 PM	9	0	31	8	0	16	0	14	0	21	0	20	14	0	28	0	5	0	144	
5:25 PM	1	0	6	6	0	8	44	4	0	0	0	15	1	0	4	46	12	0	140	
5:26 PM	10	0	22	0	0	13	0	6	0	23	0	11	9	0	24	0	15	0	124	
5:27 PM	0	0	4	4	0	8	53	2	0	0	0	12	1	0	4	43	10	0	136	
5:28 PM	0	0	27	4	0	15	0	0	0	16	0	11	7	0	24	0	13	1	112	
5:29 PM	0	0	8	8	0	7	45	5	0	0	0	12	0	0	2	54	6	0	139	
5:30 PM	11	0	28	3	0	15	1	3	0	23	0	15	7	0	24	0	3	1	123	
5:31 PM	0	0	5	5	0	6	55	5	0	0	0	11	2	1	5	52	8	0	147	
5:32 PM	6	0	34	9	0	10	3	4	1	15	0	7	7	0	24	0	11	4	114	
5:33 PM	0	0	10	9	0	6	34	5	0	0	0	16	0	0	2	41	9	0	123	
5:34 PM	12	0	31	4	0	11	7	4	0	15	0	13	7	0	21	2	6	1	122	
5:35 PM	0	0	7	7	0	6	48	8	0	0	0	15	3	1	8	28	10	0	130	
5:36 PM	16	0	24	1	0	11	3	7	0	22	0	7	5	0	22	0	12	1	124	
5:37 PM	2	0	6	6	0	9	45	3	0	0	0	16	5	0	7	42	1	0	131	
5:38 PM	9	0	36	6	0	7	4	0	0	18	0	9	9	0	16	0	9	2	108	
5:39 PM	1	0	3	3	0	8	55	5	0	0	0	12	1	0	6	46	9	0	145	
5:40 PM	7	0	27	5	0	6	6	6	0	18	0	23	17	0	22	2	7	0	124	
5:41 PM	2	0	9	9	0	8	48	5	0	0	0	13	4	0	7	50	10	0	152	
5:42 PM	11	0	32	4	1	8	5	5	0	14	0	13	8	0	25	1	10	1	122	
5:43 PM	0	0	6	6	0	10	57	5	1	0	0	18	4	0	7	50	8	0	161	
5:44 PM	6	0	31	6	0	8	5	3	0	20	0	13	11	0	25	0	11	0	122	
5:45 PM	0	0	7	7	0	8	42	4	0	0	0	16	2	0	8	37	8	0	130	
5:46 PM	6	0	24	3	0	10	6	8	0	25	0	8	7	0	19	8	10	0	124	
5:47 PM	0	0	12	12	0	6	32	9	0	0	0	13	4	0	5	37	9	0	123	
5:48 PM	6	0	12	3	0	14	7	3	0	15	0	15	9	0	24	1	10	0	107	
5:49 PM	0	0	9	9	0	8	47	3	0	0	0	11	1	0	5	51	5	0	139	
5:50 PM	11	0	27	4	1	6	7	3	0	25	0	5	5	0	21	0	6	0	111	
5:51 PM	0	0	3	3	0	10	42	3	0	0	0	12	1	0	7	60	11	0	148	
5:52 PM	9	0	31	2	0	16	5	4	0	25	0	15	11	0	21	3	7	0	134	
5:53 PM	0	0	4	4	0	8	42	1	0	0	0	13	5	0	10	17	6	0	101	
5:54 PM	10	0	27	6	0	12	6	7	0	22	0	10	10	0	17	3	7	0	121	
5:55 PM	0	0	6	6	0	10	35	9	0	0	0	14	2	2	7	54	9	0	144	
5:56 PM	11	0	25	5	0	11	6	3	0	25	0	19	17	0	21	3	4	0	128	
5:57 PM	1	0	10	10	0	11	26	3												

Start Date: 1/29/2004  
Start Time: 07:00 AM  
Site Code: 4001

Sheet Name	From North SCOTTSDALE RD				From East LOOP 202 (WB) OFF RAMP				From South SCOTTSDALE RD				From West LOOP 202 (EB) OFF RAMP				INTSEC	TOTAL	TOTAL
	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks			
7:00 AM	3	1	6	1	9	0	2	0	14	22	0	0	6	0	4	1	57		
7:01 AM	0	9	5	1	1	0	4	0	15	5	0	0	0	0	8	0	47		
7:02 AM	5	9	4	0	10	0	3	0	6	17	0	0	16	0	4	1	74		
7:03 AM	0	2	4	0	0	0	0	0	10	8	1	0	9	0	9	0	43		
7:04 AM	3	14	2	1	8	0	4	0	2	11	0	0	3	0	2	0	49		
7:05 AM	6	4	6	0	0	0	2	0	14	21	2	0	10	0	8	1	73		
7:06 AM	1	11	4	2	10	0	3	0	17	2	3	0	0	0	3	0	54		
7:07 AM	0	17	12	0	0	0	1	0	0	21	2	2	6	0	12	0	71		
7:08 AM	0	6	8	0	15	0	6	0	5	1	1	1	0	0	13	0	55		
7:09 AM	6	0	4	0	0	0	4	0	0	15	6	1	14	0	6	1	55		
7:10 AM	0	22	10	0	13	0	3	0	13	7	1	0	0	0	9	0	78		
7:11 AM	2	3	3	0	10	0	3	0	0	14	2	0	4	0	15	0	56		
7:12 AM	0	11	12	0	0	0	2	0	8	7	0	0	0	0	8	1	48		
7:13 AM	0	9	3	1	14	0	3	0	0	14	3	0	12	0	8	1	66		
7:14 AM	0	12	5	1	0	0	4	0	10	12	2	0	8	0	11	1	64		
7:15 AM	6	10	8	0	13	0	4	0	1	10	1	1	2	0	16	1	71		
7:16 AM	2	6	5	0	0	0	2	0	10	13	0	0	3	0	8	1	49		
7:17 AM	0	13	5	1	8	0	4	0	10	11	6	0	0	0	20	3	77		
7:18 AM	6	3	4	1	0	0	3	0	0	25	2	0	14	0	10	0	67		
7:19 AM	0	14	9	1	10	0	6	0	7	3	3	0	0	0	18	1	70		
7:20 AM	6	7	3	0	1	0	3	0	1	4	4	0	11	0	13	1	62		
7:21 AM	0	14	3	1	11	0	5	0	10	7	0	0	0	0	16	1	61		
7:22 AM	5	11	5	0	9	0	10	0	0	18	1	0	7	0	14	1	80		
7:23 AM	0	2	6	1	0	0	6	0	12	12	1	1	0	0	14	0	53		
7:24 AM	7	13	12	0	11	0	3	0	0	27	3	0	16	0	11	2	103		
7:25 AM	0	2	2	0	0	0	3	0	9	12	2	1	9	0	9	0	48		
7:26 AM	7	15	8	0	12	0	6	0	4	15	4	0	2	0	6	1	79		
7:27 AM	1	0	4	1	0	0	5	0	0	16	3	3	6	0	13	2	59		
7:28 AM	4	16	6	1	14	0	3	0	10	7	1	0	0	0	6	0	67		
7:29 AM	6	4	8	0	0	0	2	0	0	31	2	2	13	0	15	2	81		
7:30 AM	0	17	7	1	3	0	7	0	17	7	0	0	0	0	7	0	65		
7:31 AM	8	9	7	1	1	0	2	0	0	30	4	1	11	0	12	2	84		
7:32 AM	0	17	11	0	5	0	5	0	8	8	1	0	0	0	5	1	60		
7:33 AM	2	2	7	1	4	0	3	0	0	21	1	1	14	0	8	1	62		
7:34 AM	0	16	5	0	0	0	2	0	17	15	1	0	2	0	4	0	62		
7:35 AM	6	7	3	0	0	0	5	0	0	21	3	0	0	0	13	0	75		
7:36 AM	0	6	7	0	0	0	7	0	14	17	2	2	8	0	8	0	69		
7:37 AM	4	9	6	0	2	0	7	0	1	19	0	0	2	0	5	0	55		
7:38 AM	0	9	9	1	0	0	2	0	14	27	1	2	16	0	12	0	90		
7:39 AM	10	13	8	0	7	0	10	0	9	10	1	0	0	0	5	1	73		
7:40 AM	6	1	4	1	0	0	2	0	16	43	2	2	12	0	12	0	98		
7:41 AM	0	21	17	3	6	0	5	0	10	4	2	0	0	0	9	0	75		
7:42 AM	0	3	4	0	13	0	6	0	0	29	1	0	9	0	12	1	73		
7:43 AM	0	23	9	0	5	0	8	0	9	12	0	0	0	0	7	0	72		
7:44 AM	11	5	7	0	4	0	5	0	0	23	2	0	13	0	21	2	91		
7:45 AM	0	7	4	0	1	0	8	0	12	18	1	2	0	0	11	1	62		
7:46 AM	2	18	7	0	9	0	3	0	0	24	3	0	13	0	14	0	93		
7:47 AM	0	9	3	0	0	0	0	0	15	21	2	1	13	0	8	1	71		
7:48 AM	12	16	10	1	0	0	3	0	1	19	0	0	0	0	9	2	70		
7:49 AM	3	1	7	0	14	0	6	0	9	26	2	0	15	0	11	1	93		
7:50 AM	0	15	5	0	0	0	2	0	16	17	3	0	0	0	10	0	68		
7:51 AM	3	0	9	0	11	0	5	0	0	32	1	0	17	0	9	2	87		
7:52 AM	0	21	4	0	0	0	5	1	9	7	1	0	0	0	8	0	55		
7:53 AM	5	0	7	0	4	0	8	0	0	38	1	2	11	0	8	1	82		
7:54 AM	0	27	3	0	2	0	5	0	12	11	0	1	0	0	12	0	72		
7:55 AM	0	0	3	0	12	0	8	0	0	21	1	1	8	0	14	0	67		
7:56 AM	0	21	7	0	0	0	4	0	0	7	0	1	0	0	5	0	55		
7:57 AM	0	2	2	0	0	0	9	0	12	48	3	0	0	0	13	2	89		
7:58 AM	0	12	7	0	11	0	4	0	0	13	2	0	2	0	12	0	63		
7:59 AM	11	9	3	0	1	0	13	0	0	23	2	0	18	0	10	1	90		
8:00 AM	1	10	8	0	10	0	7	0	20	19	3	1	11	0	4	1	93		
8:01 AM	5	17	11	0	0	0	6	0	13	6	1	1	0	0	9	0	68		
8:02 AM	4	0	5	0	11	0	8	0	0	32	2	1	20	0	18	0	100		
8:03 AM	0	20	9	0	0	0	8	0	13	4	2	1	0	0	5	0	61		
8:04 AM	5	1	8	1	12	0	8	1	0	13	1	0	9	0	12	1	73		
8:05 AM	0	22	6	0	1	0	5	0	6	3	1	0	0	0	12	0	56		
8:06 AM	6	9	3	0	21	0	4	0	0	8	3	0	14	0	12	1	80		
8:07 AM	0	4	4	0	12	0	6	0	6	7	1	0	0	0	11	0	51		
8:08 AM	5	11	7	0	3	0	3	0	0	7	3	0	17	0	13	0	69		
8:09 AM	0	3	4	0	17	0	11	1	5	19	1	1	10	0	11	2	81		
8:10 AM	3	28	7	0	0	0	5	0	0	10	5	0	2	0	6	1	66		
8:11 AM	5	0	3	0	13	0	7	1	8	32	2	0	10	0	12	0	92		
8:12 AM	1	21	7	1	0	0	6	0	7	16	1	1	0	0	17	0	76		
8:13 AM	0	5	8	0	17	0	6	1	0	28	4	0	21	0	13	0	102		
8:14 AM	0	23	8	0	0	0	2	0	15	4	0	1	0	0	18	1	70		
8:15 AM	5	0	15	1	25	0	10	0	1	44	1	3	8	0	11	0	120		
8:16 AM	0	30	8	0	3	0	8	0	10	9	1	1	0	0	18	1	87		
8:17 AM	5	8	8	0	12	0	4	0	0	16	1	1	9	0	13	2	74		
8:18 AM	0	15	7	0	12	0	7	0	17	17	0	2	4	0	12	0	92		
8:19 AM	10	0	15	0	1	0	10	0	0	27	4	0	9	0	19	1	95		
8:20 AM	0	10	7	0	16	0	8	1	6	13	2	0	11	0	18	1	91		
8:21 AM	3	19	4	0	1	0	8	0	0	18	0	0	7	0	12	0	72		
8:22 AM	0	16	9	0	21	0	12	1	15	18	2	1	11	0	13	0	117		
8:23 AM	4	21	7	0	0	0	8	0	8	5	2	0	0	0	8	1	63		
8:24 AM	3	4	10	0	20	0	6	0	5	37	3	0	10	0	11	2	109		
8:25 AM	0	29	8	0	0	0	2	1	13	2	2	1	0	0	21	0	77		
8:26 AM	7	0	3	0	20	0	12	0	0	33	3	2	14	0	18	2	110		
8:27 AM	0	20	11	1	5	0	3	0	13	11	2	0	0	0	11	0	76		
8:28 AM	4	6	4	1	24	0	4	0	0	17	3	0	16	0	15	0	93		
8:29 AM	0	8	6	0	17	0	8	0	14	11	0	0	4	0	13	1	81		
Approach Total	1763				1058				2245				1571				6637		
Grand Total	246	927	588	28	590	0	468	8	589	1497	159	48	584	0	967	59			
Approach %	14.1%	52.6%	33.4%	1.6%	55.8%	0.0%	44.2%	0.8%	26.2%	66.7%	7.1%	2.1%	37.2%	0.0%	62.8%	3.8%			
Total %	3.7%	14.0%	6.9%	0.4%	8.9%	0.0%	7.1%	0.1%	8.9%	22.6%	2.4%	0.7%	8.8%	0.0%	14.9%	0.9%			
Begin Peak Hour:	7:30																		
Peak Hour Vol:	172	874	412	14	411	0	359	8	403	1690	103	34	413	0	678	36	4716		
	4715 7:30 AM to 8:30 AM																		

From East LOOP 202 (WB) OFF RAMP				From South SCOTTSDALE RD				From West LOOP 202 (EB) OFF RAMP				From			
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Start Date: 1/29/2004  
Start Time: 04:30 PM  
Site Code: 4001

Street Name	From North SCOTTSDALE RD				From East LOOP 202 (WB) OFF RAMP				From South SCOTTSDALE RD				From West LOOP 202 (EB) OFF RAMP				INTSEC	HOUR		
	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks	Left	Thru	Right	Trucks			TOTAL	TOTAL
4:30 PM	9	17	11	0	10	0	2	0	0	13	0	0	10	0	6	0	78			
4:31 PM	0	15	8	1	0	0	6	0	11	19	4	0	2	0	6	0	71			
4:32 PM	20	17	11	0	9	0	5	1	0	15	7	1	11	0	5	0	100			
4:33 PM	0	8	6	0	3	0	9	0	22	17	7	0	9	0	3	0	84			
4:34 PM	9	33	2	0	5	0	7	0	0	7	7	0	3	0	4	1	79			
4:35 PM	6	6	10	0	0	0	4	0	9	29	8	0	9	0	10	0	91			
4:36 PM	6	27	2	0	10	0	4	0	14	5	9	0	0	0	10	0	87			
4:37 PM	19	9	14	1	0	0	0	0	0	33	11	0	11	0	13	0	110			
4:38 PM	2	37	9	0	4	0	7	1	23	4	9	0	0	0	5	0	100			
4:39 PM	20	6	5	0	0	0	4	0	1	45	10	0	8	0	13	0	112			
4:40 PM	0	41	6	0	17	0	5	0	23	21	5	1	0	0	3	0	121			
4:41 PM	17	3	4	0	6	0	4	0	0	8	5	0	18	0	5	1	70			
4:42 PM	0	30	7	0	0	0	4	0	18	19	10	0	4	0	4	0	96			
4:43 PM	8	20	2	0	7	0	2	1	0	20	10	1	6	0	6	0	81			
4:44 PM	0	22	8	2	0	0	7	0	14	7	6	1	5	0	6	0	75			
4:45 PM	13	29	8	0	9	0	6	0	11	14	12	0	1	0	10	1	113			
4:46 PM	0	8	7	0	0	0	8	0	4	30	5	0	13	0	8	0	83			
4:47 PM	12	36	10	0	6	0	4	0	10	6	11	0	0	0	7	1	102			
4:48 PM	11	4	1	0	0	0	7	0	11	48	6	0	7	0	5	0	93			
4:49 PM	0	33	16	0	11	0	9	0	26	7	9	0	0	0	5	0	116			
4:50 PM	17	1	4	0	2	0	5	0	0	46	12	0	18	0	10	0	115			
4:51 PM	0	34	9	1	3	0	4	0	24	4	8	0	0	0	5	1	91			
4:52 PM	10	7	6	0	3	0	4	0	0	29	13	0	11	0	5	0	88			
4:53 PM	0	25	6	0	0	0	3	0	25	25	12	0	2	0	5	0	103			
4:54 PM	22	25	9	0	9	0	6	0	0	25	11	1	6	0	8	0	121			
4:55 PM	0	7	4	1	1	0	7	0	26	17	4	0	11	4	1	1	102			
4:56 PM	12	29	9	0	0	0	0	0	0	15	10	0	7	0	8	0	90			
4:57 PM	8	8	11	1	10	0	7	0	17	33	9	0	8	0	12	0	123			
4:58 PM	0	25	5	0	0	0	4	0	15	5	12	0	0	0	6	1	72			
4:59 PM	6	4	9	0	14	0	6	1	0	29	6	0	19	0	15	0	108			
5:00 PM	10	32	8	0	0	0	5	0	19	1	14	0	0	0	6	0	93			
5:01 PM	9	0	6	0	5	0	2	0	0	36	4	1	9	0	10	0	81			
5:02 PM	0	38	10	0	3	0	2	0	22	25	10	0	0	0	11	0	78			
5:03 PM	21	19	5	1	5	0	1	0	0	5	7	0	7	0	7	0	81			
5:04 PM	0	24	10	0	7	0	8	0	11	24	8	1	3	0	8	0	103			
5:05 PM	12	20	8	0	1	0	7	0	0	12	5	0	7	0	8	0	80			
5:06 PM	0	16	8	2	6	0	6	0	21	21	11	0	10	0	7	0	106			
5:07 PM	14	37	7	0	0	0	3	0	8	7	11	0	0	0	8	0	95			
5:08 PM	0	9	3	1	11	0	3	0	9	35	7	0	16	0	5	1	98			
5:09 PM	12	38	13	0	0	0	8	0	11	7	8	0	0	0	6	0	103			
5:10 PM	13	5	2	0	0	0	10	1	7	32	11	1	4	0	9	1	98			
5:11 PM	0	29	9	0	0	0	5	0	11	15	13	0	0	0	4	0	86			
5:12 PM	15	0	4	0	2	0	4	0	0	20	7	1	17	0	6	4	75			
5:13 PM	0	52	8	0	6	0	5	0	21	12	11	0	0	0	3	0	118			
5:14 PM	16	15	10	0	3	0	6	1	0	15	10	0	11	0	7	1	93			
5:15 PM	0	29	11	0	7	0	4	0	17	9	8	0	4	0	8	0	95			
5:16 PM	23	27	13	0	0	0	7	1	0	13	11	0	11	0	7	0	112			
5:17 PM	0	11	2	0	6	0	6	0	15	17	5	0	13	0	8	0	83			
5:18 PM	14	35	9	1	0	0	4	0	4	8	11	0	0	0	7	0	92			
5:19 PM	13	6	10	0	8	0	3	0	21	27	6	0	8	0	6	0	108			
5:20 PM	6	32	5	0	0	0	3	0	19	5	7	0	0	0	4	0	81			
5:21 PM	17	0	8	0	4	0	10	1	0	40	9	0	9	0	7	0	104			
5:22 PM	0	30	12	0	0	0	8	0	19	2	6	0	0	0	5	0	82			
5:23 PM	13	1	15	0	6	0	6	0	0	29	5	0	11	0	5	1	91			
5:24 PM	0	39	7	0	0	0	5	0	17	20	11	0	0	0	7	1	106			
5:25 PM	17	11	4	0	0	0	5	0	0	13	8	0	14	0	5	0	84			
5:26 PM	0	32	12	2	5	0	6	0	19	21	4	0	6	0	10	0	115			
5:27 PM	9	26	2	0	0	0	5	0	0	21	11	0	6	0	9	0	89			
5:28 PM	0	24	10	1	6	0	9	0	15	13	3	0	7	0	9	1	96			
5:29 PM	7	38	6	0	0	0	6	0	12	11	3	1	0	0	9	0	92	5712		
5:30 PM	11	19	10	0	7	0	8	0	10	32	12	0	10	0	6	0	125	5759		
5:31 PM	5	37	6	0	0	0	3	1	15	4	8	0	0	0	5	0	83	5771		
5:32 PM	15	9	2	0	7	0	5	0	0	16	4	0	3	0	12	0	62	5733		
5:33 PM	9	45	11	1	0	0	3	0	21	4	9	0	0	0	5	0	98	5747		
5:34 PM	17	8	4	0	13	0	4	1	0	27	8	0	10	0	11	0	102	5770		
5:35 PM	0	33	8	0	6	0	5	0	11	7	9	0	0	0	7	0	86	5765		
5:36 PM	17	27	16	2	2	0	4	0	0	19	3	1	12	0	6	0	106	5784		
5:37 PM	0	13	5	0	13	0	5	0	14	19	11	0	7	0	3	1	90	5764		
5:38 PM	8	32	10	1	0	0	9	0	0	16	7	0	6	0	10	0	98	5762		
5:39 PM	9	7	2	0	18	0	5	0	19	10	7	0	10	0	5	0	74	5724		
5:40 PM	13	47	9	0	0	0	4	1	4	4	9	0	1	0	10	2	101	5704		
5:41 PM	1	4	7	0	11	0	10	0	11	27	9	1	17	0	10	0	107	5741		
5:42 PM	9	43	2	0	0	0	2	0	12	5	5	0	0	0	2	0	80	5725		
5:43 PM	13	8	9	1	7	0	9	0	0	28	1	0	8	0	8	0	91	5735		
5:44 PM	0	34	2	0	0	0	4	0	17	16	8	1	0	0	5	0	86	5746		
5:45 PM	8	0	10	0	4	0	5	0	0	17	4	0	22	0	6	0	78	5769		
5:46 PM	0	54	10	1	5	0	8	1	21	13	4	1	0	0	3	0	118	5744		
5:47 PM	22	28	2	0	3	0	4	0	0	7	8	0	18	0	10	1	102	5744		
5:48 PM	0	29	21	0	11	0	3	0	10	21	8	0	10	0	0	0	113	5764		
5:49 PM	17	32	5	0	2	0	5	0	0	24	3	0	1	0	16	0	105	5753		
5:50 PM	3	14	8	1	10	0	6	0	13	18	1	0	9	0	7	0	89	5727		
5:51 PM	10	34	9	0	0	0	5	0	7	6	15	1	0	0	13	0	99	5735		
5:52 PM	7	11	8	0	14	0	7	0	1	31	4	0	12	0	13	1	108	5756		
5:53 PM	3	32	12	0	0	0	2	0	11	5	10	0	0	0	9	0	84	5736		
5:54 PM	14	0	1	1	10	0	5	0	0	30	2	1	11	0	10	0	83	5698		
5:55 PM	0	42	9	1	0	0	1	0	14	1	7	0	0	0	8	0	82	5699		
5:56 PM	15	17	5	0	8	0	3	0	0	29	11	1	11	0	9	0	108	5717		
5:57 PM	0	32	15	0	12	0	9	1	12	16	6	0	0	0	8	1	110	5704		
5:58 PM	7	18	9	0	2	0	4	0	0	11	3	0	15	0	3	1	72	5704		
5:59 PM	0	8	9	1	15	0	7	1	19	23	4	0	11	0	4	0	100	5696		
Approach Total	3324				880				3123				1223							
Grand Total	683	1949	692	26	422	0	458	13	835	1597	691	17	576	0	647	24	8550			
Approach %	20.5%	58.6%	20.8%	0.8%	48.0%	0.0%	52.0%	1.5%	26.7%	51.1%	22.1%	0.5%	47.1%	0.0%	52.9%	2.0%				

File Name	Location	Location2	Directi on	Count Dur	Start Date	Start Time	Avg PkHr	AM PkVol	AM PkHr	AM PHF	PM PkVol	PM PkHr	PM PHF	Latitude	Longitude
0400117	LOOP 101 (WEST) NB OFF RAMP	S. of BELL RD	NB	24	1/28/2004	12:00	16204	1123	0.8406	16:45	1367	0.9906	33.63528	-112.23822	
0400118	LOOP 101 (WEST) NB OFF RAMP-RT TURNS	S. of BELL RD	NB	24	1/28/2004	0:00	10097	813	0.9366	12:00	836	0.9048	33.63777	-112.23722	
0400119	BELL RD	BTWN LOOP 101(WEST) NB OFF RAMP (T-SPLIT)	EB	24	1/28/2004	0:00	22210	1953	0.9630	12:00	1851	0.9127	33.63798	-112.23710	
0400120	BELL RD	E. of LOOP 101 (WEST) NB ON RAMP	WB	24	1/28/2004	0:00	26977	1145	0.9348	15:00	2281	0.9348	33.63835	-112.23692	
0400121	LOOP 101 (WEST) SB OFF RAMP	N. of BELL RD	SB	24	1/28/2004	0:00	16667	715	0.8405	16:45	1385	0.9699	33.64108	-112.23682	
0400122	LOOP 101 (WEST) SB OFF RAMP-RT TURNS	N. of BELL RD	SB	24	1/28/2004	0:00	12760	1067	0.8441	16:45	1022	0.9642	33.63867	-112.23620	
0400123	BELL RD	BTWN LOOP 101(WEST) SB OFF RAMP (T-SPLIT)	WB	24	1/28/2004	0:00	27065	1145	0.9447	15:00	2387	0.9281	33.63833	-112.23637	
0400124	BELL RD	W. of LOOP 101 (WEST) SB ON RAMP	EB	24	1/28/2004	0:00	37409	2750	0.9683	16:30	2768	0.9665	33.63812	-112.24095	
0400125	SR-51 NB OFF RAMP	S. of GLENDALE AVE	NB	24	2/3/2004	0:00	8790	754	0.9472	15:00	639	0.9623	33.53799	-112.04113	
0400126	SR-51 NB OFF RAMP-RT TURNS	S. of GLENDALE AVE	NB	24	2/3/2004	0:00	3240	615	0.8737	13:30	238	0.8750	33.53611	-112.04118	
0400127	GLENDALE AVE	BTWN SR-51 NB OFF RAMP (T-SPLIT)	EB	24	2/3/2004	0:00	25584	715	0.9723	17:00	1694	0.9691	33.53826	-112.04119	
0400128	GLENDALE AVE	E. of SR-51 NB ON RAMP	WB	24	2/3/2004	0:00	24184	800	0.9711	17:15	2929	0.9803	33.53845	-112.04326	
0400129	SR-51 SB OFF RAMP	N. of GLENDALE AVE	SB	24	2/3/2004	0:00	13798	830	0.9183	16:00	815	0.9178	33.54078	-112.04140	
0400130	SR-51 SB OFF RAMP-RT TURNS	N. of GLENDALE AVE	SB	24	2/3/2004	0:00	4317	830	0.6200	17:30	331	0.9298	33.53866	-112.04196	
0400131	GLENDALE AVE	BTWN SR-51 SB OFF RAMP (T-SPLIT)	WB	24	2/3/2004	0:00	20613	730	0.8674	16:30	1910	0.9608	33.53835	-112.04198	
0400132	GLENDALE AVE	W. of SR-51 SB ON RAMP	EB	24	2/3/2004	0:00	24496	515	0.9219	15:00	2041	0.9484	33.53816	-112.04301	
0400133	SR-51 NB OFF RAMP	S. of INDIAN SCHOOL RD	NB	24	1/20/2004	0:00	8234	715	0.819	0.8625	12:00	678	0.9212	33.49205	-112.04362
0400134	SR-51 NB OFF RAMP-RT TURNS	S. of INDIAN SCHOOL RD	NB	24	1/20/2004	0:00	3872	1115	0.8750	14:15	334	0.8978	33.49440	-112.04351	
0400135	INDIAN SCHOOL RD	BTWN SR-51 NB OFF RAMP (T-SPLIT)	EB	24	1/20/2004	0:00	23208	1115	0.9107	16:45	2040	0.8629	33.49461	-112.04343	
0400136	INDIAN SCHOOL RD	E. of SR-51 NB ON RAMP	WB	24	1/20/2004	0:00	24896	715	0.9016	15:45	1935	0.9579	33.49489	-112.04126	
0400137	SR-51 SB OFF RAMP	N. of INDIAN SCHOOL RD	SB	24	1/20/2004	0:00	10214	830	0.9751	15:30	710	0.9245	33.49755	-112.04408	
0400138	SR-51 SB OFF RAMP-RT TURNS	N. of INDIAN SCHOOL RD	SB	24	1/20/2004	0:00	4940	800	0.9147	12:00	336	0.8485	33.49509	-112.04437	
0400139	INDIAN SCHOOL RD	BTWN SR-51 SB OFF RAMP (T-SPLIT)	WB	24	1/20/2004	0:00	25662	730	0.823	16:30	2111	0.9275	33.49476	-112.04430	
0400140	INDIAN SCHOOL RD	W. of SR-51 SB ON RAMP	EB	24	1/20/2004	0:00	31791	1115	0.9731	16:45	2852	0.8890	33.49457	-112.04629	
0400141	SR-51 NB OFF RAMP	S. of CACTUS RD	NB	24	1/21/2004	0:00	11284	1145	0.675	0.8789	17:00	999	0.9182	33.59393	-112.00919
0400142	SR-51 NB OFF RAMP-RT TURNS	S. of CACTUS RD	NB	24	1/21/2004	0:00	6296	1145	0.8479	17:45	576	0.9412	33.59666	-112.00895	
0400143	CACTUS RD	BTWN SR-51 NB OFF RAMP (T-SPLIT)	EB	24	1/21/2004	0:00	20258	730	0.8644	16:45	1690	0.9431	33.59661	-112.00904	
0400144	CACTUS RD	E. of SR-51 NB ON RAMP	WB	24	1/21/2004	0:00	23398	700	0.9411	16:45	2013	0.9051	33.59720	-112.00584	
0400145	SR-51 SB OFF RAMP	N. of CACTUS RD	SB	24	1/21/2004	0:00	9483	730	0.8272	16:45	788	0.9752	33.60019	-112.00975	
0400146	SR-51 SB OFF RAMP-RT TURNS	N. of CACTUS RD	SB	24	1/21/2004	0:00	4475	730	0.505	0.8417	15:15	384	0.8205	33.59747	-112.01011
0400147	CACTUS RD	BTWN SR-51 SB OFF RAMP (T-SPLIT)	WB	24	1/21/2004	0:00	17505	1145	0.9258	16:45	1646	0.9395	33.59705	-112.01020	
0400148	CACTUS RD	W. of SR-51 SB ON RAMP	EB	24	1/21/2004	0:00	23799	715	0.9536	17:00	1963	0.9474	33.59685	-112.01150	
0400149	SR-51 NB OFF RAMP	S. of GREENWAY RD	NB	24	1/27/2004	0:00	12831	730	0.9192	15:45	1205	0.8783	33.62380	-112.00748	
0400150	SR-51 NB OFF RAMP-RT TURNS	S. of GREENWAY RD	NB	24	1/27/2004	0:00	6618	630	0.8146	17:15	598	0.9061	33.62593	-112.00630	
0400151	GREENWAY RD	BTWN SR-51 NB OFF RAMP (T-SPLIT)	EB	24	1/27/2004	0:00	19866	715	0.9082	16:30	1486	0.9751	33.62618	-112.00613	
0400152	GREENWAY RD	E. of SR-51 NB ON RAMP	WB	24	1/27/2004	0:00	28811	800	0.9096	16:45	3104	0.9545	33.62647	-112.00517	
0400153	SR-51 SB OFF RAMP	N. of GREENWAY RD	SB	24	1/27/2004	0:00	7926	730	0.8647	17:00	799	0.9248	33.62847	-112.00625	
0400154	SR-51 SB OFF RAMP-RT TURNS	N. of GREENWAY RD	SB	24	1/27/2004	0:00	3291	1115	0.8565	16:30	352	0.8302	33.62677	-112.00723	
0400155	GREENWAY RD	BTWN SR-51 SB OFF RAMP (T-SPLIT)	WB	24	1/27/2004	0:00	22790	1145	0.9512	16:30	2634	0.9828	33.62648	-112.00740	
0400156	GREENWAY RD	W. of SR-51 SB ON RAMP	EB	24	1/27/2004	0:00	20235	700	0.9380	16:00	1226	0.9519	33.62633	-112.00840	
0400157	LOOP 202 EB OFF RAMP	W. of SCOTTSDALE RD	EB	24	1/29/2004	0:00	15650	800	0.9054	14:00	1004	0.9544	33.43567	-111.92925	
0400158	LOOP 202 EB OFF RAMP-RT TURNS	W. of SCOTTSDALE RD	EB	24	1/29/2004	0:00	8339	800	0.731	0.9277	12:30	495	0.8839	33.43567	-111.92865
0400159	SCOTTSDALE RD	BTWN LOOP 202 EB OFF RAMP (T-SPLIT)	SB	24	1/29/2004	0:00	17389	800	0.941	0.8730	17:00	1365	0.9004	33.43568	-111.92648
0400160	SCOTTSDALE RD	S. of LOOP 202 EB ON RAMP	NB	24	1/29/2004	0:00	32116	1145	0.8697	15:00	2884	0.8945	33.43458	-111.92610	
0400161	LOOP 202 WB OFF RAMP	E. of SCOTTSDALE RD	WB	24	1/29/2004	0:00	1919	815	0.8848	12:45	859	0.9177	33.43643	-111.92382	
0400162	LOOP 202 WB OFF RAMP-RT TURNS	E. of SCOTTSDALE RD	WB	24	1/29/2004	0:00	6026	900	0.8480	12:15	410	0.9579	33.43652	-111.92587	
0400163	SCOTTSDALE RD	BTWN LOOP 202 WB OFF RAMP (T-SPLIT)	NB	24	1/29/2004	0:00	24488	730	0.9241	15:00	1776	0.9507	33.43660	-111.92613	
0400164	SCOTTSDALE RD	N. of LOOP 202 WB ON RAMP	SB	24	1/29/2004	0:00	27418	1130	0.9097	17:15	2380	0.9917	33.43737	-111.92648	

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 3844 East Indian School Road  
 Phoenix, AZ 85018  
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File Name	Location	Location2	Direction	Count Dur	Start Date	Start Time	Avg Vol	AM PkHr	AM PkVol	AM PHF	PM PkHr	PM PkVol	PM PHF
0400133	SR-51 NB OFF RAMP	S. of INDIAN SCHOOL RD	NB	24	1/20/2004	0:00	8234	7:15	819	0.8825	12:00	678	0.9212
0400134	SR-51 NB OFF RAMP-RT TURNS	S. of INDIAN SCHOOL RD	NB	24	1/20/2004	0:00	3872	11:15	350	0.8750	14:15	334	0.8978
0400135	INDIAN SCHOOL RD	BTWN SR-51 NB OFF RAMP (T-SPLIT)	EB	24	1/20/2004	0:00	23208	11:15	1632	0.9107	16:45	2040	0.8629
0400136	INDIAN SCHOOL RD	E. of SR-51 NB ON RAMP	WB	24	1/20/2004	0:00	24896	7:15	1641	0.9016	15:45	1935	0.9579
0400137	SR-51 SB OFF RAMP	N. of INDIAN SCHOOL RD	SB	24	1/20/2004	0:00	10214	8:30	979	0.9751	15:30	710	0.9245
0400138	SR-51 SB OFF RAMP-RT TURNS	N. of INDIAN SCHOOL RD	SB	24	1/20/2004	0:00	4940	8:00	609	0.9117	12:00	336	0.8485
0400139	INDIAN SCHOOL RD	BTWN SR-51 SB OFF RAMP (T-SPLIT)	WB	24	1/20/2004	0:00	25662	7:30	1823	0.8648	15:30	2111	0.9275
0400140	INDIAN SCHOOL RD	W. of SR-51 SB ON RAMP	EB	24	1/20/2004	0:00	31791	11:15	2246	0.9731	16:45	2852	0.8890

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File Name: 0400133  
 Duration: 24  
 Site Code: 3  
 TRA ID: 0400133  
 Location 1: SR-51 NB OFF RAMP S. of INDIAN SCHOOL RD  
 Count Type: VOL  
 Direction: NB  
 Latitude: 33.49205  
 Longitude: -112.04362

Date	1/20/2004		Average	
	AM	PM	AM	PM
12:00	12	164	12	164
12:15	5	168	5	168
12:30	9	184	9	184
12:45	8	162	8	162
1:00	9	143	9	143
1:15	3	148	3	148
1:30	12	155	12	155
1:45	6	136	6	136
2:00	7	136	7	136
2:15	1	152	1	152
2:30	7	172	7	172
2:45	5	190	5	190
3:00	3	153	3	153
3:15	8	148	8	148
3:30	3	156	3	156
3:45	4	138	4	138
4:00	4	135	4	135
4:15	13	121	13	121
4:30	13	111	13	111
4:45	13	103	13	103
5:00	39	111	39	111
5:15	37	78	37	78
5:30	49	119	49	119
5:45	89	152	89	152
6:00	103	120	103	120
6:15	123	1	123	1
6:30	137	4	137	4
6:45	167	3	167	3
7:00	191	4	191	4
7:15	191	1	191	1
7:30	201	5	201	5
7:45	232	3	232	3
8:00	195	0	195	0
8:15	186	2	186	2
8:30	188	6	188	6
8:45	214	5	214	5
9:00	181	8	181	8
9:15	158	2	158	2
9:30	176	2	176	2
9:45	157	6	157	6
10:00	163	16	163	16
10:15	123	25	123	25
10:30	134	33	134	33
10:45	150	57	150	57
11:00	160	52	160	52
11:15	156	29	156	29
11:30	158	20	158	20
11:45	168	24	168	24
<b>Total</b>	<b>4371</b>	<b>3863</b>	<b>4371</b>	<b>3863</b>
<b>Day Total</b>	<b>8234</b>		<b>8234</b>	
<b>Split</b>	1.1315		1.1315	

AM Peak Hour 7:15 7:15  
 AM Peak Vol 819 819  
 AM PHF 0.8825 0.8825  
 PM Peak Hour 12:00 12:00  
 PM Peak Vol 678 678  
 PM PHF 0.9212 0.9212

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 3844 East Indian School Road  
 Phoenix, AZ 85018  
 (602) 840-1500

File Name: 0400134  
 Duration: 24  
 Site Code: 3  
 TRA ID: 0400134  
 Location 1: SR-51 NB OFF RAMP-RT TURNS S. of INDIAN SCHOOL RD  
 Count Type: VOL  
 Direction: NB  
 Latitude: 33.49440  
 Longitude: -112.04351

Date Time	1/20/2004		Average	
	AM	PM	AM	PM
12:00	8	88	8	88
12:15	3	78	3	78
12:30	4	83	4	83
12:45	4	76	4	76
1:00	5	68	5	68
1:15	3	76	3	76
1:30	8	85	8	85
1:45	1	57	1	57
2:00	5	62	5	62
2:15	1	66	1	66
2:30	1	86	1	86
2:45	2	93	2	93
3:00	1	89	1	89
3:15	4	64	4	64
3:30	1	70	1	70
3:45	1	65	1	65
4:00	2	75	2	75
4:15	18	61	18	61
4:30	8	59	8	59
4:45	9	49	9	49
5:00	22	38	22	38
5:15	15	34	15	34
5:30	17	65	17	65
5:45	28	73	28	73
6:00	41	56	41	56
6:15	38	38	38	38
6:30	48	29	48	29
6:45	27	32	27	32
7:00	70	16	70	16
7:15	68	24	68	24
7:30	71	13	71	13
7:45	98	17	98	17
8:00	77	23	77	23
8:15	69	12	69	12
8:30	67	11	67	11
8:45	98	16	98	16
9:00	75	2	75	2
9:15	62	17	62	17
9:30	75	13	75	13
9:45	83	6	83	6
10:00	66	5	66	5
10:15	57	2	57	2
10:30	58	9	58	9
10:45	79	6	79	6
11:00	76	7	76	7
11:15	79	6	79	6
11:30	100	10	100	10
11:45	83	6	83	6
<b>Total</b>	<b>1836</b>	<b>2036</b>	<b>1836</b>	<b>2036</b>
<b>Day Total</b>	<b>3872</b>		<b>3872</b>	
<b>Split</b>	0.9018		0.9018	

AM Peak Hour 11:15 11:15  
 AM Peak Vol 350 350  
 AM PHF 0.8750 0.8750  
 PM Peak Hour 14:15 14:15  
 PM Peak Vol 334 334  
 PM PHF 0.8978 0.8978

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File Name: 0400135  
 Duration: 24  
 Site Code: 3  
 TRA ID: 0400135  
 Location 1: INDIAN SCHOOL RD BTWN SR-51 NB OFF RAMPS (T-SPLIT)  
 Count Type: VOL  
 Direction: EB  
 Latitude: 33.49461  
 Longitude: -112.04343

Date Time	1/20/2004		Average	
	AM	PM	AM	PM
12:00	57	448	57	448
12:15	45	366	45	366
12:30	31	373	31	373
12:45	36	413	36	413
1:00	30	372	30	372
1:15	36	390	36	390
1:30	22	420	22	420
1:45	26	424	26	424
2:00	16	393	16	393
2:15	17	429	17	429
2:30	11	390	11	390
2:45	15	403	15	403
3:00	17	442	17	442
3:15	12	460	12	460
3:30	17	460	17	460
3:45	17	503	17	503
4:00	18	493	18	493
4:15	28	474	28	474
4:30	33	470	33	470
4:45	49	499	49	499
5:00	67	433	67	433
5:15	71	591	71	591
5:30	91	517	91	517
5:45	104	446	104	446
6:00	105	449	105	449
6:15	159	412	159	412
6:30	182	310	182	310
6:45	220	326	220	326
7:00	244	284	244	284
7:15	239	256	239	256
7:30	336	236	336	236
7:45	306	210	306	210
8:00	346	244	346	244
8:15	331	193	331	193
8:30	310	167	310	167
8:45	379	188	379	188
9:00	339	174	339	174
9:15	320	147	320	147
9:30	320	103	320	103
9:45	350	84	350	84
10:00	394	81	394	81
10:15	316	75	316	75
10:30	304	68	304	68
10:45	363	98	363	98
11:00	367	53	367	53
11:15	379	54	379	54
11:30	421	57	421	57
11:45	384	50	384	50
<b>Total</b>	<b>8280</b>	<b>14928</b>	<b>8280</b>	<b>14928</b>
<b>Day Total</b>	<b>23208</b>		<b>23208</b>	
<b>Split</b>	0.5547		0.5547	

AM Peak Hour 11:15 11:15  
 AM Peak Vol 1632 1632  
 AM PHF 0.9107 0.9107  
 PM Peak Hour 16:45 16:45  
 PM Peak Vol 2040 2040  
 PM PHF 0.8629 0.8629

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File Name: 0400136  
 Duration: 24  
 Site Code: 3  
 TRA ID: 0400136  
 Location 1: INDIAN SCHOOL RD E. of SR-51 NB ON RAMPS  
 Count Type: VOL  
 Direction: WB  
 Latitude: 33.49489  
 Longitude: -112.04126

Date	1/20/2004		Average	
	AM	PM	AM	PM
12:00	58	395	58	395
12:15	48	418	48	418
12:30	39	407	39	407
12:45	31	413	31	413
1:00	38	455	38	455
1:15	36	397	36	397
1:30	35	492	35	492
1:45	34	395	34	395
2:00	21	431	21	431
2:15	17	428	17	428
2:30	16	393	16	393
2:45	22	479	22	479
3:00	23	482	23	482
3:15	21	490	21	490
3:30	21	441	21	441
3:45	29	473	29	473
4:00	27	487	27	487
4:15	38	505	38	505
4:30	42	470	42	470
4:45	60	471	60	471
5:00	47	470	47	470
5:15	87	413	87	413
5:30	144	490	144	490
5:45	117	403	117	403
6:00	157	484	157	484
6:15	176	419	176	419
6:30	278	389	278	389
6:45	278	349	278	349
7:00	279	330	279	330
7:15	363	302	363	302
7:30	452	222	452	222
7:45	455	277	455	277
8:00	371	224	371	224
8:15	353	232	353	232
8:30	365	203	365	203
8:45	343	188	343	188
9:00	280	210	280	210
9:15	319	224	319	224
9:30	315	170	315	170
9:45	332	157	332	157
10:00	285	146	285	146
10:15	313	135	313	135
10:30	337	122	337	122
10:45	323	111	323	111
11:00	343	81	343	81
11:15	382	94	382	94
11:30	400	77	400	77
11:45	418	84	418	84
<b>Total</b>	<b>8968</b>	<b>15928</b>	<b>8968</b>	<b>15928</b>
<b>Day Total</b>	<b>24896</b>		<b>24896</b>	
<b>Split</b>	0.5630		0.5630	

AM Peak Hour	7:15	7:15
AM Peak Vol	1641	1641
AM PHF	0.9016	0.9016
PM Peak Hour	15:45	15:45
PM Peak Vol	1935	1935
PM PHF	0.9579	0.9579

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File Name: 0400137  
 Duration: 24  
 Site Code: 3  
 TRA ID: 0400137  
 Location 1: SR-51 SB OFF RAMPS N. of INDIAN SCHOOL RD  
 Count Type: VOL  
 Direction: SB  
 Latitude: 33.49755  
 Longitude: -112.04408

Date	1/20/2004		Average	
	AM	PM	AM	PM
12:00	12	157	12	157
12:15	20	168	20	168
12:30	18	141	18	141
12:45	11	161	11	161
1:00	17	186	17	186
1:15	12	148	12	148
1:30	18	163	18	163
1:45	9	159	9	159
2:00	7	164	7	164
2:15	5	159	5	159
2:30	6	176	6	176
2:45	6	180	6	180
3:00	5	175	5	175
3:15	9	159	9	159
3:30	4	187	4	187
3:45	5	168	5	168
4:00	12	192	12	192
4:15	9	163	9	163
4:30	9	183	9	183
4:45	14	150	14	150
5:00	19	151	19	151
5:15	35	153	35	153
5:30	48	174	48	174
5:45	58	159	58	159
6:00	92	131	92	131
6:15	102	90	102	90
6:30	135	106	135	106
6:45	141	93	141	93
7:00	172	80	172	80
7:15	198	97	198	97
7:30	186	100	186	100
7:45	186	95	186	95
8:00	219	74	219	74
8:15	226	62	226	62
8:30	248	78	248	78
8:45	251	79	251	79
9:00	239	76	239	76
9:15	241	77	241	77
9:30	191	52	191	52
9:45	201	51	201	51
10:00	221	53	221	53
10:15	157	39	157	39
10:30	144	30	144	30
10:45	119	20	119	20
11:00	148	27	148	27
11:15	175	19	175	19
11:30	155	25	155	25
11:45	148	21	148	21
<b>Total</b>	<b>4663</b>	<b>5551</b>	<b>4663</b>	<b>5551</b>
<b>Day Total</b>	<b>10214</b>		<b>10214</b>	
<b>Split</b>	0.8400		0.8400	

AM Peak Hour	8:30	8:30
AM Peak Vol	979	979
AM PHF	0.9751	0.9751
PM Peak Hour	15:30	15:30
PM Peak Vol	710	710
PM PHF	0.9245	0.9245

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File Name: 0400138  
 Duration: 24  
 Site Code: 3  
 TRA ID: 0400138  
 Location 1: SR-51 SB OFF RAMPS-RT TURNS N. of INDIAN SCHOOL RD  
 Count Type: VOL  
 Direction: SB  
 Latitude: 33.49509  
 Longitude: -112.04430

Date Time	1/20/2004		Average	
	AM	PM	AM	PM
12:00	7	81	7	81
12:15	3	72	3	72
12:30	5	84	5	84
12:45	7	99	7	99
1:00	5	76	5	76
1:15	9	74	9	74
1:30	1	76	1	76
1:45	2	77	2	77
2:00	2	74	2	74
2:15	1	78	1	78
2:30	4	93	4	93
2:45	4	88	4	88
3:00	1	74	1	74
3:15	3	73	3	73
3:30	2	84	2	84
3:45	5	89	5	89
4:00	4	71	4	71
4:15	3	72	3	72
4:30	10	61	10	61
4:45	9	58	9	58
5:00	16	70	16	70
5:15	29	59	29	59
5:30	37	55	37	55
5:45	71	53	71	53
6:00	67	31	67	31
6:15	87	24	87	24
6:30	109	23	109	23
6:45	126	25	126	25
7:00	132	24	132	24
7:15	133	16	133	16
7:30	113	10	113	10
7:45	140	16	140	16
8:00	141	19	141	19
8:15	158	11	158	11
8:30	167	21	167	21
8:45	143	9	143	9
9:00	135	11	135	11
9:15	111	9	111	9
9:30	125	9	125	9
9:45	118	12	118	12
10:00	66	5	66	5
10:15	75	6	75	6
10:30	62	5	62	5
10:45	62	8	62	8
11:00	82	6	82	6
11:15	85	5	85	5
11:30	69	5	69	5
11:45	85	8	85	8
<b>Total</b>	<b>2831</b>	<b>2109</b>	<b>2831</b>	<b>2109</b>
<b>Day Total</b>	<b>4940</b>		<b>4940</b>	
<b>Split</b>	1.3423		1.3423	

AM Peak Hour 8:00 8:00  
 AM Peak Vol 609 609  
 AM PHF 0.9117 0.9117  
 PM Peak Hour 12:00 12:00  
 PM Peak Vol 336 336  
 PM PHF 0.8485 0.8485

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File Name: 0400139  
 Duration: 24  
 Site Code: 3  
 TRA ID: 0400139  
 Location 1: INDIAN SCHOOL RD BTWN SR-51 SB OFF RAMPS (T-SPLIT)  
 Count Type: VOL  
 Direction: WB  
 Latitude: 33.49476  
 Longitude: -112.04437

Date Time	1/20/2004		Average	
	AM	PM	AM	PM
12:00	42	362	42	362
12:15	32	363	32	363
12:30	30	393	30	393
12:45	29	366	29	366
1:00	27	364	27	364
1:15	25	344	25	344
1:30	28	393	28	393
1:45	31	431	31	431
2:00	15	474	15	474
2:15	18	494	18	494
2:30	21	447	21	447
2:45	20	519	20	519
3:00	16	517	16	517
3:15	21	504	21	504
3:30	15	526	15	526
3:45	27	504	27	504
4:00	15	569	15	569
4:15	28	512	28	512
4:30	26	480	26	480
4:45	32	500	32	500
5:00	53	505	53	505
5:15	77	458	77	458
5:30	110	499	110	499
5:45	124	468	124	468
6:00	176	528	176	528
6:15	219	473	219	473
6:30	265	486	265	486
6:45	371	427	371	427
7:00	388	315	388	315
7:15	395	334	395	334
7:30	462	229	462	229
7:45	527	302	527	302
8:00	436	278	436	278
8:15	398	247	398	247
8:30	437	193	437	193
8:45	405	216	405	216
9:00	345	176	345	176
9:15	349	161	349	161
9:30	351	154	351	154
9:45	319	115	319	115
10:00	312	113	312	113
10:15	305	117	305	117
10:30	320	91	320	91
10:45	314	108	314	108
11:00	336	77	336	77
11:15	321	91	321	91
11:30	336	63	336	63
11:45	368	59	368	59
<b>Total</b>	<b>9317</b>	<b>16345</b>	<b>9317</b>	<b>16345</b>
<b>Day Total</b>	<b>25662</b>		<b>25662</b>	
<b>Split</b>	0.5700		0.5700	

AM Peak Hour 7:30 7:30  
 AM Peak Vol 1823 1823  
 AM PHF 0.8648 0.8648  
 PM Peak Hour 15:30 15:30  
 PM Peak Vol 2111 2111  
 PM PHF 0.9275 0.9275

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File Name: 0400140  
 Duration: 24  
 Site Code: 3  
 TRA ID: 0400140  
 Location 1: INDIAN SCHOOL RD W. of SR-51 SB ON RAMP  
 Count Type: VOL  
 Direction: EB  
 Latitude: 33.49457  
 Longitude: -112.04629

Date Time	1/20/2004		Average	
	AM	PM	AM	PM
12:00	68	577	68	577
12:15	60	542	60	542
12:30	39	525	39	525
12:45	59	570	59	570
1:00	49	558	49	558
1:15	34	507	34	507
1:30	29	554	29	554
1:45	40	549	40	549
2:00	31	570	31	570
2:15	19	567	19	567
2:30	24	560	24	560
2:45	11	617	11	617
3:00	24	607	24	607
3:15	26	575	26	575
3:30	31	663	31	663
3:45	17	715	17	715
4:00	21	702	21	702
4:15	34	637	34	637
4:30	43	642	43	642
4:45	77	708	77	708
5:00	70	645	70	645
5:15	122	802	122	802
5:30	132	697	132	697
5:45	155	620	155	620
6:00	177	601	177	601
6:15	221	598	221	598
6:30	341	494	341	494
6:45	312	447	312	447
7:00	337	381	337	381
7:15	316	346	316	346
7:30	473	308	473	308
7:45	393	281	393	281
8:00	451	300	451	300
8:15	360	242	360	242
8:30	383	262	383	262
8:45	415	226	415	226
9:00	356	235	356	235
9:15	412	218	412	218
9:30	417	206	417	206
9:45	407	156	407	156
10:00	455	140	455	140
10:15	404	149	404	149
10:30	442	119	442	119
10:45	408	156	408	156
11:00	451	107	451	107
11:15	565	98	565	98
11:30	567	105	567	105
11:45	537	92	537	92
<b>Total</b>	<b>10815</b>	<b>20976</b>	<b>10815</b>	<b>20976</b>
<b>Day Total</b>	<b>31791</b>		<b>31791</b>	
<b>Split</b>	0.5156		0.5156	

AM Peak Hour 11:15 11:15  
 AM Peak Vol 2246 2246  
 AM PHF 0.9731 0.9731  
 PM Peak Hour 16:45 16:45  
 PM Peak Vol 2852 2852  
 PM PHF 0.8890 0.8890

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Site ID	File Name	Location	Location2	Direction	Count	Start Date	Start Time	Avg Vol	AM PkHr	AM PKVol	AM PHF	PM PkHr	PM PKVol	PM PHF
2	0400125	SR-51 NB OFF RAMP	S. of GLENDALE AVE	NB	24	2/3/2004	0:00	8790	7:15	754	0.9472	15:00	639	0.9623
2	0400126	SR-51 NB OFF RAMP-RT TURNS	S. of GLENDALE AVE	NB	24	2/3/2004	0:00	3240	6:15	339	0.8737	13:30	238	0.8750
2	0400127	GLENDALE AVE	BTWN SR-51 NB OFF RAMP (T-SPLIT)	EB	24	2/3/2004	0:00	25584	7:15	3302	0.9723	17:00	1694	0.9691
2	0400128	GLENDALE AVE	E. of SR-51 NB ON RAMP	WB	24	2/3/2004	0:00	24184	8:00	1378	0.9211	17:15	2929	0.9803
2	0400129	SR-51 SB OFF RAMP	N. of GLENDALE AVE	SB	24	2/3/2004	0:00	13798	8:30	1708	0.9183	16:00	815	0.9178
2	0400130	SR-51 SB OFF RAMP-RT TURNS	N. of GLENDALE AVE	SB	24	2/3/2004	0:00	4317	8:30	462	0.7000	17:30	331	0.9298
2	0400131	GLENDALE AVE	BTWN SR-51 SB OFF RAMP (T-SPLIT)	WB	24	2/3/2004	0:00	20613	7:30	1419	0.8674	16:30	1910	0.9608
2	0400132	GLENDALE AVE	W. of SR-51 SB ON RAMP	EB	24	2/3/2004	0:00	24496	5:15	2231	0.9219	15:00	2041	0.9484

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File Name: 0400125  
 Duration: 24  
 Site Code: 2  
 TRA ID: 0400125  
 Location 1: SR-51 NB OFF RAMP S. of GLENDALE AVE  
 Count Type: VOL  
 Direction: NB  
 Latitude: 33.53799  
 Longitude: -112.04113

Date	2/3/2004		Average	
	AM	PM	AM	PM
12:00	16	145	16	145
12:15	7	128	7	128
12:30	5	146	5	146
12:45	16	119	16	119
1:00	9	133	9	133
1:15	5	131	5	131
1:30	10	133	10	133
1:45	4	141	4	141
2:00	5	140	5	140
2:15	4	156	4	156
2:30	10	180	10	180
2:45	5	118	5	118
3:00	5	154	5	154
3:15	2	166	2	166
3:30	5	156	5	156
3:45	11	163	11	163
4:00	9	136	9	136
4:15	7	130	7	130
4:30	12	150	12	150
4:45	24	110	24	110
5:00	22	104	22	104
5:15	29	104	29	104
5:30	43	84	43	84
5:45	100	70	100	70
6:00	75	95	75	95
6:15	100	98	100	98
6:30	135	159	135	159
6:45	179	150	179	150
7:00	168	95	168	95
7:15	199	106	199	106
7:30	169	96	169	96
7:45	195	104	195	104
8:00	191	86	191	86
8:15	174	87	174	87
8:30	163	75	163	75
8:45	177	53	177	53
9:00	158	72	158	72
9:15	147	82	147	82
9:30	123	54	123	54
9:45	146	66	146	66
10:00	113	46	113	46
10:15	124	41	124	41
10:30	107	38	107	38
10:45	121	33	121	33
11:00	118	29	118	29
11:15	139	23	139	23
11:30	139	21	139	21
11:45	143	16	143	16
<b>Total</b>	<b>3868</b>	<b>4922</b>	<b>3868</b>	<b>4922</b>
<b>Day Total</b>	<b>8790</b>		<b>8790</b>	
<b>Split</b>	0.7859		0.7859	

AM Peak Hour 7:15 7:15  
 AM Peak Vol 754 754  
 AM PHF 0.9472 0.9472  
 PM Peak Hour 15:00 15:00  
 PM Peak Vol 639 639  
 PM PHF 0.9623 0.9623

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File Name: 0400126  
 Duration: 24  
 Site Code: 2  
 TRA ID: 0400126  
 Location 1: SR-51 NB OFF RAMP-RT TURNS S. of GLENDALE AVE  
 Count Type: VOL  
 Direction: NB  
 Latitude: 33.53611  
 Longitude: -112.04118

Date	2/3/2004		Average	
	Time	AM	PM	AM
12:00	2	51	2	51
12:15	1	58	1	58
12:30	4	47	4	47
12:45	2	43	2	43
1:00	1	42	1	42
1:15	0	40	0	40
1:30	0	55	0	55
1:45	0	48	0	48
2:00	1	67	1	67
2:15	2	68	2	68
2:30	2	33	2	33
2:45	2	56	2	56
3:00	0	61	0	61
3:15	2	66	2	66
3:30	6	50	6	50
3:45	4	51	4	51
4:00	2	53	2	53
4:15	5	43	5	43
4:30	14	29	14	29
4:45	11	36	11	36
5:00	19	36	19	36
5:15	27	34	27	34
5:30	44	26	44	26
5:45	37	33	37	33
6:00	51	40	51	40
6:15	72	55	72	55
6:30	97	48	97	48
6:45	73	39	73	39
7:00	97	40	97	40
7:15	66	31	66	31
7:30	91	29	91	29
7:45	74	30	74	30
8:00	65	25	65	25
8:15	72	26	72	26
8:30	69	18	69	18
8:45	55	22	55	22
9:00	55	31	55	31
9:15	41	12	41	12
9:30	51	12	51	12
9:45	44	11	44	11
10:00	50	11	50	11
10:15	41	7	41	7
10:30	38	11	38	11
10:45	29	5	29	5
11:00	50	7	50	7
11:15	30	3	30	3
11:30	48	3	48	3
11:45	49	2	49	2
<b>Total</b>	<b>1596</b>	<b>1644</b>	<b>1596</b>	<b>1644</b>
<b>Day Total</b>	<b>3240</b>		<b>3240</b>	
<b>Split</b>	0.9708		0.9708	

AM Peak Hour 6:15 6:15  
 AM Peak Vol 339 339  
 AM PHF 0.8737 0.8737  
 PM Peak Hour 13:30 13:30  
 PM Peak Vol 238 238  
 PM PHF 0.8750 0.8750

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File Name: 0400127  
 Duration: 24  
 Site Code: 2  
 TRA ID: 0400127  
 Location 1: GLENDALE AVE BTWN SR-51 NB OFF RAMPS (T-SPLIT)  
 Count Type: VOL  
 Direction: EB  
 Latitude: 33.53826  
 Longitude: -112.04119

Date	2/3/2004		Average	
	AM	PM	AM	PM
12:00	18	303	18	303
12:15	16	341	16	341
12:30	15	345	15	345
12:45	6	385	6	385
1:00	15	326	15	326
1:15	18	316	18	316
1:30	8	297	8	297
1:45	10	302	10	302
2:00	2	337	2	337
2:15	4	340	4	340
2:30	6	342	6	342
2:45	5	347	5	347
3:00	7	339	7	339
3:15	5	368	5	368
3:30	9	379	9	379
3:45	11	397	11	397
4:00	14	393	14	393
4:15	53	403	53	403
4:30	58	404	58	404
4:45	71	406	71	406
5:00	66	417	66	417
5:15	99	427	99	427
5:30	151	413	151	413
5:45	254	437	254	437
6:00	232	343	232	343
6:15	386	308	386	308
6:30	480	330	480	330
6:45	693	307	693	307
7:00	721	218	721	218
7:15	814	185	814	185
7:30	849	181	849	181
7:45	801	184	801	184
8:00	838	162	838	162
8:15	683	156	683	156
8:30	720	142	720	142
8:45	736	137	736	137
9:00	631	125	631	125
9:15	531	117	531	117
9:30	407	112	407	112
9:45	340	102	340	102
10:00	353	107	353	107
10:15	304	74	304	74
10:30	321	83	321	83
10:45	261	63	261	63
11:00	259	44	259	44
11:15	324	42	324	42
11:30	340	27	340	27
11:45	297	29	297	29
<b>Total</b>	<b>13242</b>	<b>12342</b>	<b>13242</b>	<b>12342</b>
<b>Day Total</b>	<b>25584</b>		<b>25584</b>	
<b>Split</b>	1.0729		1.0729	

AM Peak Hour 7:15 7:15  
 AM Peak Vol 3302 3302  
 AM PHF 0.9723 0.9723  
 PM Peak Hour 17:00 17:00  
 PM Peak Vol 1694 1694  
 PM PHF 0.9691 0.9691

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 3844 East Indian School Road  
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File Name: 0400128  
 Duration: 24  
 Site Code: 2  
 TRA ID: 0400128  
 Location 1: GLENDALE AVE E. of SR-51 NB ON RAMP  
 Count Type: VOL  
 Direction: WB  
 Latitude: 33.53845  
 Longitude: -112.03926

Date	2/3/2004		Average	
	Time	AM	PM	AM
12:00	51	319	51	319
12:15	35	308	35	308
12:30	42	313	42	313
12:45	26	327	26	327
1:00	17	353	17	353
1:15	19	324	19	324
1:30	19	303	19	303
1:45	27	344	27	344
2:00	7	307	7	307
2:15	14	337	14	337
2:30	8	369	8	369
2:45	10	513	10	513
3:00	15	449	15	449
3:15	10	499	10	499
3:30	7	513	7	513
3:45	8	590	8	590
4:00	8	567	8	567
4:15	10	658	10	658
4:30	13	594	13	594
4:45	24	669	24	669
5:00	24	737	24	737
5:15	34	731	34	731
5:30	23	721	23	721
5:45	64	730	64	730
6:00	67	747	67	747
6:15	75	668	75	668
6:30	127	510	127	510
6:45	158	439	158	439
7:00	207	339	207	339
7:15	244	267	244	267
7:30	292	255	292	255
7:45	356	225	356	225
8:00	374	183	374	183
8:15	301	180	301	180
8:30	341	198	341	198
8:45	362	177	362	177
9:00	312	191	312	191
9:15	288	205	288	205
9:30	291	223	291	223
9:45	290	159	290	159
10:00	305	148	305	148
10:15	260	144	260	144
10:30	292	148	292	148
10:45	260	106	260	106
11:00	259	73	259	73
11:15	285	70	285	70
11:30	262	83	262	83
11:45	303	45	303	45
<b>Total</b>	<b>6826</b>	<b>17358</b>	<b>6826</b>	<b>17358</b>
<b>Day Total</b>	<b>24184</b>		<b>24184</b>	
<b>Split</b>	0.3932		0.3932	

AM Peak Hour 8:00 8:00  
 AM Peak Vol 1378 1378  
 AM PHF 0.9211 0.9211  
 PM Peak Hour 17:15 17:15  
 PM Peak Vol 2929 2929  
 PM PHF 0.9803 0.9803

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File Name: 0400129  
 Duration: 24  
 Site Code: 2  
 TRA ID: 0400129  
 Location 1: SR-51 SB OFF RAMP N. of GLENDALE AVE  
 Count Type: VOL  
 Direction: SB  
 Latitude: 33.54078  
 Longitude: -112.04140

Date	2/3/2004		Average	
	AM	PM	AM	PM
12:00	11	158	11	158
12:15	14	183	14	183
12:30	9	198	9	198
12:45	16	183	16	183
1:00	9	167	9	167
1:15	11	139	11	139
1:30	4	141	4	141
1:45	4	139	4	139
2:00	4	170	4	170
2:15	8	166	8	166
2:30	3	183	3	183
2:45	2	206	2	206
3:00	11	176	11	176
3:15	4	202	4	202
3:30	6	173	6	173
3:45	3	198	3	198
4:00	5	222	5	222
4:15	24	205	24	205
4:30	28	182	28	182
4:45	28	206	28	206
5:00	29	206	29	206
5:15	43	181	43	181
5:30	59	216	59	216
5:45	111	193	111	193
6:00	110	162	110	162
6:15	218	176	218	176
6:30	243	168	243	168
6:45	411	157	411	157
7:00	429	110	429	110
7:15	445	118	445	118
7:30	367	116	367	116
7:45	406	77	406	77
8:00	400	93	400	93
8:15	399	103	399	103
8:30	401	76	401	76
8:45	403	76	403	76
9:00	465	77	465	77
9:15	439	83	439	83
9:30	247	92	247	92
9:45	226	59	226	59
10:00	167	73	167	73
10:15	154	62	154	62
10:30	179	52	179	52
10:45	139	37	139	37
11:00	145	33	145	33
11:15	149	28	149	28
11:30	177	21	177	21
11:45	160	31	160	31
<b>Total</b>	<b>7325</b>	<b>6473</b>	<b>7325</b>	<b>6473</b>
<b>Day Total</b>	<b>13798</b>		<b>13798</b>	
<b>Split</b>	1.1316		1.1316	

AM Peak Hour 8:30 8:30  
 AM Peak Vol 1708 1708  
 AM PHF 0.9183 0.9183  
 PM Peak Hour 16:00 16:00  
 PM Peak Vol 815 815  
 PM PHF 0.9178 0.9178

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File Name: 0400130  
 Duration: 24  
 Site Code: 2  
 TRA ID: 0400130  
 Location 1: SR-51 SB OFF RAMP-RT TURNS N. of GLENDALE AVE  
 Count Type: VOL  
 Direction: SB  
 Latitude: 33.53866  
 Longitude: -112.04196

Date Time	2/3/2004		Average	
	AM	PM	AM	PM
12:00	3	53	3	53
12:15	9	60	9	60
12:30	1	86	1	86
12:45	9	56	9	56
1:00	7	58	7	58
1:15	4	47	4	47
1:30	4	56	4	56
1:45	3	66	3	66
2:00	3	54	3	54
2:15	6	60	6	60
2:30	3	75	3	75
2:45	1	102	1	102
3:00	1	77	1	77
3:15	0	68	0	68
3:30	1	69	1	69
3:45	1	74	1	74
4:00	1	73	1	73
4:15	5	86	5	86
4:30	4	63	4	63
4:45	10	68	10	68
5:00	5	77	5	77
5:15	12	60	12	60
5:30	8	89	8	89
5:45	14	84	14	84
6:00	19	73	19	73
6:15	17	85	17	85
6:30	0	67	0	67
6:45	20	73	20	73
7:00	97	45	97	45
7:15	95	50	95	50
7:30	89	54	89	54
7:45	76	27	76	27
8:00	78	28	78	28
8:15	75	45	75	45
8:30	95	37	95	37
8:45	86	35	86	35
9:00	116	28	116	28
9:15	165	35	165	35
9:30	84	40	84	40
9:45	71	19	71	19
10:00	51	30	51	30
10:15	47	23	47	23
10:30	61	23	61	23
10:45	64	18	64	18
11:00	66	18	66	18
11:15	56	13	56	13
11:30	55	9	55	9
11:45	68	15	68	15
<b>Total</b>	<b>1766</b>	<b>2551</b>	<b>1766</b>	<b>2551</b>
<b>Day Total</b>	<b>4317</b>		<b>4317</b>	
<b>Split</b>	0.6923		0.6923	

AM Peak Hour 8:30 8:30  
 AM Peak Vol 462 462  
 AM PHF 0.7000 0.7000  
 PM Peak Hour 17:30 17:30  
 PM Peak Vol 331 331  
 PM PHF 0.9298 0.9298

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File Name: 0400131  
 Duration: 24  
 Site Code: 2  
 TRA ID: 0400131  
 Location 1: GLENDALE AVE BTWN SR-51 SB OFF RAMPS (T-SPLIT)  
 Count Type: VOL  
 Direction: WB  
 Latitude: 33.53835  
 Longitude: -112.04198

Date	2/3/2004		Average	
	Time	AM	PM	AM
12:00	35	292	35	292
12:15	33	282	33	282
12:30	24	306	24	306
12:45	25	289	25	289
1:00	18	344	18	344
1:15	24	349	24	349
1:30	28	340	28	340
1:45	11	304	11	304
2:00	10	325	10	325
2:15	6	298	6	298
2:30	17	445	17	445
2:45	19	383	19	383
3:00	11	398	11	398
3:15	9	426	9	426
3:30	9	420	9	420
3:45	6	463	6	463
4:00	9	462	9	462
4:15	13	412	13	412
4:30	30	478	30	478
4:45	23	476	23	476
5:00	39	459	39	459
5:15	27	497	27	497
5:30	46	466	46	466
5:45	99	469	99	469
6:00	91	407	91	407
6:15	140	358	140	358
6:30	174	352	174	352
6:45	212	337	212	337
7:00	245	227	245	227
7:15	296	252	296	252
7:30	358	205	358	205
7:45	409	185	409	185
8:00	314	171	314	171
8:15	338	168	338	168
8:30	305	161	305	161
8:45	315	132	315	132
9:00	285	166	285	166
9:15	281	212	281	212
9:30	264	154	264	154
9:45	275	148	275	148
10:00	246	116	246	116
10:15	239	106	239	106
10:30	235	100	235	100
10:45	225	77	225	77
11:00	288	69	288	69
11:15	263	67	263	67
11:30	286	49	286	49
11:45	311	45	311	45
<b>Total</b>	<b>6966</b>	<b>13647</b>	<b>6966</b>	<b>13647</b>
<b>Day Total</b>	<b>20613</b>		<b>20613</b>	
<b>Split</b>	0.5104		0.5104	

AM Peak Hour 7:30 7:30  
 AM Peak Vol 1419 1419  
 AM PHF 0.8674 0.8674  
 PM Peak Hour 16:30 16:30  
 PM Peak Vol 1910 1910  
 PM PHF 0.9608 0.9608

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File Name: 0400132  
 Duration: 24  
 Site Code: 2  
 TRA ID: 0400132  
 Location 1: GLENDALE AVE W. of SR-51 SB ON RAMP  
 Count Type: VOL  
 Direction: EB  
 Latitude: 33.53816  
 Longitude: -112.04301

Date	2/3/2004		Average	
	AM	PM	AM	PM
12:00	13	362	13	362
12:15	4	340	4	340
12:30	10	363	10	363
12:45	8	364	8	364
1:00	6	429	6	429
1:15	5	395	5	395
1:30	9	455	9	455
1:45	11	419	11	419
2:00	26	441	26	441
2:15	40	476	40	476
2:30	47	457	47	457
2:45	69	497	69	497
3:00	69	519	69	519
3:15	117	538	117	538
3:30	178	483	178	483
3:45	201	501	201	501
4:00	218	426	218	426
4:15	297	383	297	383
4:30	449	382	449	382
4:45	478	302	478	302
5:00	464	263	464	263
5:15	566	201	566	201
5:30	605	198	605	198
5:45	529	170	529	170
6:00	531	179	531	179
6:15	524	164	524	164
6:30	524	176	524	176
6:45	487	188	487	188
7:00	395	150	395	150
7:15	375	124	375	124
7:30	332	101	332	101
7:45	349	78	349	78
8:00	331	96	331	96
8:15	311	77	311	77
8:30	339	94	339	94
8:45	279	72	279	72
9:00	313	49	313	49
9:15	350	57	350	57
9:30	320	41	320	41
9:45	327	37	327	37
10:00	346	31	346	31
10:15	304	38	304	38
10:30	368	24	368	24
10:45	358	24	358	24
11:00	352	12	352	12
11:15	329	26	329	26
11:30	370	17	370	17
11:45	329	15	329	15
<b>Total</b>	<b>13262</b>	<b>11234</b>	<b>13262</b>	<b>11234</b>
<b>Day Total</b>	<b>24496</b>		<b>24496</b>	
<b>Split</b>	1.1805		1.1805	

AM Peak Hour 5:15 5:15  
 AM Peak Vol 2231 2231  
 AM PHF 0.9219 0.9219  
 PM Peak Hour 15:00 15:00  
 PM Peak Vol 2041 2041  
 PM PHF 0.9484 0.9484

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File Name	Location	Location2	Direction	Count	Start Date	Start Time	Avg Vol	AM PkHr	AM PkVol	AM PHF	PM PkHr	PM PkVol	PM PHF
0400141	SR-51 NB OFF RAMP	S. of CACTUS RD	NB	24	1/21/2004	0:00	11284	11:45	675	0.8789	17:00	999	0.9182
0400142	SR-51 NB OFF RAMP-RT TURNS	S. of CACTUS RD	NB	24	1/21/2004	0:00	6296	11:45	407	0.8479	17:45	576	0.9412
0400143	CACTUS RD	BTWN SR-51 NB OFF RAMP (T-SPLIT)	EB	24	1/21/2004	0:00	20258	7:30	1881	0.8644	16:45	1690	0.9431
0400144	CACTUS RD	E. of SR-51 NB ON RAMP	WB	24	1/21/2004	0:00	22398	7:00	1310	0.9411	16:45	2013	0.9051
0400145	SR-51 SB OFF RAMP	N. of CACTUS RD	SB	24	1/21/2004	0:00	9483	7:30	1029	0.8272	16:45	788	0.9752
0400146	SR-51 SB OFF RAMP-RT TURNS	N. of CACTUS RD	SB	24	1/21/2004	0:00	4475	7:30	505	0.8417	15:15	384	0.8205
0400147	CACTUS RD	BTWN SR-51 SB OFF RAMP (T-SPLIT)	WB	24	1/21/2004	0:00	17505	11:45	1011	0.9258	16:45	1646	0.9395
0400148	CACTUS RD	W. of SR-51 SB ON RAMP	EB	24	1/21/2004	0:00	23799	7:15	1934	0.9536	17:00	1963	0.9474

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 3844 East Indian School Road  
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File Name: 0400141  
 Duration: 24  
 Site Code: 4  
 TRA ID: 0400141  
 Location 1: SR-51 NB OFF RAMP S. of CACTUS RD  
 Count Type: VOL  
 Direction: NB  
 Latitude: 33.59393  
 Longitude: -112.00919

Date	1/21/2004		Average	
	AM	PM	AM	PM
12:00	26	151	26	151
12:15	25	192	25	192
12:30	15	153	15	153
12:45	20	199	20	199
1:00	12	135	12	135
1:15	19	137	19	137
1:30	10	192	10	192
1:45	16	211	16	211
2:00	6	221	6	221
2:15	10	205	10	205
2:30	6	227	6	227
2:45	7	213	7	213
3:00	6	221	6	221
3:15	7	239	7	239
3:30	4	200	4	200
3:45	5	233	5	233
4:00	10	273	10	273
4:15	5	240	5	240
4:30	10	218	10	218
4:45	28	240	28	240
5:00	16	272	16	272
5:15	21	242	21	242
5:30	27	243	27	243
5:45	55	242	55	242
6:00	48	216	48	216
6:15	68	240	68	240
6:30	92	266	92	266
6:45	123	207	123	207
7:00	102	176	102	176
7:15	130	148	130	148
7:30	136	119	136	119
7:45	160	132	160	132
8:00	121	116	121	116
8:15	159	112	159	112
8:30	144	98	144	98
8:45	143	98	143	98
9:00	135	73	135	73
9:15	142	96	142	96
9:30	151	86	151	86
9:45	134	62	134	62
10:00	122	73	122	73
10:15	135	67	135	67
10:30	138	72	138	72
10:45	155	41	155	41
11:00	144	32	144	32
11:15	167	38	167	38
11:30	149	40	149	40
11:45	179	34	179	34
<b>Total</b>	<b>3543</b>	<b>7741</b>	<b>3543</b>	<b>7741</b>
<b>Day Total</b>	<b>11284</b>		<b>11284</b>	
<b>Split</b>	0.4577		0.4577	

AM Peak Hour	11:45	11:45
AM Peak Vol	675	675
AM PHF	0.8789	0.8789
PM Peak Hour	17:00	17:00
PM Peak Vol	999	999
PM PHF	0.9182	0.9182

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File Name: 0400142  
 Duration: 24  
 Site Code: 4  
 TRA ID: 0400142  
 Location 1: SR-51 NB OFF RAMP-RT TURNS S. of CACTUS RD  
 Count Type: VOL  
 Direction: NB  
 Latitude: 33.59666  
 Longitude: -112.00895

Date	1/21/2004		Average	
	AM	PM	AM	PM
12:00	7	76	7	76
12:15	13	110	13	110
12:30	5	101	5	101
12:45	6	109	6	109
1:00	5	66	5	66
1:15	8	76	8	76
1:30	7	102	7	102
1:45	8	106	8	106
2:00	2	119	2	119
2:15	8	121	8	121
2:30	2	121	2	121
2:45	3	107	3	107
3:00	2	127	2	127
3:15	3	124	3	124
3:30	3	112	3	112
3:45	0	119	0	119
4:00	2	131	2	131
4:15	5	135	5	135
4:30	5	123	5	123
4:45	17	125	17	125
5:00	10	151	10	151
5:15	15	144	15	144
5:30	19	137	19	137
5:45	40	142	40	142
6:00	31	129	31	129
6:15	47	152	47	152
6:30	54	153	54	153
6:45	85	90	85	90
7:00	62	90	62	90
7:15	90	84	90	84
7:30	79	57	79	57
7:45	105	71	105	71
8:00	75	48	75	48
8:15	100	51	100	51
8:30	91	52	91	52
8:45	95	54	95	54
9:00	82	35	82	35
9:15	74	36	74	36
9:30	86	40	86	40
9:45	74	29	74	29
10:00	70	41	70	41
10:15	78	32	78	32
10:30	88	35	88	35
10:45	93	20	93	20
11:00	79	16	79	16
11:15	107	16	107	16
11:30	93	9	93	9
11:45	120	19	120	19
<b>Total</b>	<b>2153</b>	<b>4143</b>	<b>2153</b>	<b>4143</b>
<b>Day Total</b>	<b>6296</b>		<b>6296</b>	
<b>Split</b>	0.5197		0.5197	

AM Peak Hour 11:45 11:45  
 AM Peak Vol 407 407  
 AM PHF 0.8479 0.8479  
 PM Peak Hour 17:45 17:45  
 PM Peak Vol 576 576  
 PM PHF 0.9412 0.9412

Traffic Research and Analysis, Inc.  
 3844 East Indian School Road  
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File Name: 0400143  
 Duration: 24  
 Site Code: 4  
 TRA ID: 0400143  
 Location 1: CACTUS RD BTWN SR-51 NB OFF RAMPS (T-SPLIT)  
 Count Type: VOL  
 Direction: EB  
 Latitude: 33.59691  
 Longitude: -112.00904

Date Time	1/21/2004		Average	
	AM	PM	AM	PM
12:00	27	295	27	295
12:15	26	273	26	273
12:30	19	311	19	311
12:45	27	290	27	290
1:00	21	274	21	274
1:15	21	242	21	242
1:30	20	299	20	299
1:45	16	292	16	292
2:00	12	285	12	285
2:15	11	308	11	308
2:30	10	341	10	341
2:45	9	332	9	332
3:00	9	307	9	307
3:15	16	322	16	322
3:30	11	333	11	333
3:45	9	345	9	345
4:00	13	354	13	354
4:15	13	355	13	355
4:30	30	355	30	355
4:45	56	448	56	448
5:00	40	393	40	393
5:15	70	443	70	443
5:30	96	406	96	406
5:45	129	431	129	431
6:00	149	363	149	363
6:15	175	319	175	319
6:30	269	309	269	309
6:45	312	321	312	321
7:00	319	270	319	270
7:15	375	211	375	211
7:30	461	157	461	157
7:45	544	173	544	173
8:00	434	168	434	168
8:15	442	153	442	153
8:30	389	178	389	178
8:45	389	134	389	134
9:00	269	170	269	170
9:15	285	139	285	139
9:30	296	144	296	144
9:45	313	112	313	112
10:00	274	127	274	127
10:15	256	91	256	91
10:30	227	66	227	66
10:45	292	54	292	54
11:00	308	43	308	43
11:15	307	38	307	38
11:30	277	49	277	49
11:45	339	23	339	23
<b>Total</b>	<b>8412</b>	<b>11846</b>	<b>8412</b>	<b>11846</b>
<b>Day Total</b>	<b>20258</b>		<b>20258</b>	
<b>Split</b>	0.7101		0.7101	

AM Peak Hour	7:30	7:30
AM Peak Vol	1881	1881
AM PHF	0.8644	0.8644
PM Peak Hour	16:45	16:45
PM Peak Vol	1690	1690
PM PHF	0.9431	0.9431

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File Name: 0400144  
 Duration: 24  
 Site Code: 4  
 TRA ID: 0400144  
 Location 1: CACTUS RD E. of SR-51 NB ON RAMP  
 Count Type: VOL  
 Direction: WB  
 Latitude: 33.59720  
 Longitude: -112.00584

Date	1/21/2004		Average	
	Time	AM	PM	AM
12:00	45	306	45	306
12:15	39	357	39	357
12:30	19	329	19	329
12:45	30	387	30	387
1:00	27	350	27	350
1:15	24	393	24	393
1:30	14	356	14	356
1:45	18	383	18	383
2:00	17	381	17	381
2:15	18	378	18	378
2:30	14	404	14	404
2:45	10	434	10	434
3:00	10	339	10	339
3:15	13	469	13	469
3:30	18	417	18	417
3:45	12	431	12	431
4:00	8	461	8	461
4:15	16	478	16	478
4:30	44	407	44	407
4:45	37	461	37	461
5:00	51	504	51	504
5:15	55	492	55	492
5:30	96	556	96	556
5:45	107	441	107	441
6:00	129	453	129	453
6:15	183	451	183	451
6:30	240	313	240	313
6:45	227	315	227	315
7:00	312	341	312	341
7:15	348	284	348	284
7:30	327	221	327	221
7:45	323	258	323	258
8:00	304	213	304	213
8:15	326	233	326	233
8:30	317	238	317	238
8:45	296	208	296	208
9:00	283	291	283	291
9:15	271	256	271	256
9:30	268	198	268	198
9:45	235	148	235	148
10:00	269	153	269	153
10:15	257	129	257	129
10:30	242	130	242	130
10:45	267	94	267	94
11:00	306	53	306	53
11:15	291	56	291	56
11:30	285	45	285	45
11:45	313	42	313	42
<b>Total</b>	<b>7361</b>	<b>15037</b>	<b>7361</b>	<b>15037</b>
<b>Day Total</b>	<b>22398</b>		<b>22398</b>	
<b>Split</b>	0.4895		0.4895	

AM Peak Hour 7:00 7:00  
 AM Peak Vol 1310 1310  
 AM PHF 0.9411 0.9411  
 PM Peak Hour 16:45 16:45  
 PM Peak Vol 2013 2013  
 PM PHF 0.9051 0.9051

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File Name: 0400145  
 Duration: 24  
 Site Code: 4  
 TRA ID: 0400145  
 Location 1: SR-51 SB OFF RAMPS N. of CACTUS RD  
 Count Type: VOL  
 Direction: SB  
 Latitude: 33.60019  
 Longitude: -112.00975

Date	1/21/2004		Average	
	Time	AM	PM	AM
12:00	13	144	13	144
12:15	18	130	18	130
12:30	22	129	22	129
12:45	11	116	11	116
1:00	8	132	8	132
1:15	11	123	11	123
1:30	10	120	10	120
1:45	7	154	7	154
2:00	5	138	5	138
2:15	8	162	8	162
2:30	3	148	3	148
2:45	6	165	6	165
3:00	5	141	5	141
3:15	12	175	12	175
3:30	4	184	4	184
3:45	5	202	5	202
4:00	6	142	6	142
4:15	6	181	6	181
4:30	14	159	14	159
4:45	17	202	17	202
5:00	26	187	26	187
5:15	21	199	21	199
5:30	28	200	28	200
5:45	66	176	66	176
6:00	48	166	48	166
6:15	54	150	54	150
6:30	59	129	59	129
6:45	94	149	94	149
7:00	129	117	129	117
7:15	180	101	180	101
7:30	263	93	263	93
7:45	311	92	311	92
8:00	177	82	177	82
8:15	278	90	278	90
8:30	199	74	199	74
8:45	139	78	139	78
9:00	137	78	137	78
9:15	110	67	110	67
9:30	98	72	98	72
9:45	145	73	145	73
10:00	112	56	112	56
10:15	110	62	110	62
10:30	123	44	123	44
10:45	141	29	141	29
11:00	118	39	118	39
11:15	129	32	129	32
11:30	120	27	120	27
11:45	155	13	155	13
<b>Total</b>	<b>3761</b>	<b>5722</b>	<b>3761</b>	<b>5722</b>
<b>Day Total</b>	<b>9483</b>		<b>9483</b>	
<b>Split</b>	0.6573		0.6573	

AM Peak Hour 7:30 7:30  
 AM Peak Vol 1029 1029  
 AM PHF 0.8272 0.8272  
 PM Peak Hour 16:45 16:45  
 PM Peak Vol 788 788  
 PM PHF 0.9752 0.9752

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File Name: 0400146  
 Duration: 24  
 Site Code: 4  
 TRA ID: 0400146  
 Location 1: SR-51 SB OFF RAMPS-RT TURNS N. of CACTUS RD  
 Count Type: VOL  
 Direction: SB  
 Latitude: 33.59747  
 Longitude: -112.01011

Date Time	1/21/2004		Average	
	AM	PM	AM	PM
12:00	8	69	8	69
12:15	11	69	11	69
12:30	13	47	13	47
12:45	6	44	6	44
1:00	2	60	2	60
1:15	8	52	8	52
1:30	6	47	6	47
1:45	3	75	3	75
2:00	3	70	3	70
2:15	6	88	6	88
2:30	1	77	1	77
2:45	5	89	5	89
3:00	4	59	4	59
3:15	5	95	5	95
3:30	3	103	3	103
3:45	2	117	2	117
4:00	3	69	3	69
4:15	3	82	3	82
4:30	7	76	7	76
4:45	7	76	7	76
5:00	11	90	11	90
5:15	11	117	11	117
5:30	17	85	17	85
5:45	31	72	31	72
6:00	17	81	17	81
6:15	31	71	31	71
6:30	27	65	27	65
6:45	46	62	46	62
7:00	62	51	62	51
7:15	110	41	110	41
7:30	145	44	145	44
7:45	150	31	150	31
8:00	78	37	78	37
8:15	132	39	132	39
8:30	91	31	91	31
8:45	44	36	44	36
9:00	48	34	48	34
9:15	52	31	52	31
9:30	36	37	36	37
9:45	49	35	49	35
10:00	54	33	54	33
10:15	53	32	53	32
10:30	59	28	59	28
10:45	53	16	53	16
11:00	53	24	53	24
11:15	59	25	59	25
11:30	60	10	60	10
11:45	59	9	59	9
<b>Total</b>	<b>1744</b>	<b>2731</b>	<b>1744</b>	<b>2731</b>
<b>Day Total</b>	<b>4475</b>		<b>4475</b>	
<b>Split</b>	0.6386		0.6386	

AM Peak Hour	7:30	7:30
AM Peak Vol	505	505
AM PHF	0.8417	0.8417
PM Peak Hour	15:15	15:15
PM Peak Vol	384	384
PM PHF	0.8205	0.8205

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File Name: 0400147  
 Duration: 24  
 Site Code: 4  
 TRA ID: 0400147  
 Location 1: CACTUS RD BTWN SR-51 SB OFF RAMPS (T-SPLIT)  
 Count Type: VOL  
 Direction: WB  
 Latitude: 33.59705  
 Longitude: -112.01020

Date	1/21/2004		Average	
	Time	AM	PM	AM
12:00	44	235	44	235
12:15	39	265	39	265
12:30	16	273	16	273
12:45	34	274	34	274
1:00	30	274	30	274
1:15	21	285	21	285
1:30	16	300	16	300
1:45	21	311	21	311
2:00	10	305	10	305
2:15	16	294	16	294
2:30	11	346	11	346
2:45	8	348	8	348
3:00	14	299	14	299
3:15	7	368	7	368
3:30	8	348	8	348
3:45	14	369	14	369
4:00	11	463	11	463
4:15	6	372	6	372
4:30	19	393	19	393
4:45	28	362	28	362
5:00	27	438	27	438
5:15	30	417	30	417
5:30	46	429	46	429
5:45	57	349	57	349
6:00	75	358	75	358
6:15	92	327	92	327
6:30	122	301	122	301
6:45	146	297	146	297
7:00	165	292	165	292
7:15	207	234	207	234
7:30	212	193	212	193
7:45	221	194	221	194
8:00	227	196	227	196
8:15	190	191	190	191
8:30	222	174	222	174
8:45	214	158	214	158
9:00	209	214	209	214
9:15	190	199	190	199
9:30	208	175	208	175
9:45	184	124	184	124
10:00	190	118	190	118
10:15	210	117	210	117
10:30	176	113	176	113
10:45	217	91	217	91
11:00	231	51	231	51
11:15	222	54	222	54
11:30	214	55	214	55
11:45	238	47	238	47
<b>Total</b>	<b>5115</b>	<b>12390</b>	<b>5115</b>	<b>12390</b>
<b>Day Total</b>	<b>17505</b>		<b>17505</b>	
<b>Split</b>	0.4128		0.4128	

AM Peak Hour 11:45 11:45  
 AM Peak Vol 1011 1011  
 AM PHF 0.9258 0.9258  
 PM Peak Hour 16:45 16:45  
 PM Peak Vol 1646 1646  
 PM PHF 0.9395 0.9395

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File Name: 0400148  
 Duration: 24  
 Site Code: 4  
 TRA ID: 0400148  
 Location 1: CACTUS RD W. of SR-51 SB ON RAMP  
 Count Type: VOL  
 Direction: EB  
 Latitude: 33.59685  
 Longitude: -112.01150

Date	1/21/2004		Average	
	AM	PM	AM	PM
12:00	34	363	34	363
12:15	25	351	25	351
12:30	19	366	19	366
12:45	27	340	27	340
1:00	22	295	22	295
1:15	30	294	30	294
1:30	25	386	25	386
1:45	15	358	15	358
2:00	17	346	17	346
2:15	17	388	17	388
2:30	14	459	14	459
2:45	5	395	5	395
3:00	10	373	10	373
3:15	19	377	19	377
3:30	26	363	26	363
3:45	19	386	19	386
4:00	27	407	27	407
4:15	24	411	24	411
4:30	70	429	70	429
4:45	78	462	78	462
5:00	53	471	53	471
5:15	117	518	117	518
5:30	177	463	177	463
5:45	183	511	183	511
6:00	251	424	251	424
6:15	282	397	282	397
6:30	446	356	446	356
6:45	415	342	415	342
7:00	415	269	415	269
7:15	473	228	473	228
7:30	484	194	484	194
7:45	507	171	507	171
8:00	470	175	470	175
8:15	439	178	439	178
8:30	430	173	430	173
8:45	406	165	406	165
9:00	324	184	324	184
9:15	320	137	320	137
9:30	361	157	361	157
9:45	324	110	324	110
10:00	312	141	312	141
10:15	294	100	294	100
10:30	302	81	302	81
10:45	335	70	335	70
11:00	341	43	341	43
11:15	350	46	350	46
11:30	362	46	362	46
11:45	367	37	367	37
<b>Total</b>	<b>10063</b>	<b>13736</b>	<b>10063</b>	<b>13736</b>
<b>Day Total</b>	<b>23799</b>		<b>23799</b>	
<b>Split</b>	0.7326		0.7326	

AM Peak Hour	7:15	7:15
AM Peak Vol	1934	1934
AM PHF	0.9536	0.9536
PM Peak Hour	17:00	17:00
PM Peak Vol	1963	1963
PM PHF	0.9474	0.9474

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File Name	Location	Location2	Direction	Count	Start Date	Start Time	Avg Vol	AM PKHr	AM PKVol	AM PHF	PM PKHr	PM PKVol	PM PHF
0400149	SR-51 NB OFF RAMP	S. of GREENWAY RD	NB	24	1/27/2004	0:00	12831	7:30	739	0.9192	15:45	1205	0.8783
0400150	SR-51 NB OFF RAMP-RT TURNS	S. of GREENWAY RD	NB	24	1/27/2004	0:00	6618	6:30	479	0.8146	17:15	598	0.9061
0400151	GREENWAY RD	BTWN SR-51 NB OFF RAMPS (T-SPLIT)	EB	24	1/27/2004	0:00	19866	7:15	2090	0.8782	16:30	1486	0.9751
0400152	GREENWAY RD	E. of SR-51 NB ON RAMP	WB	24	1/27/2004	0:00	28811	8:00	1961	0.9096	16:45	3104	0.9545
0400153	SR-51 SB OFF RAMPS	N. of GREENWAY RD	SB	24	1/27/2004	0:00	7926	7:30	460	0.8647	17:00	799	0.9248
0400154	SR-51 SB OFF RAMPS-RT TURNS	N. of GREENWAY RD	SB	24	1/27/2004	0:00	3291	11:15	185	0.8565	16:30	352	0.8302
0400155	GREENWAY RD	BTWN SR-51 SB OFF RAMPS (T-SPLIT)	WB	24	1/27/2004	0:00	22790	11:45	1305	0.9512	16:30	2634	0.9828
0400156	GREENWAY RD	W. of SR-51 SB ON RAMP	EB	24	1/27/2004	0:00	20235	7:00	2026	0.9380	16:00	1226	0.9519

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File Name: 0400149  
 Duration: 24  
 Site Code: 5  
 TRA ID: 0400149  
 Location 1: SR-51 NB OFF RAMP S. of GREENWAY RD  
 Count Type: VOL  
 Direction: NB  
 Latitude: 33.62380  
 Longitude: -112.00748

Date	1/27/2004		Average	
	Time	AM	PM	AM
12:00	19	166	19	166
12:15	23	175	23	175
12:30	17	179	17	179
12:45	15	171	15	171
1:00	12	172	12	172
1:15	16	179	16	179
1:30	11	158	11	158
1:45	11	179	11	179
2:00	10	161	10	161
2:15	6	203	6	203
2:30	13	214	13	214
2:45	15	234	15	234
3:00	7	248	7	248
3:15	4	272	4	272
3:30	7	270	7	270
3:45	7	343	7	343
4:00	9	290	9	290
4:15	8	258	8	258
4:30	17	314	17	314
4:45	23	293	23	293
5:00	30	262	30	262
5:15	44	287	44	287
5:30	62	268	62	268
5:45	76	326	76	326
6:00	77	272	77	272
6:15	120	283	120	283
6:30	180	323	180	323
6:45	182	264	182	264
7:00	168	166	168	166
7:15	143	160	143	160
7:30	195	151	195	151
7:45	201	137	201	137
8:00	180	113	180	113
8:15	163	120	163	120
8:30	160	88	160	88
8:45	197	108	197	108
9:00	140	106	140	106
9:15	129	105	129	105
9:30	146	105	146	105
9:45	169	83	169	83
10:00	132	96	132	96
10:15	140	109	140	109
10:30	129	58	129	58
10:45	146	68	146	68
11:00	138	39	138	39
11:15	158	53	158	53
11:30	138	33	138	33
11:45	153	23	153	23
<b>Total</b>	<b>4146</b>	<b>8685</b>	<b>4146</b>	<b>8685</b>
<b>Day Total</b>	<b>12831</b>		<b>12831</b>	
<b>Split</b>	0.4774		0.4774	

AM Peak Hour	7:30	7:30
AM Peak Vol	739	739
AM PHF	0.9192	0.9192
PM Peak Hour	15:45	15:45
PM Peak Vol	1205	1205
PM PHF	0.8783	0.8783

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File Name: 0400150  
 Duration: 24  
 Site Code: 5  
 TRA ID: 0400150  
 Location 1: SR-51 NB OFF RAMP-RT TURNS S. of GREENWAY RD  
 Count Type: VOL  
 Direction: NB  
 Latitude: 33.62593  
 Longitude: -112.00630

Date	1/27/2004		Average	
	Time	AM	PM	AM
12:00	7	82	7	82
12:15	9	86	9	86
12:30	11	91	11	91
12:45	8	90	8	90
1:00	2	79	2	79
1:15	6	84	6	84
1:30	5	81	5	81
1:45	6	90	6	90
2:00	3	85	3	85
2:15	2	104	2	104
2:30	5	107	5	107
2:45	2	116	2	116
3:00	3	129	3	129
3:15	2	141	2	141
3:30	5	146	5	146
3:45	4	153	4	153
4:00	4	140	4	140
4:15	5	121	5	121
4:30	11	150	11	150
4:45	11	137	11	137
5:00	22	116	22	116
5:15	28	147	28	147
5:30	48	146	48	146
5:45	62	165	62	165
6:00	54	140	54	140
6:15	91	135	91	135
6:30	123	138	123	138
6:45	147	120	147	120
7:00	115	71	115	71
7:15	94	63	94	63
7:30	116	67	116	67
7:45	132	54	132	54
8:00	102	48	102	48
8:15	105	56	105	56
8:30	105	39	105	39
8:45	103	53	103	53
9:00	77	46	77	46
9:15	69	62	69	62
9:30	78	51	78	51
9:45	94	43	94	43
10:00	69	41	69	41
10:15	68	36	68	36
10:30	72	30	72	30
10:45	73	31	73	31
11:00	78	16	78	16
11:15	77	22	77	22
11:30	55	19	55	19
11:45	73	10	73	10
<b>Total</b>	<b>2441</b>	<b>4177</b>	<b>2441</b>	<b>4177</b>
<b>Day Total</b>	<b>6618</b>		<b>6618</b>	
<b>Split</b>	0.5844		0.5844	

AM Peak Hour	6:30	6:30
AM Peak Vol	479	479
AM PHF	0.8146	0.8146
PM Peak Hour	17:15	17:15
PM Peak Vol	598	598
PM PHF	0.9061	0.9061

Traffic Research and Analysis, Inc.  
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File Name: 0400151  
 Duration: 24  
 Site Code: 5  
 TRA ID: 0400151  
 Location 1: GREENWAY RD BTWN SR-51 NB OFF RAMPS (T-SPLIT)  
 Count Type: VOL  
 Direction: EB  
 Latitude: 33.62618  
 Longitude: -112.00613

Date Time	1/27/2004		Average	
	AM	PM	AM	PM
12:00	34	250	34	250
12:15	35	249	35	249
12:30	16	255	16	255
12:45	16	244	16	244
1:00	29	234	29	234
1:15	30	222	30	222
1:30	7	265	7	265
1:45	25	286	25	286
2:00	13	267	13	267
2:15	11	318	11	318
2:30	6	318	6	318
2:45	4	309	4	309
3:00	5	313	5	313
3:15	8	355	8	355
3:30	16	334	16	334
3:45	9	339	9	339
4:00	18	363	18	363
4:15	22	358	22	358
4:30	43	362	43	362
4:45	41	372	41	372
5:00	65	371	65	371
5:15	60	381	60	381
5:30	106	350	106	350
5:45	137	370	137	370
6:00	156	336	156	336
6:15	269	318	269	318
6:30	336	299	336	299
6:45	427	302	427	302
7:00	385	245	385	245
7:15	524	238	524	238
7:30	566	208	566	208
7:45	595	151	595	151
8:00	405	181	405	181
8:15	497	206	497	206
8:30	312	160	312	160
8:45	371	144	371	144
9:00	292	177	292	177
9:15	229	176	229	176
9:30	236	120	236	120
9:45	263	111	263	111
10:00	204	109	204	109
10:15	240	106	240	106
10:30	205	76	205	76
10:45	244	63	244	63
11:00	244	66	244	66
11:15	199	45	199	45
11:30	229	50	229	50
11:45	270	40	270	40
<b>Total</b>	<b>8454</b>	<b>11412</b>	<b>8454</b>	<b>11412</b>
<b>Day Total</b>	<b>19866</b>		<b>19866</b>	
<b>Split</b>	0.7408		0.7408	

AM Peak Hour 7:15 7:15  
 AM Peak Vol 2090 2090  
 AM PHF 0.8782 0.8782  
 PM Peak Hour 16:30 16:30  
 PM Peak Vol 1486 1486  
 PM PHF 0.9751 0.9751

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File Name: 0400152  
 Duration: 24  
 Site Code: 5  
 TRA ID: 0400152  
 Location 1: GREENWAY RD E. of SR-51 NB ON RAMP  
 Count Type: VOL  
 Direction: WB  
 Latitude: 33.62647  
 Longitude: -112.00517

Date	1/27/2004		Average	
	AM	PM	AM	PM
12:00	49	358	49	358
12:15	48	394	48	394
12:30	32	388	32	388
12:45	29	345	29	345
1:00	23	395	23	395
1:15	22	343	22	343
1:30	20	341	20	341
1:45	9	352	9	352
2:00	19	390	19	390
2:15	15	421	15	421
2:30	12	470	12	470
2:45	19	541	19	541
3:00	12	536	12	536
3:15	4	612	4	612
3:30	27	660	27	660
3:45	23	696	23	696
4:00	17	717	17	717
4:15	27	714	27	714
4:30	47	773	47	773
4:45	60	740	60	740
5:00	65	771	65	771
5:15	115	780	115	780
5:30	166	813	166	813
5:45	196	707	196	707
6:00	265	594	265	594
6:15	299	499	299	499
6:30	420	439	420	439
6:45	441	380	441	380
7:00	510	332	510	332
7:15	472	283	472	283
7:30	448	246	448	246
7:45	427	245	427	245
8:00	463	214	463	214
8:15	499	222	499	222
8:30	539	215	539	215
8:45	460	227	460	227
9:00	342	191	342	191
9:15	388	188	388	188
9:30	308	166	308	166
9:45	344	153	344	153
10:00	276	142	276	142
10:15	313	134	313	134
10:30	298	115	298	115
10:45	295	96	295	96
11:00	329	94	329	94
11:15	306	83	306	83
11:30	362	59	362	59
11:45	328	49	328	49
<b>Total</b>	<b>10188</b>	<b>18623</b>	<b>10188</b>	<b>18623</b>
<b>Day Total</b>	<b>28811</b>		<b>28811</b>	
<b>Split</b>	0.5471		0.5471	

AM Peak Hour 8:00 8:00  
 AM Peak Vol 1961 1961  
 AM PHF 0.9096 0.9096  
 PM Peak Hour 16:45 16:45  
 PM Peak Vol 3104 3104  
 PM PHF 0.9545 0.9545

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File Name: 0400153  
 Duration: 24  
 Site Code: 5  
 TRA ID: 0400153  
 Location 1: SR-51 SB OFF RAMPS N. of GREENWAY RD  
 Count Type: VOL  
 Direction: SB  
 Latitude: 33.62847  
 Longitude: -112.00625

Date	1/27/2004		Average	
	AM	PM	AM	PM
12:00	15	92	15	92
12:15	18	95	18	95
12:30	15	103	15	103
12:45	15	100	15	100
1:00	13	81	13	81
1:15	16	104	16	104
1:30	3	93	3	93
1:45	11	124	11	124
2:00	11	109	11	109
2:15	5	142	5	142
2:30	4	139	4	139
2:45	6	135	6	135
3:00	2	151	2	151
3:15	7	164	7	164
3:30	11	173	11	173
3:45	10	179	10	179
4:00	14	184	14	184
4:15	4	186	4	186
4:30	8	184	8	184
4:45	23	183	23	183
5:00	12	209	12	209
5:15	19	216	19	216
5:30	26	168	26	168
5:45	33	206	33	206
6:00	27	190	27	190
6:15	43	171	43	171
6:30	47	145	47	145
6:45	64	126	64	126
7:00	78	116	78	116
7:15	105	101	105	101
7:30	121	91	121	91
7:45	98	88	98	88
8:00	108	85	108	85
8:15	133	87	133	87
8:30	108	99	108	99
8:45	107	72	107	72
9:00	105	73	105	73
9:15	95	108	95	108
9:30	79	70	79	70
9:45	88	49	88	49
10:00	64	77	64	77
10:15	92	64	92	64
10:30	82	50	82	50
10:45	95	33	95	33
11:00	75	40	75	40
11:15	95	27	95	27
11:30	85	25	85	25
11:45	99	25	99	25
<b>Total</b>	<b>2394</b>	<b>5532</b>	<b>2394</b>	<b>5532</b>
<b>Day Total</b>	<b>7926</b>		<b>7926</b>	
<b>Split</b>	0.4328		0.4328	

AM Peak Hour	7:30	7:30
AM Peak Vol	460	460
AM PHF	0.8647	0.8647
PM Peak Hour	17:00	17:00
PM Peak Vol	799	799
PM PHF	0.9248	0.9248

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File Name: 0400154  
 Duration: 24  
 Site Code: 5  
 TRA ID: 0400154  
 Location 1: SR-51 SB OFF RAMPS-RT TURNS N. of GREENWAY RD  
 Count Type: VOL  
 Direction: SB  
 Latitude: 33.62677  
 Longitude: -112.00723

Date	1/27/2004		Average	
	AM	PM	AM	PM
12:00	6	43	6	43
12:15	5	43	5	43
12:30	9	43	9	43
12:45	5	44	5	44
1:00	1	33	1	33
1:15	4	49	4	49
1:30	0	49	0	49
1:45	1	50	1	50
2:00	3	40	3	40
2:15	4	67	4	67
2:30	3	58	3	58
2:45	3	60	3	60
3:00	2	59	2	59
3:15	1	77	1	77
3:30	4	81	4	81
3:45	4	85	4	85
4:00	4	83	4	83
4:15	1	87	1	87
4:30	5	82	5	82
4:45	10	77	10	77
5:00	8	87	8	87
5:15	9	106	9	106
5:30	13	64	13	64
5:45	16	83	16	83
6:00	10	84	10	84
6:15	15	74	15	74
6:30	11	51	11	51
6:45	14	62	14	62
7:00	22	50	22	50
7:15	43	36	43	36
7:30	35	40	35	40
7:45	38	35	38	35
8:00	39	29	39	29
8:15	37	31	37	31
8:30	43	37	43	37
8:45	41	34	41	34
9:00	34	22	34	22
9:15	46	34	46	34
9:30	31	33	31	33
9:45	42	16	42	16
10:00	31	23	31	23
10:15	33	27	33	27
10:30	40	14	40	14
10:45	47	14	47	14
11:00	38	13	38	13
11:15	54	14	54	14
11:30	37	6	37	6
11:45	51	9	51	9
<b>Total</b>	<b>953</b>	<b>2338</b>	<b>953</b>	<b>2338</b>
<b>Day Total</b>	<b>3291</b>		<b>3291</b>	
<b>Split</b>	0.4076		0.4076	

AM Peak Hour 11:15 11:15  
 AM Peak Vol 185 185  
 AM PHF 0.8565 0.8565  
 PM Peak Hour 16:30 16:30  
 PM Peak Vol 352 352  
 PM PHF 0.8302 0.8302

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File Name: 0400155  
 Duration: 24  
 Site Code: 5  
 TRA ID: 0400155  
 Location 1: GREENWAY RD BTWN SR-51 SB OFF RAMPS (T-SPLIT)  
 Count Type: VOL  
 Direction: WB  
 Latitude: 33.62648  
 Longitude: -112.00740

Date Time	1/27/2004		Average	
	AM	PM	AM	PM
12:00	31	336	31	336
12:15	42	343	42	343
12:30	24	329	24	329
12:45	19	294	19	294
1:00	18	328	18	328
1:15	25	315	25	315
1:30	20	294	20	294
1:45	16	312	16	312
2:00	16	328	16	328
2:15	16	359	16	359
2:30	16	419	16	419
2:45	20	461	20	461
3:00	10	431	10	431
3:15	4	528	4	528
3:30	13	477	13	477
3:45	20	585	20	585
4:00	12	563	12	563
4:15	14	611	14	611
4:30	23	670	23	670
4:45	38	662	38	662
5:00	40	641	40	641
5:15	67	661	67	661
5:30	69	653	69	653
5:45	93	596	93	596
6:00	108	521	108	521
6:15	136	467	136	467
6:30	155	453	155	453
6:45	177	404	177	404
7:00	207	322	207	322
7:15	247	301	247	301
7:30	299	248	299	248
7:45	262	248	262	248
8:00	278	212	278	212
8:15	279	204	279	204
8:30	267	187	267	187
8:45	316	188	316	188
9:00	220	196	220	196
9:15	266	170	266	170
9:30	210	167	210	167
9:45	265	137	265	137
10:00	219	131	219	131
10:15	245	172	245	172
10:30	245	103	245	103
10:45	242	93	242	93
11:00	273	92	273	92
11:15	250	67	250	67
11:30	287	57	287	57
11:45	297	38	297	38
<b>Total</b>	<b>6416</b>	<b>16374</b>	<b>6416</b>	<b>16374</b>
<b>Day Total</b>	<b>22790</b>		<b>22790</b>	
<b>Split</b>	0.3918		0.3918	

AM Peak Hour 11:45 11:45  
 AM Peak Vol 1305 1305  
 AM PHF 0.9512 0.9512  
 PM Peak Hour 16:30 16:30  
 PM Peak Vol 2634 2634  
 PM PHF 0.9828 0.9828

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File Name: 0400156  
 Duration: 24  
 Site Code: 5  
 TRA ID: 0400156  
 Location 1: GREENWAY RD W. of SR-51 SB ON RAMP  
 Count Type: VOL  
 Direction: EB  
 Latitude: 33.62633  
 Longitude: -112.00840

Date	1/27/2004		Average	
	Time	AM	PM	AM
12:00	26	282	26	282
12:15	31	304	31	304
12:30	18	278	18	278
12:45	21	273	21	273
1:00	21	283	21	283
1:15	23	248	23	248
1:30	16	295	16	295
1:45	17	310	17	310
2:00	15	272	15	272
2:15	16	301	16	301
2:30	9	305	9	305
2:45	10	301	10	301
3:00	14	270	14	270
3:15	13	302	13	302
3:30	16	305	16	305
3:45	14	286	14	286
4:00	18	301	18	301
4:15	37	296	37	296
4:30	59	322	59	322
4:45	53	307	53	307
5:00	83	288	83	288
5:15	86	307	86	307
5:30	182	299	182	299
5:45	207	307	207	307
6:00	221	288	221	288
6:15	362	285	362	285
6:30	456	239	456	239
6:45	502	270	502	270
7:00	454	231	454	231
7:15	507	211	507	211
7:30	525	183	525	183
7:45	540	155	540	155
8:00	440	174	440	174
8:15	466	190	466	190
8:30	367	159	367	159
8:45	409	140	409	140
9:00	312	177	312	177
9:15	306	149	306	149
9:30	287	112	287	112
9:45	308	109	308	109
10:00	277	93	277	93
10:15	267	89	267	89
10:30	254	72	254	72
10:45	288	68	288	68
11:00	273	52	273	52
11:15	236	39	236	39
11:30	263	43	263	43
11:45	301	39	301	39
<b>Total</b>	<b>9626</b>	<b>10609</b>	<b>9626</b>	<b>10609</b>
<b>Day Total</b>	<b>20235</b>		<b>20235</b>	
<b>Split</b>	0.9073		0.9073	

AM Peak Hour	7:00	7:00
AM Peak Vol	2026	2026
AM PHF	0.9380	0.9380
PM Peak Hour	16:00	16:00
PM Peak Vol	1226	1226
PM PHF	0.9519	0.9519

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File Name	Location	Location2	Direction	Count Dur	Start Date	Start Time	Avg Vol	AM PKHr	AM PKVol	AM PHF	PM PKHr	PM PKVol	PM PHF
0400117	LOOP 101 (WEST) NB OFF RAMP	S. of BELL RD	NB	24	1/28/2004	12:00	16204	7:15	1123	0.8406	16:45	1367	0.9906
0400118	LOOP 101 (WEST) NB OFF RAMP-RT TURNS	S. of BELL RD	NB	24	1/28/2004	0:00	10097	11:45	813	0.9366	12:00	836	0.9048
0400119	BELL RD	BTWN LOOP 101(WEST) NB OFF RAMP (T-SPLIT)	EB	24	1/28/2004	0:00	22210	11:30	1953	0.9630	12:00	1851	0.9127
0400120	BELL RD	E. of LOOP 101 (WEST) NB ON RAMP	WB	24	1/28/2004	0:00	26977	11:45	1891	0.9197	15:00	2281	0.9348
0400121	LOOP 101 (WEST) SB OFF RAMP	N. of BELL RD	SB	24	1/28/2004	0:00	16867	7:15	1217	0.8405	16:45	1385	0.9699
0400122	LOOP 101 (WEST) SB OFF RAMP-RT TURNS	N. of BELL RD	SB	24	1/28/2004	0:00	12760	7:15	1067	0.8441	16:45	1022	0.9642
0400123	BELL RD	BTWN LOOP 101(WEST) SB OFF RAMP (T-SPLIT)	WB	24	1/28/2004	0:00	27065	11:45	1829	0.9447	15:00	2387	0.9281
0400124	BELL RD	W. of LOOP 101 (WEST) SB ON RAMP	EB	24	1/28/2004	0:00	37409	11:30	2750	0.9683	16:30	2768	0.9665

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File Name: 0400117  
 Duration: 24  
 Site Code: 1  
 TRA ID: 0400117  
 Location 1: LOOP 101 (WEST) NB OFF RAMP S. of BELL RD  
 Count Type: VOL  
 Direction: NB  
 Latitude: 33.63528  
 Longitude: -112.23822

Date Time	1/28/2004		1/29/2004		Average	
	AM	PM	AM	PM	AM	PM
12:00		256	23		23	256
12:15		302	24		24	302
12:30		272	20		20	272
12:45		317	14		14	317
1:00		231	14		14	231
1:15		256	20		20	256
1:30		285	15		15	285
1:45		290	12		12	290
2:00		254	11		11	254
2:15		247	14		14	247
2:30		256	12		12	256
2:45		314	17		17	314
3:00		298	20		20	298
3:15		305	5		5	305
3:30		310	16		16	310
3:45		339	20		20	339
4:00		276	5		5	276
4:15		330	22		22	330
4:30		333	28		28	333
4:45		342	29		29	342
5:00		345	30		30	345
5:15		338	40		40	338
5:30		342	50		50	342
5:45		335	96		96	335
6:00		286	96		96	286
6:15		266	120		120	266
6:30		269	159		159	269
6:45		254	268		268	254
7:00		193	207		207	193
7:15		187	243		243	187
7:30		170	303		303	170
7:45		124	334		334	124
8:00		141	243		243	141
8:15		117	222		222	117
8:30		91	253		253	91
8:45		98	281		281	98
9:00		90	214		214	90
9:15		81	192		192	81
9:30		72	237		237	72
9:45		89	281		281	89
10:00		55	214		214	55
10:15		56	227		227	56
10:30		50	211		211	50
10:45		45	288		288	45
11:00		24	257		257	24
11:15		33	268		268	33
11:30		28	231		231	28
11:45		32	274		274	32
<b>Total</b>	<b>0</b>	<b>10024</b>	<b>6180</b>	<b>0</b>	<b>6180</b>	<b>10024</b>
<b>Day Total</b>	<b>10024</b>		<b>6180</b>		<b>16204</b>	
<b>Split</b>	0.0000		0.0000		0.6165	

AM Peak Hour 7:15 7:15  
 AM Peak Vol 1123 1123  
 AM PHF 0.8406 0.8406  
 PM Peak Hour 16:45  
 PM Peak Vol 1367  
 PM PHF 0.9906

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File Name: 0400118  
 Duration: 24  
 Site Code: 1  
 TRA ID: 0400118  
 Location 1: LOOP 101 (WEST) NB OFF RAMP-RT TURNS S. of BELL RD  
 Count Type: VOL  
 Direction: NB  
 Latitude: 33.63777  
 Longitude: -112.23722

Date	1/28/2004		Average	
	Time	AM	PM	AM
12:00	12	189	12	189
12:15	13	217	13	217
12:30	19	199	19	199
12:45	13	231	13	231
1:00	6	172	6	172
1:15	10	191	10	191
1:30	3	187	3	187
1:45	12	175	12	175
2:00	3	182	3	182
2:15	5	172	5	172
2:30	12	166	12	166
2:45	12	197	12	197
3:00	6	168	6	168
3:15	8	205	8	205
3:30	4	170	4	170
3:45	10	232	10	232
4:00	7	166	7	166
4:15	10	193	10	193
4:30	15	191	15	191
4:45	18	230	18	230
5:00	17	195	17	195
5:15	27	199	27	199
5:30	31	195	31	195
5:45	57	201	57	201
6:00	41	185	41	185
6:15	58	178	58	178
6:30	82	181	82	181
6:45	129	149	129	149
7:00	111	133	111	133
7:15	136	131	136	131
7:30	133	131	133	131
7:45	182	81	182	81
8:00	123	95	123	95
8:15	118	70	118	70
8:30	112	55	112	55
8:45	129	66	129	66
9:00	125	49	125	49
9:15	141	48	141	48
9:30	147	38	147	38
9:45	175	55	175	55
10:00	142	30	142	30
10:15	148	24	148	24
10:30	179	30	179	30
10:45	188	20	188	20
11:00	135	10	135	10
11:15	209	20	209	20
11:30	183	15	183	15
11:45	208	16	208	16
<b>Total</b>	<b>3664</b>	<b>6433</b>	<b>3664</b>	<b>6433</b>
<b>Day Total</b>	<b>10097</b>		<b>10097</b>	
<b>Split</b>	0.5696		0.5696	

AM Peak Hour 11:45 11:45  
 AM Peak Vol 813 813  
 AM PHF 0.9366 0.9366  
 PM Peak Hour 12:00 12:00  
 PM Peak Vol 836 836  
 PM PHF 0.9048 0.9048

Traffic Research and Analysis, Inc.  
 3844 East Indian School Road  
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File Name: 0400119  
 Duration: 24  
 Site Code: 1  
 TRA ID: 0400119  
 Location 1: BELL RD BTWN LOOP 101(WEST) NB OFF RAMP (T-SPLIT)  
 Count Type: VOL  
 Direction: EB  
 Latitude: 33.63798  
 Longitude: -112.23710

Date Time	1/28/2004		Average	
	AM	PM	AM	PM
12:00	21	507	21	507
12:15	14	478	14	478
12:30	24	470	24	470
12:45	6	396	6	396
1:00	4	501	4	501
1:15	15	396	15	396
1:30	13	441	13	441
1:45	11	381	11	381
2:00	12	422	12	422
2:15	5	399	5	399
2:30	10	394	10	394
2:45	13	365	13	365
3:00	8	367	8	367
3:15	7	398	7	398
3:30	9	423	9	423
3:45	17	357	17	357
4:00	14	388	14	388
4:15	14	369	14	369
4:30	23	414	23	414
4:45	31	393	31	393
5:00	39	394	39	394
5:15	61	381	61	381
5:30	74	455	74	455
5:45	94	399	94	399
6:00	108	406	108	406
6:15	183	413	183	413
6:30	202	358	202	358
6:45	316	330	316	330
7:00	307	297	307	297
7:15	348	259	348	259
7:30	286	229	286	229
7:45	381	161	381	161
8:00	280	165	280	165
8:15	284	136	284	136
8:30	209	144	209	144
8:45	267	107	267	107
9:00	282	117	282	117
9:15	315	90	315	90
9:30	299	86	299	86
9:45	376	102	376	102
10:00	391	65	391	65
10:15	360	57	360	57
10:30	434	53	434	53
10:45	437	52	437	52
11:00	495	43	495	43
11:15	466	32	466	32
11:30	489	28	489	28
11:45	479	59	479	59
<b>Total</b>	<b>8533</b>	<b>13677</b>	<b>8533</b>	<b>13677</b>
<b>Day Total</b>	<b>22210</b>		<b>22210</b>	
<b>Split</b>	0.6239		0.6239	

AM Peak Hour 11:30 11:30  
 AM Peak Vol 1953 1953  
 AM PHF 0.9630 0.9630  
 PM Peak Hour 12:00 12:00  
 PM Peak Vol 1851 1851  
 PM PHF 0.9127 0.9127

Traffic Research and Analysis, Inc.  
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File Name: 0400120  
 Duration: 24  
 Site Code: 1  
 TRA ID: 0400120  
 Location 1: BELL RD E. of LOOP 101 (WEST) NB ON RAMP  
 Count Type: VOL  
 Direction: WB  
 Latitude: 33.63835  
 Longitude: -112.23475

Date	1/28/2004		Average	
	AM	PM	AM	PM
12:00	51	452	51	452
12:15	38	473	38	473
12:30	35	514	35	514
12:45	34	518	34	518
1:00	43	562	43	562
1:15	36	518	36	518
1:30	30	566	30	566
1:45	13	577	13	577
2:00	15	516	15	516
2:15	12	506	12	506
2:30	21	564	21	564
2:45	16	531	16	531
3:00	22	550	22	550
3:15	14	553	14	553
3:30	13	610	13	610
3:45	12	568	12	568
4:00	22	514	22	514
4:15	37	485	37	485
4:30	32	544	32	544
4:45	30	474	30	474
5:00	50	519	50	519
5:15	65	497	65	497
5:30	76	519	76	519
5:45	97	492	97	492
6:00	125	512	125	512
6:15	192	545	192	545
6:30	214	525	214	525
6:45	226	461	226	461
7:00	257	442	257	442
7:15	274	432	274	432
7:30	293	361	293	361
7:45	268	365	268	365
8:00	202	378	202	378
8:15	212	375	212	375
8:30	211	320	211	320
8:45	221	324	221	324
9:00	225	386	225	386
9:15	234	328	234	328
9:30	253	245	253	245
9:45	244	188	244	188
10:00	261	207	261	207
10:15	291	135	291	135
10:30	327	96	327	96
10:45	331	104	331	104
11:00	365	98	365	98
11:15	392	90	392	90
11:30	427	69	427	69
11:45	452	58	452	58
<b>Total</b>	<b>7311</b>	<b>19666</b>	<b>7311</b>	<b>19666</b>
<b>Day Total</b>	<b>26977</b>		<b>26977</b>	
<b>Split</b>	0.3718		0.3718	

AM Peak Hour 11:45 11:45  
 AM Peak Vol 1891 1891  
 AM PHF 0.9197 0.9197  
 PM Peak Hour 15:00 15:00  
 PM Peak Vol 2281 2281  
 PM PHF 0.9348 0.9348

Traffic Research and Analysis, Inc.  
 3844 East Indian School Road  
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File Name: 0400121  
 Duration: 24  
 Site Code: 1  
 TRA ID: 0400121  
 Location 1: LOOP 101 (WEST) SB OFF RAMP N. of BELL RD  
 Count Type: VOL  
 Direction: SB  
 Latitude: 33.64108  
 Longitude: -112.23692

Date	1/28/2004		Average	
	Time	AM	PM	AM
12:00	34	254	34	254
12:15	24	221	24	221
12:30	23	252	23	252
12:45	15	233	15	233
1:00	17	225	17	225
1:15	16	243	16	243
1:30	13	232	13	232
1:45	14	259	14	259
2:00	13	214	13	214
2:15	9	250	9	250
2:30	9	236	9	236
2:45	14	283	14	283
3:00	10	269	10	269
3:15	9	313	9	313
3:30	9	309	9	309
3:45	19	347	19	347
4:00	13	299	13	299
4:15	16	294	16	294
4:30	37	306	37	306
4:45	22	350	22	350
5:00	43	357	43	357
5:15	45	323	45	323
5:30	85	355	85	355
5:45	93	292	93	292
6:00	144	321	144	321
6:15	203	340	203	340
6:30	279	283	279	283
6:45	288	266	288	266
7:00	254	200	254	200
7:15	263	202	263	202
7:30	309	170	309	170
7:45	362	162	362	162
8:00	283	188	283	188
8:15	263	171	263	171
8:30	253	156	253	156
8:45	277	137	277	137
9:00	236	138	236	138
9:15	246	127	246	127
9:30	246	125	246	125
9:45	240	113	240	113
10:00	225	99	225	99
10:15	217	67	217	67
10:30	219	69	219	69
10:45	229	88	229	88
11:00	237	48	237	48
11:15	208	61	208	61
11:30	224	50	224	50
11:45	225	38	225	38
<b>Total</b>	<b>6532</b>	<b>10335</b>	<b>6532</b>	<b>10335</b>
<b>Day Total</b>	<b>16867</b>		<b>16867</b>	
<b>Split</b>	0.6320		0.6320	

AM Peak Hour	7:15	7:15
AM Peak Vol	1217	1217
AM PHF	0.8405	0.8405
PM Peak Hour	16:45	16:45
PM Peak Vol	1385	1385
PM PHF	0.9699	0.9699

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File Name: 0400122  
 Duration: 24  
 Site Code: 1  
 TRA ID: 0400122  
 Location 1: LOOP 101 (WEST) SB OFF RAMP-RT TURNS N. of BELL RD  
 Count Type: VOL  
 Direction: SB  
 Latitude: 33.63867  
 Longitude: -112.23820

Date	1/28/2004		Average	
	AM	PM	AM	PM
12:00	27	167	27	167
12:15	17	160	17	160
12:30	19	177	19	177
12:45	12	173	12	173
1:00	15	158	15	158
1:15	13	173	13	173
1:30	9	165	9	165
1:45	11	187	11	187
2:00	12	164	12	164
2:15	9	176	9	176
2:30	7	173	7	173
2:45	12	212	12	212
3:00	9	200	9	200
3:15	7	222	7	222
3:30	6	220	6	220
3:45	17	244	17	244
4:00	11	222	11	222
4:15	15	235	15	235
4:30	32	222	32	222
4:45	19	265	19	265
5:00	33	265	33	265
5:15	40	244	40	244
5:30	63	248	63	248
5:45	80	227	80	227
6:00	125	226	125	226
6:15	181	240	181	240
6:30	253	203	253	203
6:45	245	174	245	174
7:00	213	126	213	126
7:15	225	121	225	121
7:30	279	120	279	120
7:45	316	120	316	120
8:00	247	138	247	138
8:15	215	132	215	132
8:30	212	110	212	110
8:45	214	103	214	103
9:00	182	106	182	106
9:15	195	109	195	109
9:30	193	103	193	103
9:45	173	90	173	90
10:00	164	83	164	83
10:15	155	56	155	56
10:30	154	56	154	56
10:45	165	72	165	72
11:00	154	39	154	39
11:15	141	49	141	49
11:30	170	42	170	42
11:45	145	32	145	32

<b>Total</b>	<b>5211</b>	<b>7549</b>	<b>5211</b>	<b>7549</b>
<b>Day Total</b>	<b>12760</b>		<b>12760</b>	
<b>Split</b>	0.6903		0.6903	

AM Peak Hour 7:15 7:15  
 AM Peak Vol 1067 1067  
 AM PHF 0.8441 0.8441  
 PM Peak Hour 16:45 16:45  
 PM Peak Vol 1022 1022  
 PM PHF 0.9642 0.9642

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File Name: 0400123  
 Duration: 24  
 Site Code: 1  
 TRA ID: 0400123  
 Location 1: BELL RD BTWN LOOP 101(WEST) SB OFF RAMP (T-SPLIT)  
 Count Type: VOL  
 Direction: WB  
 Latitude: 33.63833  
 Longitude: -112.23837

Date	1/28/2004		Average	
	AM	PM	AM	PM
12:00	55	440	55	440
12:15	34	469	34	469
12:30	32	484	32	484
12:45	23	544	23	544
1:00	31	503	31	503
1:15	41	505	41	505
1:30	27	531	27	531
1:45	11	531	11	531
2:00	15	502	15	502
2:15	4	489	4	489
2:30	18	522	18	522
2:45	13	579	13	579
3:00	15	575	15	575
3:15	13	533	13	533
3:30	15	643	15	643
3:45	16	636	16	636
4:00	19	556	19	556
4:15	32	512	32	512
4:30	16	588	16	588
4:45	36	498	36	498
5:00	43	543	43	543
5:15	57	528	57	528
5:30	66	569	66	569
5:45	136	519	136	519
6:00	114	506	114	506
6:15	183	502	183	502
6:30	273	518	273	518
6:45	272	473	272	473
7:00	313	383	313	383
7:15	325	360	325	360
7:30	378	319	378	319
7:45	384	291	384	291
8:00	293	307	293	307
8:15	252	310	252	310
8:30	311	267	311	267
8:45	284	272	284	272
9:00	282	262	282	262
9:15	265	210	265	210
9:30	306	199	306	199
9:45	302	149	302	149
10:00	294	119	294	119
10:15	330	116	330	116
10:30	351	86	351	86
10:45	360	70	360	70
11:00	380	75	380	75
11:15	408	76	408	76
11:30	424	52	424	52
11:45	436	56	436	56
<b>Total</b>	<b>8288</b>	<b>18777</b>	<b>8288</b>	<b>18777</b>
<b>Day Total</b>	<b>27065</b>		<b>27065</b>	
<b>Split</b>	0.4414		0.4414	

AM Peak Hour 11:45 11:45  
 AM Peak Vol 1829 1829  
 AM PHF 0.9447 0.9447  
 PM Peak Hour 15:00 15:00  
 PM Peak Vol 2387 2387  
 PM PHF 0.9281 0.9281

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File Name: 0400124  
 Duration: 24  
 Site Code: 1  
 TRA ID: 0400124  
 Location 1: BELL RD W. of LOOP 101 (WEST) SB ON RAMP  
 Count Type: VOL  
 Direction: EB  
 Latitude: 33.63812  
 Longitude: -112.24095

Date	1/28/2004		Average	
	AM	PM	AM	PM
12:00	49	694	49	694
12:15	30	710	30	710
12:30	41	657	41	657
12:45	12	562	12	562
1:00	11	736	11	736
1:15	26	589	26	589
1:30	28	646	28	646
1:45	25	594	25	594
2:00	21	667	21	667
2:15	21	622	21	622
2:30	20	621	20	621
2:45	22	607	22	607
3:00	22	591	22	591
3:15	34	651	34	651
3:30	30	680	30	680
3:45	35	637	35	637
4:00	45	681	45	681
4:15	79	640	79	640
4:30	93	716	93	716
4:45	100	655	100	655
5:00	154	709	154	709
5:15	262	688	262	688
5:30	327	697	327	697
5:45	366	626	366	626
6:00	463	645	463	645
6:15	519	557	519	557
6:30	568	488	568	488
6:45	637	421	637	421
7:00	646	405	646	405
7:15	701	318	701	318
7:30	657	287	657	287
7:45	656	243	656	243
8:00	578	252	578	252
8:15	552	229	552	229
8:30	519	237	519	237
8:45	501	164	501	164
9:00	532	216	532	216
9:15	506	170	506	170
9:30	513	126	513	126
9:45	567	164	567	164
10:00	598	114	598	114
10:15	592	113	592	113
10:30	635	85	635	85
10:45	638	87	638	87
11:00	710	68	710	68
11:15	690	45	690	45
11:30	666	45	666	45
11:45	680	77	680	77
<b>Total</b>	<b>16177</b>	<b>21232</b>	<b>16177</b>	<b>21232</b>
<b>Day Total</b>	<b>37409</b>		<b>37409</b>	
<b>Split</b>	0.7619		0.7619	

AM Peak Hour	11:30	11:30
AM Peak Vol	2750	2750
AM PHF	0.9683	0.9683
PM Peak Hour	16:30	16:30
PM Peak Vol	2768	2768
PM PHF	0.9665	0.9665

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File Name	Location	Location2	Direction	Count	Start Date	Start Time	Avg Vol	AM PkHr	AM PKVol	AM PHF	PM PkHr	PM PKVol	PM PHF
0400157	LOOP 202 EB OFF RAMP	W. of SCOTTSDALE RD	EB	24	1/29/2004	0:00	15650	8:00	1282	0.9054	14:00	1004	0.9544
0400158	LOOP 202 EB OFF RAMP-RT TURNS	W. of SCOTTSDALE RD	EB	24	1/29/2004	0:00	8339	8:00	731	0.9277	12:30	495	0.8839
0400159	SCOTTSDALE RD	BTWN LOOP 202 EB OFF RAMP (T-SPLIT)	SB	24	1/29/2004	0:00	17389	8:00	1341	0.8730	17:00	1365	0.9004
0400160	SCOTTSDALE RD	S. of LOOP 202 EB ON RAMP	NB	24	1/29/2004	0:00	32116	11:45	2070	0.8697	15:00	2884	0.8945
0400161	LOOP 202 WB OFF RAMP	E. of SCOTTSDALE RD	WB	24	1/29/2004	0:00	11919	8:15	1214	0.8848	12:45	859	0.9177
0400162	LOOP 202 WB OFF RAMP-RT TURNS	E. of SCOTTSDALE RD	WB	24	1/29/2004	0:00	6026	9:00	580	0.8480	12:15	410	0.9579
0400163	SCOTTSDALE RD	BTWN LOOP 202 WB OFF RAMP (T-SPLIT)	NB	24	1/29/2004	0:00	24488	7:30	1778	0.9241	15:00	1776	0.9507
0400164	SCOTTSDALE RD	N. of LOOP 202 WB ON RAMP	SB	24	1/29/2004	0:00	27418	11:30	1732	0.9097	17:15	2380	0.9917

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File Name: 0400157  
 Duration: 24  
 Site Code: 6  
 TRA ID: 0400157  
 Location 1: LOOP 202 EB OFF RAMPS W. of SCOTTSDALE RD  
 Count Type: VOL  
 Direction: EB  
 Latitude: 33.43567  
 Longitude: -111.92925

Date	1/29/2004		Average	
	AM	PM	AM	PM
12:00	43	262	43	262
12:15	31	221	31	221
12:30	32	225	32	225
12:45	28	233	28	233
1:00	29	251	29	251
1:15	32	222	32	222
1:30	28	221	28	221
1:45	18	208	18	208
2:00	25	249	25	249
2:15	19	230	19	230
2:30	18	263	18	263
2:45	14	262	14	262
3:00	13	216	13	216
3:15	7	245	7	245
3:30	8	239	8	239
3:45	17	237	17	237
4:00	14	235	14	235
4:15	24	199	24	199
4:30	22	204	22	204
4:45	29	206	29	206
5:00	36	213	36	213
5:15	44	199	44	199
5:30	75	199	75	199
5:45	125	240	125	240
6:00	151	215	151	215
6:15	181	219	181	219
6:30	207	205	207	205
6:45	222	245	222	245
7:00	211	206	211	206
7:15	262	204	262	204
7:30	241	158	241	158
7:45	256	169	256	169
8:00	300	166	300	166
8:15	322	133	322	133
8:30	306	134	306	134
8:45	354	135	354	135
9:00	223	138	223	138
9:15	208	140	208	140
9:30	229	145	229	145
9:45	243	144	243	144
10:00	302	143	302	143
10:15	261	131	261	131
10:30	207	103	207	103
10:45	196	109	196	109
11:00	226	109	226	109
11:15	249	77	249	77
11:30	262	73	262	73
11:45	254	66	254	66
<b>Total</b>	<b>6604</b>	<b>9046</b>	<b>6604</b>	<b>9046</b>
<b>Day Total</b>	<b>15650</b>		<b>15650</b>	
<b>Split</b>	0.7300		0.7300	

AM Peak Hour 8:00 8:00  
 AM Peak Vol 1282 1282  
 AM PHF 0.9054 0.9054  
 PM Peak Hour 14:00 14:00  
 PM Peak Vol 1004 1004  
 PM PHF 0.9544 0.9544

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File Name: 0400158  
 Duration: 24  
 Site Code: 6  
 TRA ID: 0400158  
 Location 1: LOOP 202 EB OFF RAMP-RT TURNS W. of SCOTTSDALE RD  
 Count Type: VOL  
 Direction: EB  
 Latitude: 33.43567  
 Longitude: -111.92665

Date	1/29/2004		Average	
	AM	PM	AM	PM
12:00	20	131	20	131
12:15	19	111	19	111
12:30	16	110	16	110
12:45	15	117	15	117
1:00	18	128	18	128
1:15	18	140	18	140
1:30	17	104	17	104
1:45	13	91	13	91
2:00	14	114	14	114
2:15	10	117	10	117
2:30	11	119	11	119
2:45	6	133	6	133
3:00	5	109	5	109
3:15	4	120	4	120
3:30	6	121	6	121
3:45	9	101	9	101
4:00	9	124	9	124
4:15	20	110	20	110
4:30	11	108	11	108
4:45	17	110	17	110
5:00	18	108	18	108
5:15	33	104	33	104
5:30	47	109	47	109
5:45	69	121	69	121
6:00	82	117	82	117
6:15	88	116	88	116
6:30	103	106	103	106
6:45	117	144	117	144
7:00	118	103	118	103
7:15	176	102	176	102
7:30	124	77	124	77
7:45	150	76	150	76
8:00	167	84	167	84
8:15	197	64	197	64
8:30	185	65	185	65
8:45	182	65	182	65
9:00	152	74	152	74
9:15	113	69	113	69
9:30	136	94	136	94
9:45	130	78	130	78
10:00	185	69	185	69
10:15	153	58	153	58
10:30	114	48	114	48
10:45	96	49	96	49
11:00	121	62	121	62
11:15	139	43	139	43
11:30	143	42	143	42
11:45	145	33	145	33
<b>Total</b>	<b>3741</b>	<b>4598</b>	<b>3741</b>	<b>4598</b>
<b>Day Total</b>	<b>8339</b>		<b>8339</b>	
<b>Split</b>	0.8136		0.8136	

AM Peak Hour	8:00	8:00
AM Peak Vol	731	731
AM PHF	0.9277	0.9277
PM Peak Hour	12:30	12:30
PM Peak Vol	495	495
PM PHF	0.8839	0.8839

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File Name: 0400159  
 Duration: 24  
 Site Code: 6  
 TRA ID: 0400159  
 Location 1: SCOTTSDALE RD BTWN LOOP 202 EB OFF RAMPS (T-SPLIT)  
 Count Type: VOL  
 Direction: SB  
 Latitude: 33.43568  
 Longitude: -111.92648

Date	1/29/2004		Average	
	AM	PM	AM	PM
12:00	38	287	38	287
12:15	42	226	42	226
12:30	45	262	45	262
12:45	56	303	56	303
1:00	57	344	57	344
1:15	35	262	35	262
1:30	20	188	20	188
1:45	19	217	19	217
2:00	19	243	19	243
2:15	17	290	17	290
2:30	11	275	11	275
2:45	16	241	16	241
3:00	8	222	8	222
3:15	10	251	10	251
3:30	7	254	7	254
3:45	8	256	8	256
4:00	18	268	18	268
4:15	25	290	25	290
4:30	35	272	35	272
4:45	19	292	19	292
5:00	38	291	38	291
5:15	68	350	68	350
5:30	80	379	80	379
5:45	67	345	67	345
6:00	91	289	91	289
6:15	88	270	88	270
6:30	128	273	128	273
6:45	185	243	185	243
7:00	188	218	188	218
7:15	172	192	172	192
7:30	195	188	195	188
7:45	240	202	240	202
8:00	311	148	311	148
8:15	375	169	375	169
8:30	384	192	384	192
8:45	271	183	271	183
9:00	199	224	199	224
9:15	201	195	201	195
9:30	240	189	240	189
9:45	310	162	310	162
10:00	343	163	343	163
10:15	204	127	204	127
10:30	197	148	197	148
10:45	227	130	227	130
11:00	282	123	282	123
11:15	299	108	299	108
11:30	311	81	311	81
11:45	276	89	276	89
<b>Total</b>	<b>6475</b>	<b>10914</b>	<b>6475</b>	<b>10914</b>
<b>Day Total</b>	<b>17389</b>		<b>17389</b>	
<b>Split</b>	0.5933		0.5933	

AM Peak Hour	8:00	8:00
AM Peak Vol	1341	1341
AM PHF	0.8730	0.8730
PM Peak Hour	17:00	17:00
PM Peak Vol	1365	1365
PM PHF	0.9004	0.9004

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File Name: 0400160  
 Duration: 24  
 Site Code: 6  
 TRA ID: 0400160  
 Location 1: SCOTTSDALE RD S. of LOOP 202 EB ON RAMP  
 Count Type: VOL  
 Direction: NB  
 Latitude: 33.43458  
 Longitude: -111.92610

Date	1/29/2004		Average	
	AM	PM	AM	PM
12:00	174	578	174	578
12:15	130	595	130	595
12:30	96	441	96	441
12:45	130	439	130	439
1:00	152	422	152	422
1:15	117	466	117	466
1:30	91	556	91	556
1:45	73	840	73	840
2:00	61	657	61	657
2:15	38	425	38	425
2:30	27	449	27	449
2:45	33	500	33	500
3:00	38	727	38	727
3:15	19	806	19	806
3:30	8	725	8	725
3:45	19	626	19	626
4:00	25	469	25	469
4:15	33	504	33	504
4:30	30	564	30	564
4:45	38	688	38	688
5:00	49	588	49	588
5:15	60	555	60	555
5:30	59	484	59	484
5:45	90	466	90	466
6:00	126	512	126	512
6:15	172	478	172	478
6:30	232	506	232	506
6:45	308	515	308	515
7:00	326	416	326	416
7:15	374	432	374	432
7:30	494	361	494	361
7:45	546	388	546	388
8:00	403	417	403	417
8:15	484	348	484	348
8:30	330	384	330	384
8:45	349	387	349	387
9:00	306	420	306	420
9:15	298	492	298	492
9:30	263	532	263	532
9:45	270	351	270	351
10:00	253	287	253	287
10:15	292	279	292	279
10:30	397	216	397	216
10:45	535	252	535	252
11:00	326	231	326	231
11:15	348	193	348	193
11:30	322	166	322	166
11:45	456	183	456	183
<b>Total</b>	<b>9800</b>	<b>22316</b>	<b>9800</b>	<b>22316</b>
<b>Day Total</b>	<b>32116</b>		<b>32116</b>	
<b>Split</b>	0.4391		0.4391	

AM Peak Hour	11:45	11:45
AM Peak Vol	2070	2070
AM PHF	0.8697	0.8697
PM Peak Hour	15:00	15:00
PM Peak Vol	2884	2884
PM PHF	0.8945	0.8945

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File Name: 0400161  
 Duration: 24  
 Site Code: 6  
 TRA ID: 0400161  
 Location 1: LOOP 202 WB OFF RAMPS E. of SCOTTSDALE RD  
 Count Type: VOL  
 Direction: WB  
 Latitude: 33.43643  
 Longitude: -111.92382

Date	1/29/2004		Average	
	AM	PM	AM	PM
12:00	19	204	19	204
12:15	21	198	21	198
12:30	17	177	17	177
12:45	14	211	14	211
1:00	16	234	16	234
1:15	24	223	24	223
1:30	17	191	17	191
1:45	11	145	11	145
2:00	8	164	8	164
2:15	8	154	8	154
2:30	11	198	11	198
2:45	8	179	8	179
3:00	6	150	6	150
3:15	11	148	11	148
3:30	12	136	12	136
3:45	3	149	3	149
4:00	8	152	8	152
4:15	8	149	8	149
4:30	26	141	26	141
4:45	47	150	47	150
5:00	27	136	27	136
5:15	44	140	44	140
5:30	81	171	81	171
5:45	104	172	104	172
6:00	89	151	89	151
6:15	119	154	119	154
6:30	121	135	121	135
6:45	113	160	113	160
7:00	135	129	135	129
7:15	164	118	164	118
7:30	120	110	120	110
7:45	162	108	162	108
8:00	229	96	229	96
8:15	284	89	284	89
8:30	343	80	343	80
8:45	320	98	320	98
9:00	267	116	267	116
9:15	218	117	218	117
9:30	225	101	225	101
9:45	271	97	271	97
10:00	318	83	318	83
10:15	289	70	289	70
10:30	180	74	180	74
10:45	165	71	165	71
11:00	172	61	172	61
11:15	184	49	184	49
11:30	235	44	235	44
11:45	228	34	228	34
<b>Total</b>	<b>5502</b>	<b>6417</b>	<b>5502</b>	<b>6417</b>
<b>Day Total</b>	<b>11919</b>		<b>11919</b>	
<b>Split</b>	0.8574		0.8574	

AM Peak Hour 8:15 8:15  
 AM Peak Vol 1214 1214  
 AM PHF 0.8848 0.8848  
 PM Peak Hour 12:45 12:45  
 PM Peak Vol 859 859  
 PM PHF 0.9177 0.9177

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File Name: 0400162  
 Duration: 24  
 Site Code: 6  
 TRA ID: 0400162  
 Location 1: LOOP 202 WB OFF RAMP-RT TURNS E. of SCOTTSDALE RD  
 Count Type: VOL  
 Direction: WB  
 Latitude: 33.43652  
 Longitude: -111.92587

Date	1/29/2004		Average	
	Time	AM	PM	AM
12:00	5	85	5	85
12:15	13	106	13	106
12:30	7	94	7	94
12:45	7	103	7	103
1:00	9	107	9	107
1:15	8	94	8	94
1:30	6	85	6	85
1:45	5	78	5	78
2:00	7	98	7	98
2:15	5	69	5	69
2:30	6	102	6	102
2:45	6	99	6	99
3:00	5	84	5	84
3:15	11	94	11	94
3:30	7	78	7	78
3:45	2	101	2	101
4:00	6	90	6	90
4:15	6	91	6	91
4:30	12	86	12	86
4:45	27	93	27	93
5:00	14	91	14	91
5:15	25	100	25	100
5:30	27	102	27	102
5:45	59	86	59	86
6:00	54	78	54	78
6:15	64	79	64	79
6:30	77	72	77	72
6:45	49	72	49	72
7:00	52	73	52	73
7:15	80	53	80	53
7:30	85	57	85	57
7:45	97	41	97	41
8:00	121	59	121	59
8:15	134	46	134	46
8:30	128	44	128	44
8:45	138	32	138	32
9:00	145	43	145	43
9:15	132	58	132	58
9:30	132	49	132	49
9:45	171	42	171	42
10:00	113	41	113	41
10:15	115	40	115	40
10:30	99	31	99	31
10:45	95	31	95	31
11:00	76	28	76	28
11:15	60	18	60	18
11:30	101	15	101	15
11:45	88	17	88	17
<b>Total</b>	<b>2691</b>	<b>3335</b>	<b>2691</b>	<b>3335</b>
<b>Day Total</b>	<b>6026</b>		<b>6026</b>	
<b>Split</b>	0.8069		0.8069	

AM Peak Hour 9:00 9:00  
 AM Peak Vol 580 580  
 AM PHF 0.8480 0.8480  
 PM Peak Hour 12:15 12:15  
 PM Peak Vol 410 410  
 PM PHF 0.9579 0.9579

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File Name: 0400163  
 Duration: 24  
 Site Code: 6  
 TRA ID: 0400163  
 Location 1: SCOTTSDALE RD BTWN LOOP 202 WB OFF RAMPS (T-SPLIT)  
 Count Type: VOL  
 Direction: NB  
 Latitude: 33.43660  
 Longitude: -111.92613

Date	1/29/2004		Average	
	AM	PM	AM	PM
12:00	80	408	80	408
12:15	75	401	75	401
12:30	57	341	57	341
12:45	65	344	65	344
1:00	72	356	72	356
1:15	77	348	77	348
1:30	50	377	50	377
1:45	43	480	43	480
2:00	46	424	46	424
2:15	28	359	28	359
2:30	20	388	20	388
2:45	27	423	27	423
3:00	26	425	26	425
3:15	20	467	20	467
3:30	6	445	6	445
3:45	17	439	17	439
4:00	24	372	24	372
4:15	20	350	20	350
4:30	22	419	22	419
4:45	41	460	41	460
5:00	45	411	45	411
5:15	41	387	41	387
5:30	81	335	81	335
5:45	118	417	118	417
6:00	182	355	182	355
6:15	205	328	205	328
6:30	296	354	296	354
6:45	339	364	339	364
7:00	313	309	313	309
7:15	351	320	351	320
7:30	459	263	459	263
7:45	481	303	481	303
8:00	417	283	417	283
8:15	421	224	421	224
8:30	338	255	338	255
8:45	353	235	353	235
9:00	254	255	254	255
9:15	301	235	301	235
9:30	275	280	275	280
9:45	301	235	301	235
10:00	295	208	295	208
10:15	316	233	316	233
10:30	314	178	314	178
10:45	359	192	359	192
11:00	307	176	307	176
11:15	325	127	325	127
11:30	311	120	311	120
11:45	334	132	334	132
<b>Total</b>	<b>8948</b>	<b>15540</b>	<b>8948</b>	<b>15540</b>
<b>Day Total</b>	<b>24488</b>		<b>24488</b>	
<b>Split</b>	0.5758		0.5758	

AM Peak Hour 7:30 7:30  
 AM Peak Vol 1778 1778  
 AM PHF 0.9241 0.9241  
 PM Peak Hour 15:00 15:00  
 PM Peak Vol 1776 1776  
 PM PHF 0.9507 0.9507

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File Name: 0400164  
 Duration: 24  
 Site Code: 6  
 TRA ID: 0400164  
 Location 1: SCOTTSDALE RD N. of LOOP 202 WB ON RAMP  
 Count Type: VOL  
 Direction: SB  
 Latitude: 33.43737  
 Longitude: -111.92648

Date	1/29/2004		Average	
	AM	PM	AM	PM
12:00	107	403	107	403
12:15	85	447	85	447
12:30	74	382	74	382
12:45	85	391	85	391
1:00	111	450	111	450
1:15	103	454	103	454
1:30	68	383	68	383
1:45	49	394	49	394
2:00	37	414	37	414
2:15	38	410	38	410
2:30	27	457	27	457
2:45	30	419	30	419
3:00	36	448	36	448
3:15	31	457	31	457
3:30	27	534	27	534
3:45	26	493	26	493
4:00	37	487	37	487
4:15	37	486	37	486
4:30	48	511	48	511
4:45	35	552	35	552
5:00	47	569	47	569
5:15	58	600	58	600
5:30	123	596	123	596
5:45	120	600	120	600
6:00	117	584	117	584
6:15	155	525	155	525
6:30	195	444	195	444
6:45	230	396	230	396
7:00	273	392	273	392
7:15	294	332	294	332
7:30	346	303	346	303
7:45	321	269	321	269
8:00	317	296	317	296
8:15	384	262	384	262
8:30	397	280	397	280
8:45	400	305	400	305
9:00	337	257	337	257
9:15	256	316	256	316
9:30	264	276	264	276
9:45	305	288	305	288
10:00	339	260	339	260
10:15	391	218	391	218
10:30	281	213	281	213
10:45	341	212	341	212
11:00	359	193	359	193
11:15	357	192	357	192
11:30	406	157	406	157
11:45	476	131	476	131
<b>Total</b>	<b>8980</b>	<b>18438</b>	<b>8980</b>	<b>18438</b>
<b>Day Total</b>	<b>27418</b>		<b>27418</b>	
<b>Split</b>	0.4870		0.4870	

AM Peak Hour 11:30 11:30  
 AM Peak Vol 1732 1732  
 AM PHF 0.9097 0.9097  
 PM Peak Hour 17:15 17:15  
 PM Peak Vol 2380 2380  
 PM PHF 0.9917 0.9917

	Intersection Control Delay	Stopped Vehicle Count		Trucks - RT Lane		Date
		7:00 - 8:30 am	4:30 - 6:00 pm	AM	PM	
1	Indian School Rd & SR- 51 NB Off Ramp - Right Turn Lane Only	223	157	-	3	1/20/2004
2	Indian School Rd & SR- 51 SB Off Ramp - Right Turn Lane Only	488	141	1	1	1/20/2004
3	Cactus Rd & SR-51 NB Off Ramp - Right Turn Lane Only	293	378	10	1	1/21/2004
4	Cactus Rd & SR-51 SB Off Ramp - Right Turn Lane Only	230	298	5	3	1/21/2004
5	Glendale Rd & SR-51 NB Off Ramp - Right Turn Lane Only	456	115	15	0	2/3/2004
6	Glendale Rd & SR-51 SB Off Ramp - Right Turn Lane Only	223	293	5	1	2/3/2004
7	Greenway Rd & SR-51 NB Off Ramp - Right Turn Lane Only	413	476	10	4	1/27/2004
8	Greenway Rd & SR-51 SB Off Ramp - Right Turn Lane Only	81	393	4	1	1/27/2004
9	Bell Rd & Loop 101(W) NB Off Ramp - Right Turn Lane Only	423	744	24	5	1/28/2004
10	Bell Rd & Loop 101(W) SB Off Ramp - Right Turn Lane Only	674	1194	62	9	1/28/2004
11	Scottsdale Rd & Loop 202 EB Off Ramp - Right Turn Lane Only	530	403	10	1	1/29/2004
12	Scottsdale Rd & Loop 202 WB Off Ramp - Right Turn Lane Only	282	262	4	9	1/29/2004



## **Appendix B**

### **Control Delay Calculation Description & Example** (Excerpt from *Highway Capacity Manual 2000*)

## APPENDIX A. FIELD MEASUREMENT OF INTERSECTION CONTROL DELAY

### GENERAL NOTES

As an alternative to the estimation of control delay per vehicle using Equation 16-9 and the progression adjustment factor, delay at existing locations may be measured directly. There are a number of techniques for making this measurement, including the use of test-car observations, path tracing of individual vehicles, and the recording of arrival and departure volumes on a cycle-by-cycle basis. The method summarized here is based on direct observation of vehicle-in-queue counts at the intersection and normally requires two field personnel per lane group surveyed, unless the volume is light. Also needed is a multifunction digital watch that includes a countdown-repeat timer, with the countdown interval in seconds, plus a volume-count board with at least two tally counters. As an alternative, a laptop computer can be programmed to emit audio count markers at user-selected intervals, take volume counts, and execute real-time delay computations, thus simplifying data reduction.

In general, this method is applicable to all undersaturated signalized intersections. For oversaturated conditions, queue buildup normally makes the method impractical. Under such conditions, more personnel will be required to complete the field study, and other methods may be considered, such as an input-output technique or a zoned-survey technique.

In the input-output technique, different observers count arrivals separately from departures and vehicles in queue are calculated as the accumulated difference, subject to in-process checks for vehicles leaving the queue before they reach the stop line. The zoned-survey technique requires subdividing the approach into manageable segments to which the observers are assigned; they then count queued vehicles in their assigned zone. Both of these techniques require more personnel and are more complicated in setup and execution.

The method described here is applicable to situations in which the average maximum queue per cycle is no more than about 20 to 25 veh/ln. When queues are long or the demand to capacity ratio is near 1.0, care must be taken to continue the vehicle-in-queue count past the end of the arrival count period, as detailed below. This requirement is for consistency with the analytic delay equation used in the chapter text.

The method does not directly measure delay during deceleration and during a portion of acceleration, which are very difficult to measure without sophisticated tracking equipment. However, this method has been shown to yield a reasonable estimate of control delay. The method includes an adjustment for errors that may occur when this type of sampling technique is used, as well as an acceleration-deceleration delay correction factor. The acceleration-deceleration factor is a function of the typical number of vehicles in queue during each cycle and the normal free-flow speed when vehicles are unimpeded by the signal.

Exhibit A16-1 is a worksheet that can be used for recording observations and computation of average time-in-queue delay. Before beginning the detailed survey, the observers need to make an estimate of the average free-flow speed during the study period. Free-flow speed is the speed at which vehicles would pass unimpeded through the intersection if the signal were green for an extended period. This speed may be obtained by driving through the intersection a few times when the signal is green and there is no queue and recording the speed at a location least affected by signal control. Typically, the recording location should be upstream about midblock.

*Equipment and  
personnel requirements*

*Delay during  
deceleration is not  
directly measured*



1. Keeps track of the end of standing queues for each cycle in the survey period by observing the last vehicle in each lane that stops because of the signal. This count includes vehicles arriving when the signal is actually green but stopped because vehicles in front have not yet started moving. For purposes of the survey, a vehicle is considered as having joined the queue when it approaches within one car length of a stopped vehicle and is itself about to stop. This definition is used because of the difficulty of keeping precise track of the moment when a vehicle comes to a stop. All vehicles that join a queue are then included in the vehicle-in-queue counts until they cross the stop line.

2. At regular intervals of between 10 and 20 s, records the number of vehicles in queue (e.g., using the countdown-repeat timer on a digital watch to signal the count time). The regular intervals should not be an integral divisor of the cycle length (e.g., if the cycle length is 120 s, 14-s or 16-s count intervals should be used, not 15-s intervals). Vehicles in queue are those that are included in the queue of stopping vehicles as defined in Step 1 and have not yet exited the intersection. For through vehicles, exiting the intersection can be considered to occur when the rear axle of a vehicle crosses the stop line. For turning vehicles, exiting the intersection occurs the instant a vehicle clears opposing through traffic or pedestrians to which it must yield and begins accelerating back to free-flow speed. Note that the vehicle-in-queue count often includes some vehicles that have regained speed but have not yet exited the intersection.

3. Enters the vehicle-in-queue counts in the appropriate box on the worksheet. Cycles of the survey period are listed in the second column of the sheet, after the column to record clock time every five cycles, and interval count identifiers are listed as column headings. For ease in conducting the study, the survey period is most conveniently defined as an integer number of cycles, though a precisely defined time length for the survey period (e.g., 15 min) can be used. The key point is that the end of the survey period must be clearly defined in advance since the last arriving vehicle or vehicles that stop in the period must be identified and counted until they exit the intersection, per the next step.

4. At the end of the survey period, continues taking vehicle-in-queue counts for all vehicles that arrived during the survey period until all of them have exited the intersection. This step requires mentally noting the last stopping vehicle that arrived during the survey period in each lane of the lane group and continuing the vehicle-in-queue counts until the last stopping vehicle or vehicles, plus all vehicles in front of the last stopping vehicles, exit the intersection. Stopping vehicles that arrive after the end of the survey period are not included in the final vehicle-in-queue counts.

Observer 2 performs the following study task.

1. During the entire survey period, maintains separate volume counts of total vehicles arriving during the survey period and total vehicles arriving during the survey period that stop one or more times. A vehicle stopping multiple times is counted only once as a stopping vehicle. Enters these volumes in the appropriate boxes on the worksheet.

Data reduction is accomplished with the following steps.

1. Sum each column of vehicle-in-queue counts, then sum the column totals for the entire survey period.

2. A vehicle recorded as part of a vehicle-in-queue count is in queue, on average, for the time interval between counts. The average time-in-queue per vehicle arriving during the survey period is estimated using Equation A16-1.

$$\text{Time-in-queue per vehicle} = \left( I_s \cdot \frac{\sum V_{iq}}{V_{tot}} \right) \cdot 0.9 \quad (\text{A16-1})$$

where

$$I_s = \text{interval between vehicle-in-queue counts (s),}$$

$$\sum V_{iq} = \text{sum of vehicle-in-queue counts (veh),}$$

$V_{tot}$  = total number of vehicles arriving during the survey period (veh), and  
 0.9 = empirical adjustment factor.

The 0.9 adjustment factor accounts for the errors that may occur when this type of sampling technique is used to derive actual delay values, normally resulting in an overestimate of delay. Research has shown the correction required to be fairly consistent over a variety of conditions.

3. Compute the fraction of vehicles stopping and the average number of vehicles stopping per lane in each signal cycle, as indicated on the worksheet.
4. Using Exhibit A16-2, look up a correction factor appropriate to the lane group free-flow speed and the average number of vehicles stopping per lane in each cycle. This factor adds an adjustment for deceleration and acceleration delay, which cannot be measured directly with manual techniques.

EXHIBIT A16-2. ACCELERATION-DECELERATION DELAY CORRECTION FACTOR, CF (s)

Free-Flow Speed	≤ 7 Vehicles	8 - 19 Vehicles	20 - 30 Vehicles <sup>a</sup>
≤ 37 mi/h	+ 5	+ 2	- 1
> 37-45 mi/h	+ 7	+ 4	+ 2
> 45 mi/h	+ 9	+ 7	+ 5

Note:

- a. Vehicle-in-queue counts in excess of about 30 vehicles per lane are typically unreliable.

5. Multiply the correction factor by the fraction of vehicles stopping, then add this product to the time-in-queue value of Step 2 to obtain the final estimate of control delay per vehicle.

Exhibit A16-3 presents a sample computation for a study site over a 15-min period, operating with a 115-s cycle over almost eight cycles. The exhibit is annotated to clarify the procedure. The 15-s count interval is not an integral divisor of the cycle length, thus eliminating potential survey bias due to queue buildup in a regular, cyclic pattern.

Exhibit A16-4 shows how the field study would have been finished if a queue still remained at the end of the 15-min study period. Only the vehicles that arrived during the 15-min period would be counted.

If the study site is an actuated signal with varying cycle and phase lengths, the count interval may be chosen as the most convenient value for conducting the field survey on the basis of volume and vantage point considerations.



EXHIBIT A16-4. EXAMPLE APPLICATION WITH RESIDUAL QUEUE AT END

INTERSECTION CONTROL DELAY WORKSHEET												
<b>General Information</b>						<b>Site Information</b>						
Analyst _____						Intersection <u>Cicero &amp; Belmont</u>						
Agency or Company _____						Area Type <input type="checkbox"/> CBD <input checked="" type="checkbox"/> Other						
Date Performed _____						Jurisdiction _____						
Analysis Time Period <u>PM</u>						Analysis Year <u>1999</u>						
<b>Input Initial Parameters</b>												
Number of lanes, N <u>2</u>						Total vehicles arriving, $V_{tot}$ _____						
Free-flow speed, FFS (mi/h) <u>40</u>						Stopped-vehicle count, $V_{stop}$ _____						
Survey count interval, $I_s$ (s) <u>15</u>						Cycle length, C (s) _____						
<b>Input Field Data</b>												
Clock Time	Cycle Number	Number of Vehicles in Queue										
		Count Interval										
		1	2	3	4	5	6	7	8	9	10	
4:47	3	4	7	11	16	9	0	} Queue count in previous example				
4:47	6	4	4*	4*	4*	0	First four in queue have cleared by now					
say 15 min. survey period ends here												
4* - last stopping vehicles in survey period; count only until they clear.												
Total		37	61	81	99	52	4	0	6			
<b>Computations</b>												
Total vehicles in queue, $\sum V_{iq} =$ _____						Number of cycles surveyed, $N_c =$ _____						
Time-in-queue per vehicle, $d_{vq} = (I_s \cdot \frac{\sum V_{iq}}{V_{tot}}) \cdot 0.9$ _____ s						Fraction of vehicles stopping, $FVS = \frac{V_{stop}}{V_{tot}}$ _____						
No. of vehicles stopping per lane each cycle = $\frac{V_{stop}}{(N_c \times N)}$ _____						Accel/Decel correction delay, $d_{ad} = FVS \cdot CF$ _____ s						
Accel/Decel correction factor, CF (Ex. A16-2) _____						Control delay/vehicle, $d = d_{vq} + d_{ad}$ _____ s						